CERTIFICATE OF AUTHENTICITY

THIS IS TO CERTIFY THAT THE FOLLOWING ELECTRONIC RECORDS ARE TRUE AND ACCURATE REPRODUCTIONS OF THE ORIGINAL RECORDS OF JAMES CITY COUNTY GENERAL SERVICES DEPARTMENT- STORMWATER DIVISION; WERE SCANNED IN THE REGULAR COURSE OF BUSINESS PURSUANT TO GUIDELINES ESTABLISHED BY THE LIBRARY OF VIRGINIA AND ARCHIVES; AND HAVE BEEN VERIFIED IN THE CUSTODY OF THE INDIVIDUAL LISTED BELOW.

BMP NUMBER: **MC011**

DATE VERIFIED: **January 9, 2013**

QUALITY ASSURANCE TECHNICIAN: **Leah Hardenbergh**

Leah Hardenbergh

LOCATION: WILLIAMSBURG, VIRGINIA

Stormwater Division

MEMORANDUM

Date: March 28, 2012

To: Michael J. Gillis, Virginia Correctional Enterprises Document Management Services

From: Leah Hardenbergh

PO: 110426

Re: Files Approved for Scanning

General File ID or BMP ID: **MC011**

PIN: **4710100007**

Owner Name (if known): **INGRAM ROAD LLC**

Legal Property Description: **ADJ J B VAIDEN, 6 98 AC**

Site Address: **108 INGRAM ROAD**

(For internal use only):

Box # 3

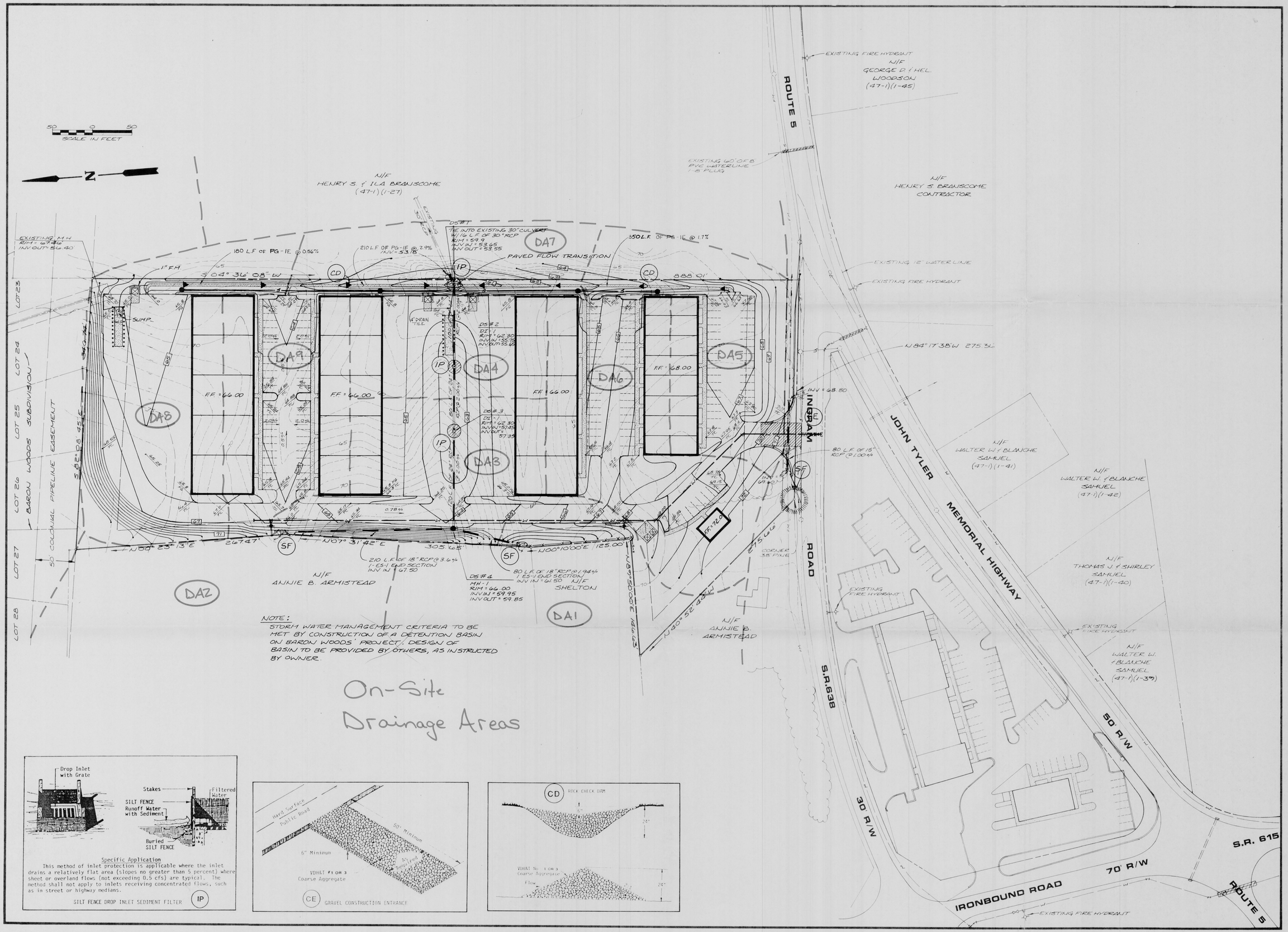
Agreements (in file as of scan date): N Book or Doc #:

MC-011

Contents for Stormwater Management Facilities As-built Files

Each file is to contain:

1. As-built plan
2. Completed construction certification
3. Construction Plan
4. Design Calculations
5. Watershed Map
6. Maintenance Agreement
7. Correspondence with owners
8. Inspection Records
9. Enforcement Actions



NOTE:
STORM WATER MANAGEMENT CRITERIA TO BE MET BY CONSTRUCTION OF A DETENTION BASIN ON BARON WOODS' PROJECT. DESIGN OF BASIN TO BE PROVIDED BY OTHERS, AS INSTRUCTED BY OWNER.

Specific Application
This method of inlet protection is applicable where the inlet drains a relatively flat area (slopes no greater than 5 percent) where sheet or overland flows (not exceeding 0.5 cfs) are typical. The method shall not apply to inlets receiving concentrated flows, such as in street or highway medians.

SILT FENCE DROP INLET SEDIMENT FILTER (IP)

(CE) GRAVEL CONSTRUCTION ENTRANCE

No.	DATE	REVISION / COMMENT / NOTE	BY

COMMONWEALTH OF VIRGINIA
RICHARD A. CASTELLO
CERTIFICATE NO.
017239
REGISTERED PROFESSIONAL ENGINEER

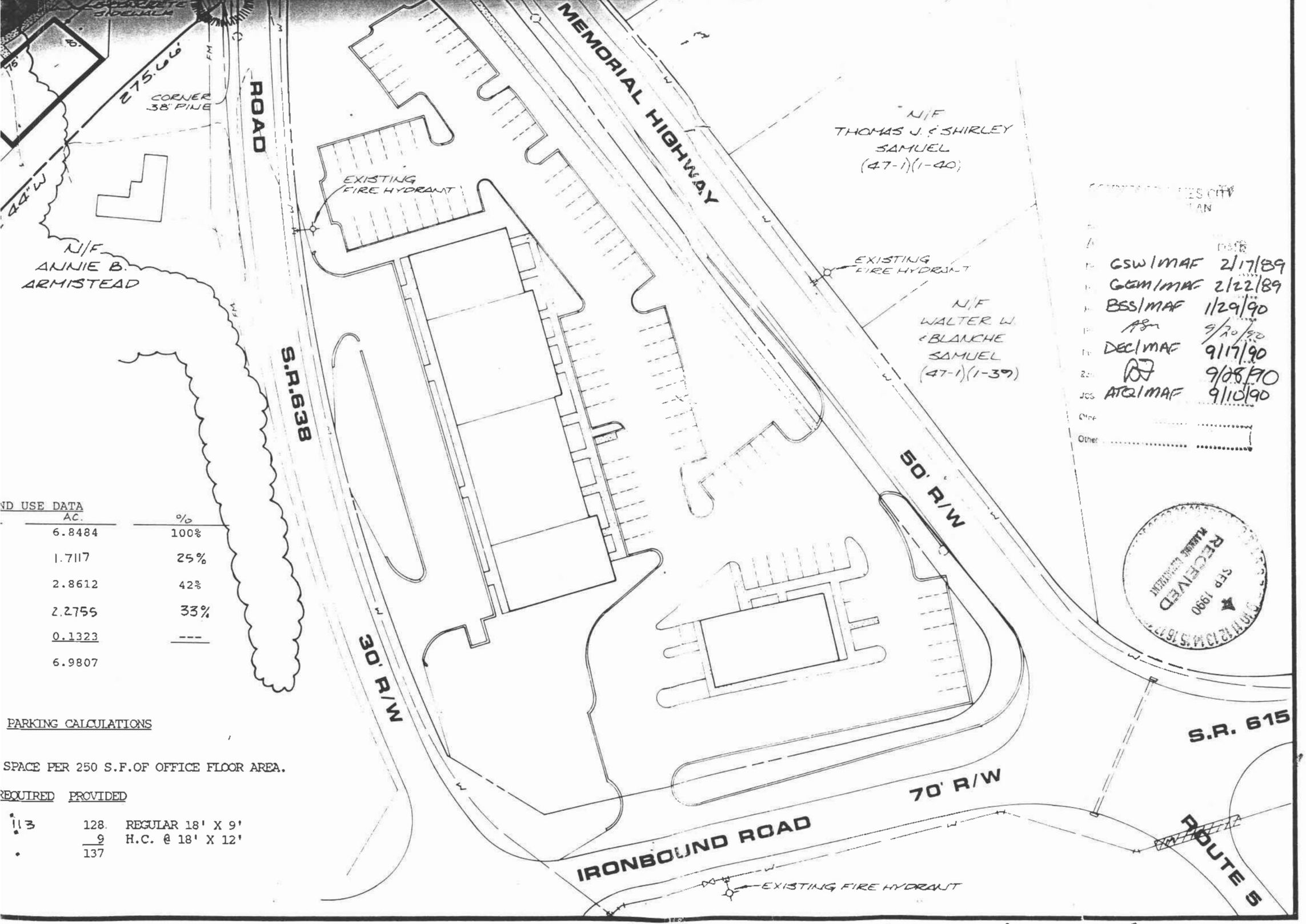
AES, a professional corporation
1761 Jamestown Road
Williamsburg, Virginia 23185
804-253-0040
Engineering, Planning, Surveying

DRAINAGE, SEDIMENT AND EROSION CONTROL PLAN
DIGGES BROTHERS OFFICE/WAREHOUSES

OWNER/DEVELOPER: DIGGES BROTHERS, INC.

JAMES CITY COUNTY VIRGINIA

Designed	Drawn
SOW	RMS
Scale	Date
NOTED	MAY, 1988
Project No.	
7080	
Drawing No.	
3	



ND USE DATA

AC.	%
6.8484	100%
1.7117	25%
2.8612	42%
2.2755	33%
0.1323	---
6.9807	

PARKING CALCULATIONS

SPACE PER 250 S.F.OF OFFICE FLOOR AREA.

REQUIRED	PROVIDED	
113	128	REGULAR 18' X 9'
	9	H.C. @ 18' X 12'
	137	

		Date
	CSW/MAF	2/17/89
	GEM/MAF	2/22/89
	BSS/MAF	1/29/90
	AB	9/20/90
	DEC/MAF	9/17/90
	DT	9/28/90
JCS	ATQ/MAF	9/10/90
Other		
Other		

RECEIVED
SEP 1990
PLANNING COMMISSION

SITE & LANDSCAPING PLAN

DIGGES BROTHERS OFFICE/WAREHOUSES

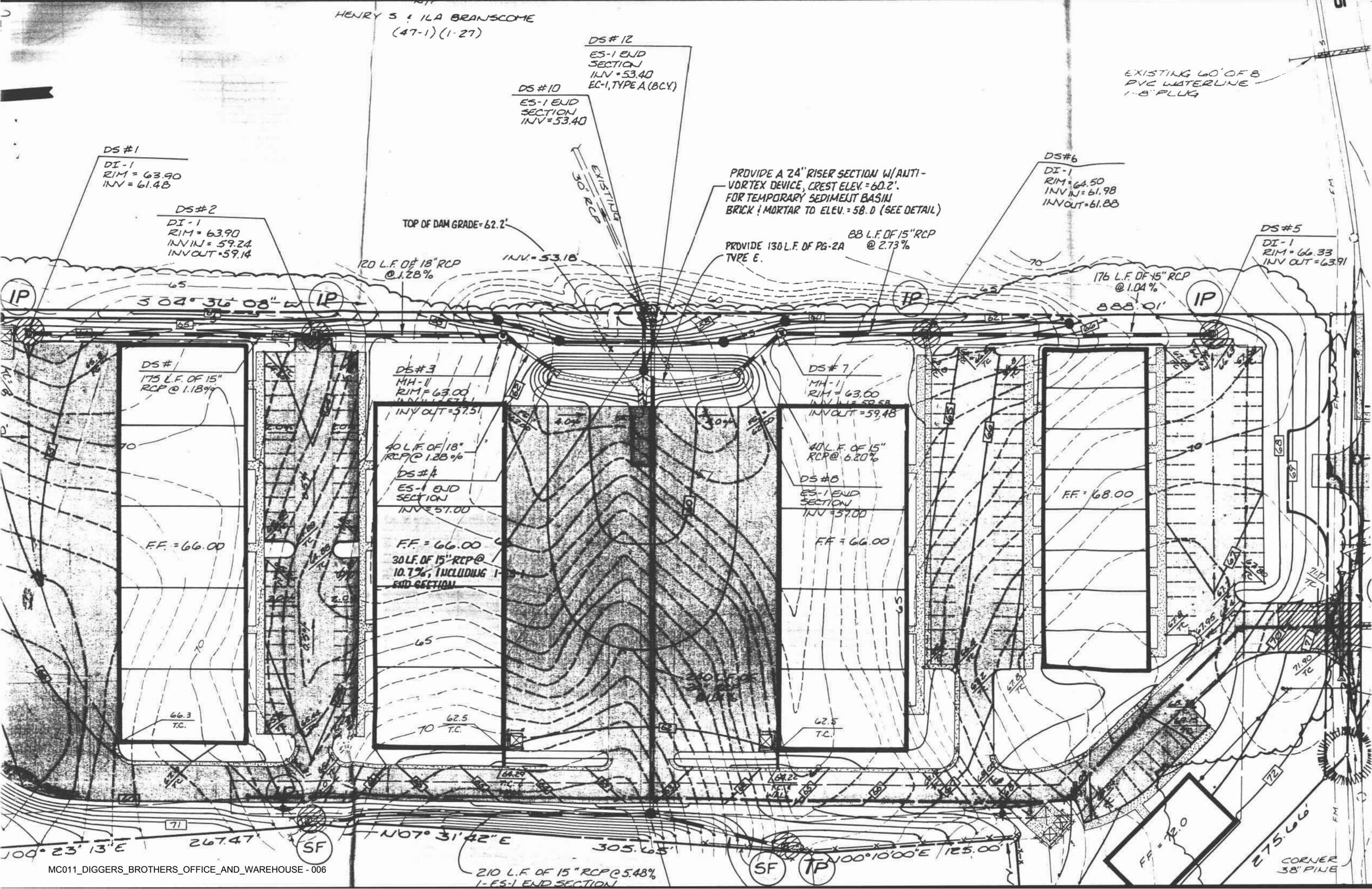
OWNER/DEVELOPER: DIGGES BROTHERS, INC.

JAMES CITY COUNTY VIRGINIA

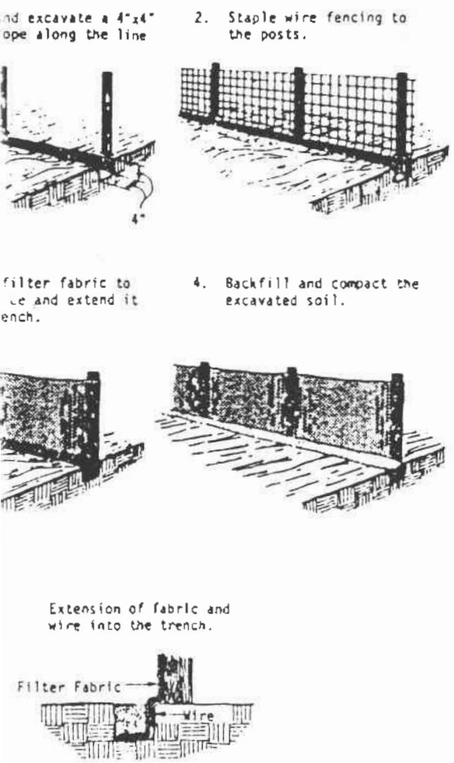
Designed SOW	Drawn C.A.N
Scale 1"=50'	Date MAY,1
Project No 7080	
Drawing N 1	

MC011_DIGGERS_BROTHERS_OFFICE_AND_WAREHOUSE - 005

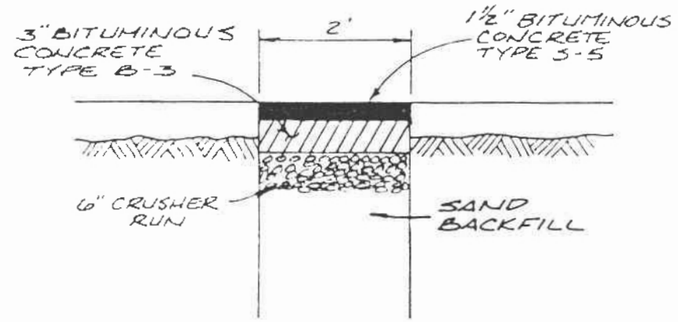
MC011; SP-62-88



MC011_DIGGERS_BROTHERS_OFFICE_AND_WAREHOUSE - 006

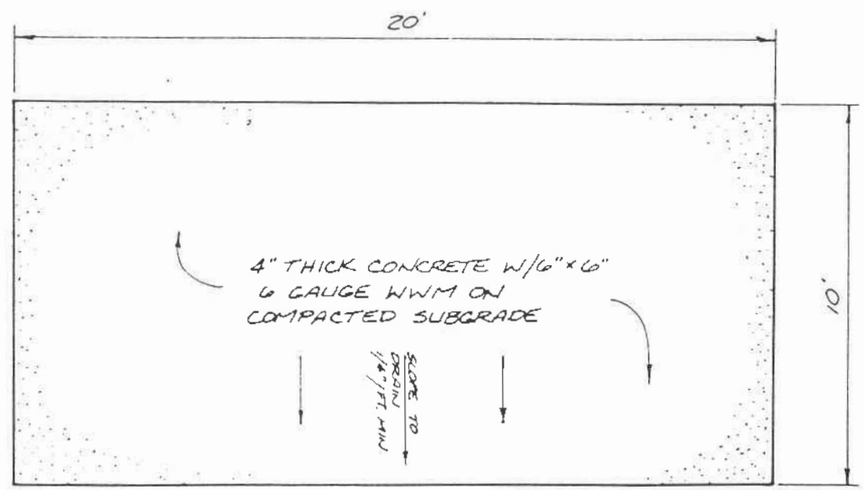


NSTRUCTION OF A SILT FENCE"



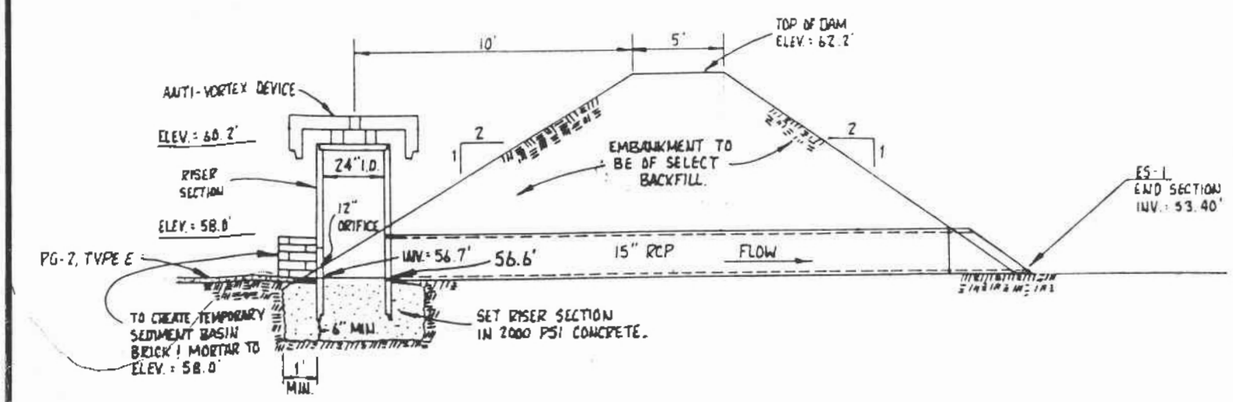
OPEN CUT RESTORATION DETAIL

N.T.S.

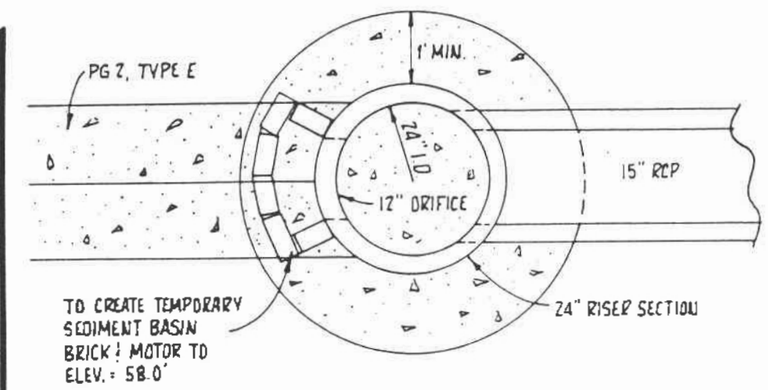


DUMPSTER PAD DETAIL

N.T.S.

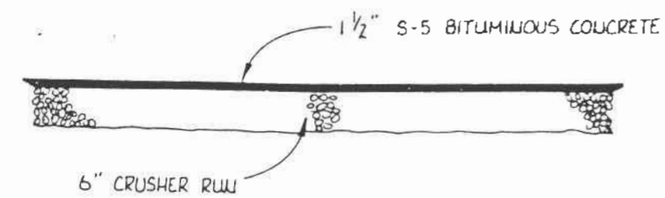


CROSS SECTION

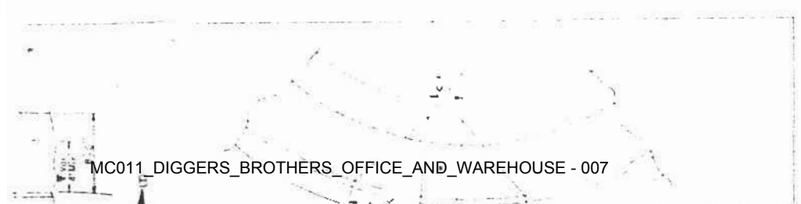


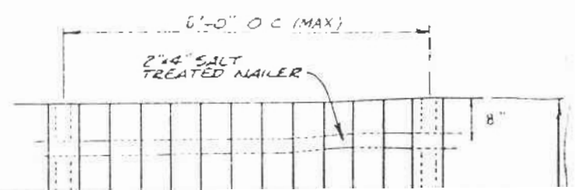
PLAN VIEW

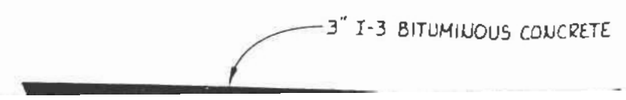
SEDIMENT BASIN DETAIL
(NTS)



PARKING AREA







Project: DIGGES BROTHERS OFFICE/WAREHOUSE

Site Plan No:	SP-62-88	Geo No:	3A5
E+S Fee:	Yes	Amt Paid:	$1,000
Date Paid:	5/17/1988		
Released?		Release Date:	
Planner/Engineers:			
Inspector Area Num:	2	Notations	
Comments:			

LD Permit No:	92-21	Fiscal Year:	1992
Acres Disturbed:	2.3	DOC Required:	
LD Issue Date:	9/13/1991	Received:	
LD Expire Date:		Recorded:	
Watershed Insp Code:		Treas Cert:	
Comments:	HUC Code:	VSMP:	No

RELEASED 2/8/00

Storm Water

Cert to Construct	No
Date:	
Inspection Fee	No
Amount:	$0.00
Number of BMPs:	
Conveyance System Length	
Storm Drain Length (Ft):	
Open Channel Length (Ft):	

Subdivisions/Public Improvement

Street Light Fee		Amount:	$0.00
Agreement?	No	Date Paid:	
Public Imp?	No	Agrmt Date:	
Surety Type:		Amount:	$0
Notation:		Surety Released?	
Expiration Date:			
Surety Number:			
Surety/Principal:			
Comments:			

Siltation

Agreemnt?	Yes	Agrmnt Date:	
Surety Type:	CHECK	Amount:	$18,000
Notation:		Surety Released:	
Expiration Date:	9/13/2012		
Surety Number:			
Surety/Principal:	Thomas R. O'Rourk		
Comments:			

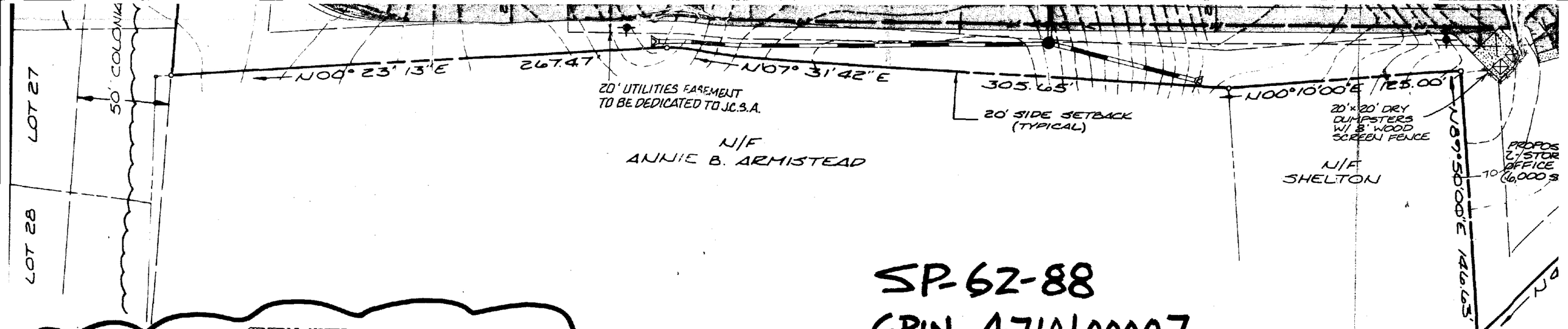
Reviewd 3/14/12 - keep all- Not build per plan

General Comments:

Delete Record | Undo | Last Permit No | Add | Find | Save | Print | Menu | CaseTrak

4710100007
GPIN ~~4710100026~~
108 Ingram Road
6.98 acres
Zoned B-1
SP-62-88 (SP-67-97)
Siltation agreement siltation + ES (9/10/91)
ENV approval ~~9/17/90~~ 5/1/89
Required AB dwgs for sed. basin; No comments for non-Tim

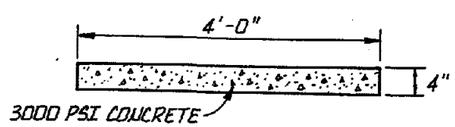
- BS+P notes non-compliance with approved site plan landscaping, parking lot, dumpster pad fences incomplete. However they issued C.O. (April 1995)
- Appears $18,000 siltation surety being held
- Onsite stormwater basin was a TSB. Onsite runoff to existing 30" RCP storm system to regional County Mill Creek Detention Basin MC045.
- Onsite storm system DI-1's and 15", 18" + 24" storm pipe RCP. →



SP-62-88
GPIN 4710100007
MC 011

GENERAL NOTES

1. SITE IS ZONED B-1, GENERAL BUSINESS.
2. TAX MAP PARCEL NO. (47-1)(1-7).
3. TOTAL ACREAGE OF SITE = 6.9807 ACRES.
4. ALL UTILITY WIRES ARE TO BE INSTALLED UNDERGROUND.
5. SITE IS TO BE SERVED WITH BOTH PUBLIC WATER AND SEWER.
6. PLAT FOR THE CONVEYANCE OF UTILITY EASEMENTS SHALL BE SUBMITTED TO JAMES CITY COUNTY UPON FIELD LOCATIONS.
7. ALL PARKING SPACES ARE TO BE PROVIDED WITH 4 INCHES PAINTED STRIPING.
8. ALL HANDICAP SPACES ARE TO BE PROVIDED WITH A "HANDICAPPED PARKING SPACE" SIGN, A MINIMUM OF 4 FEET IN HEIGHT.
9. LIGHTING TO BE PHOTOCELL ACTIVATED.
10. DEVELOPER AGREES TO CONFORM WITH THE CURRENT JAMES CITY COUNTY ORDINANCE REQUIREMENTS CONCERNING SIGNAGE.
11. BUILDINGS ARE NOT TO EXCEED A MAXIMUM HEIGHT OF 24 FEET.
12. BUILDINGS ARE CLASSIFIED AS USE GROUP B/S1 AND ARE TO BE OF CONSTRUCTION TYPE 3B.
13. FREESTANDING ROAD SIGN IS NOT TO EXCEED A MAXIMUM HEIGHT OF 20 FEET NOR A SIZE OF 32 SQUARE FEET PER FACE.



TYPICAL CONCRETE SIDEWALK DETAIL

SCALE: 1/2" = 1'-0"

LANDSCAPING NOTES

1. TREES REQUIRED IN PARKING LOT = 137/10 = 14
   TREES PROVIDED = 17
2. PERIMETER TREE REQUIREMENT = ONE PER 30 L.F.
   PERIMETER LENGTH = 2,313 L.F.
   TREES REQUIRED = 77
   TREES PROVIDED = (SEE LANDSCAPING PLAN)
3. THE SITE CONTAINS 50% VARIOUS HARDWOODS (MOSTLY MAPLE, WALNUT AND OAKS) 8" - 10" IN DIAMETER, 30% EVERGREENS (PINES AND HOLLY) 6" - 10" IN DIAMETER, 20% UNDERBRUSH.
4. LANDSCAPING IS TO BE PROVIDED AT THE BASE OF THE PROPOSED FREESTANDING ROAD SIGN.
5. FRONT FOUNDATION PLANTINGS, CONTAINING AZELEAS, JUNIPERS AND THE LIKE, ARE TO BE PROVIDED ALONG THE SOUTH SIDE OF THE TWO PROPOSED BUILDINGS FACING INGRAM ROAD.

PLANTING SCHEDULE

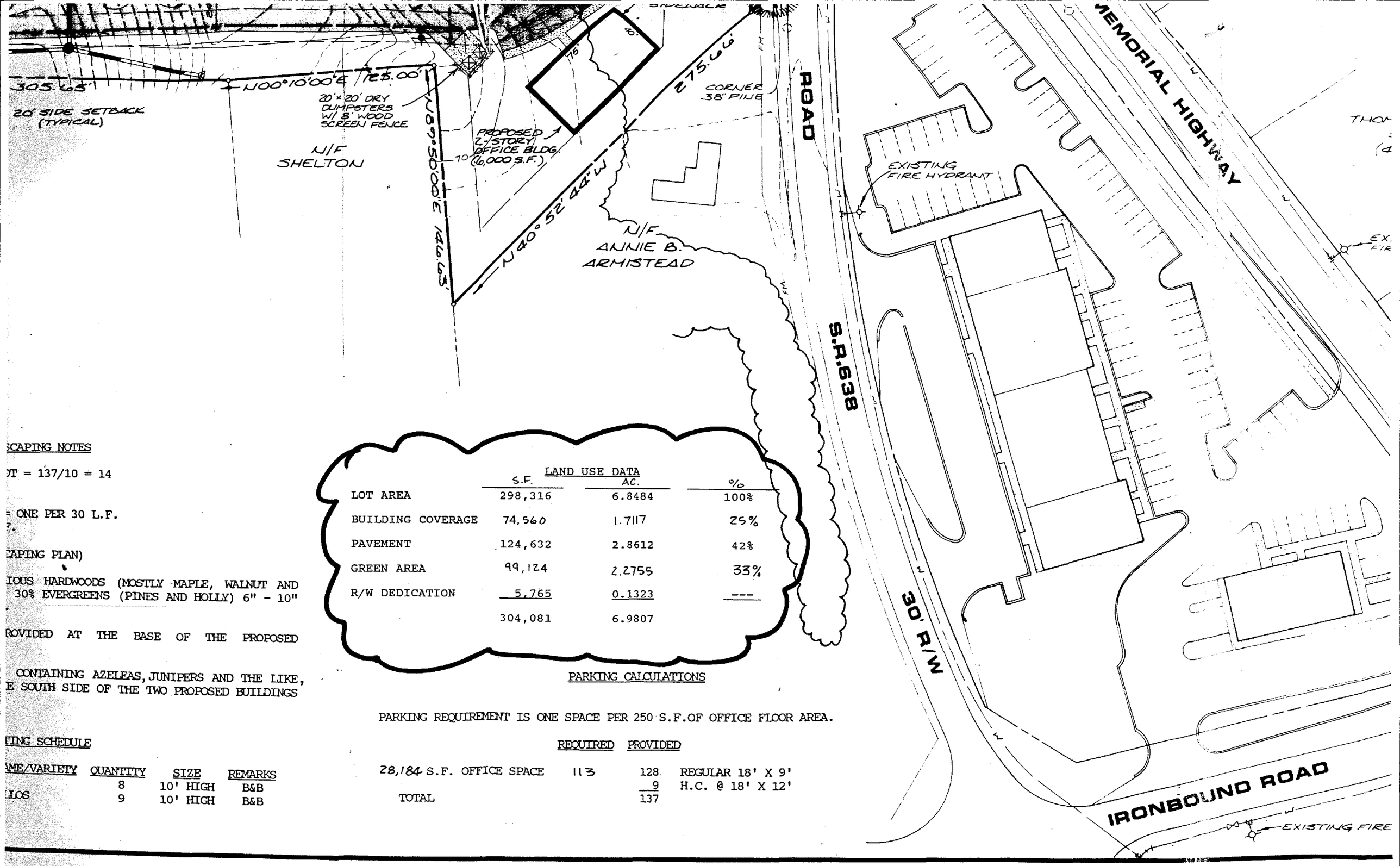
MARK	COMMON NAME	BOTANICAL NAME/VARIETY	QUANTITY	SIZE	REMARKS
A	RED MAPLE	ACER RUBRUM	8	10' HIGH	B&B
B	WILLOW OAK	QUERCOS PHELLOS	9	10' HIGH	B&B

	S.F
LOT AREA	298,
BUILDING COVERAGE	74,5
PAVEMENT	124,
GREEN AREA	99,
R/W DEDICATION	5,
	304,

PARKING REQUIREMENT IS

28,184 S.F. OFFICE SPA

TOTAL



SCAPING NOTES

OT = 137/10 = 14

= ONE PER 30 L.F.

CAPING PLAN)

IOUS HARDWOODS (MOSTLY MAPLE, WALNUT AND 30% EVERGREENS (PINES AND HOLLY) 6" - 10"

ROVIDED AT THE BASE OF THE PROPOSED

CONTAINING AZELEAS, JUNIPERS AND THE LIKE, E SOUTH SIDE OF THE TWO PROPOSED BUILDINGS

TING SCHEDULE

AME/VARIETY	QUANTITY	SIZE	REMARKS
	8	10' HIGH	B&B
LLOS	9	10' HIGH	B&B

LAND USE DATA

	S.F.	AC.	%
LOT AREA	298,316	6.8484	100%
BUILDING COVERAGE	74,560	1.7117	25%
PAVEMENT	124,632	2.8612	42%
GREEN AREA	99,124	2.2755	33%
R/W DEDICATION	5,765	0.1323	---
	304,081	6.9807	

PARKING CALCULATIONS

PARKING REQUIREMENT IS ONE SPACE PER 250 S.F.OF OFFICE FLOOR AREA.

	REQUIRED	PROVIDED	
28,184 S.F. OFFICE SPACE	113	128	REGULAR 18' X 9'
		9	H.C. @ 18' X 12'
TOTAL		137	

EXISTING 60' OF 8" PVC WATERLINE 1-8" PLUG

PROPOSED EDGE OF PAVEMENT

NOTE: SEE SHEET #6 FOR COMPLETE LANDSCAPE DETAILS.

10'x20' DRY DUMPSTER W/8' WOOD SCREEN FENCE (SEE DETAIL)

EXISTING 30" RCP

INV.=53.18'

S 04° 36' 08" W

8' CONCRETE DITCH

888.01'

50' REAR SETBACK

PROPOSED WAREHOUSE F.F.=66.0' 17,000 S.F. (WAREHOUSE) 3,000 S.F. (OFFICE SPACE)

HANDICAP RAMP

OFFICE SPACE

5' CONCRETE SIDEWALK (TYPICAL)

PROPOSED WAREHOUSE F.F.=66.0' 14,688 S.F. (WAREHOUSE) 2592 S.F. (OFFICE SPACE)

10'x20' DRY DUMPSTER W/8' WOOD SCREEN FENCE (SEE DETAIL)

HANDICAP RAMP

PROVIDE CONCRETE BUMPER BLOCKS ALONG EDGE OF PAVEMENT.

250W H.P.S. LUMINARE ON 20' POLE (TYPICAL)

PROPOSED WAREHOUSE F.F.=66.0' 14,688 S.F. (WAREHOUSE) 2592 S.F. (OFFICE SPACE)

HANDICAP RAMP

10'x20' DRY DUMPSTER W/8' WOOD SCREEN FENCE (SEE DETAIL)

PROPOSED OFFICE SPACE F.F.=68.0' 14000 S.F. OFFICE SPACE

HANDICAP RAMP

HANDICAP RAMP

5' CONCRETE SIDEWALK

INGRAM ROAD

23' 13" E 267.47'

N07° 31' 42" E

20' UTILITIES EASEMENT TO BE DEDICATED TO J.C.S.A.

305.65'

20' SIDE SETBACK (TYPICAL)

N00° 10' 00" E 125.00'

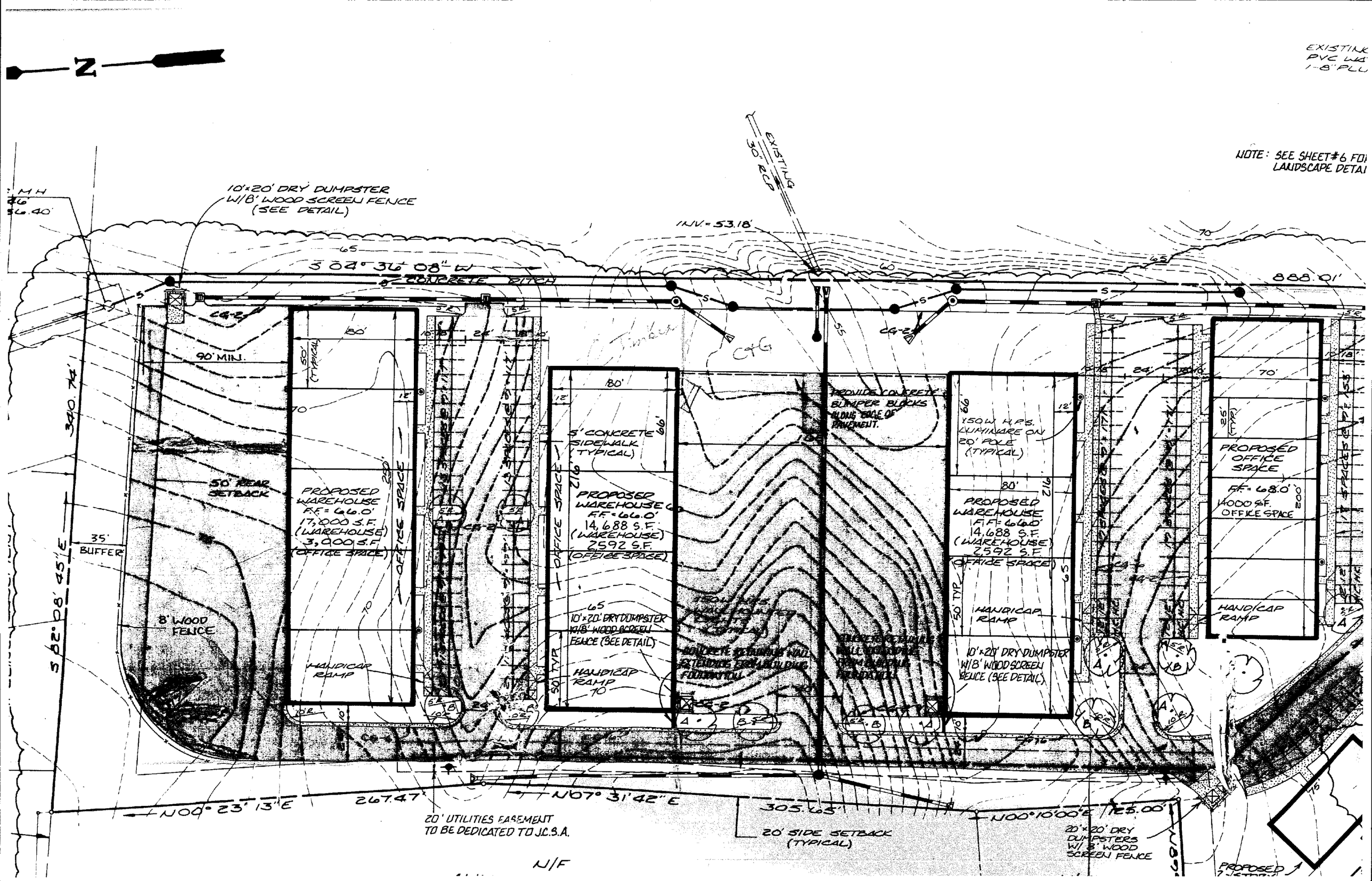
20'x20' DRY DUMPSTERS W/ 8' WOOD SCREEN FENCE

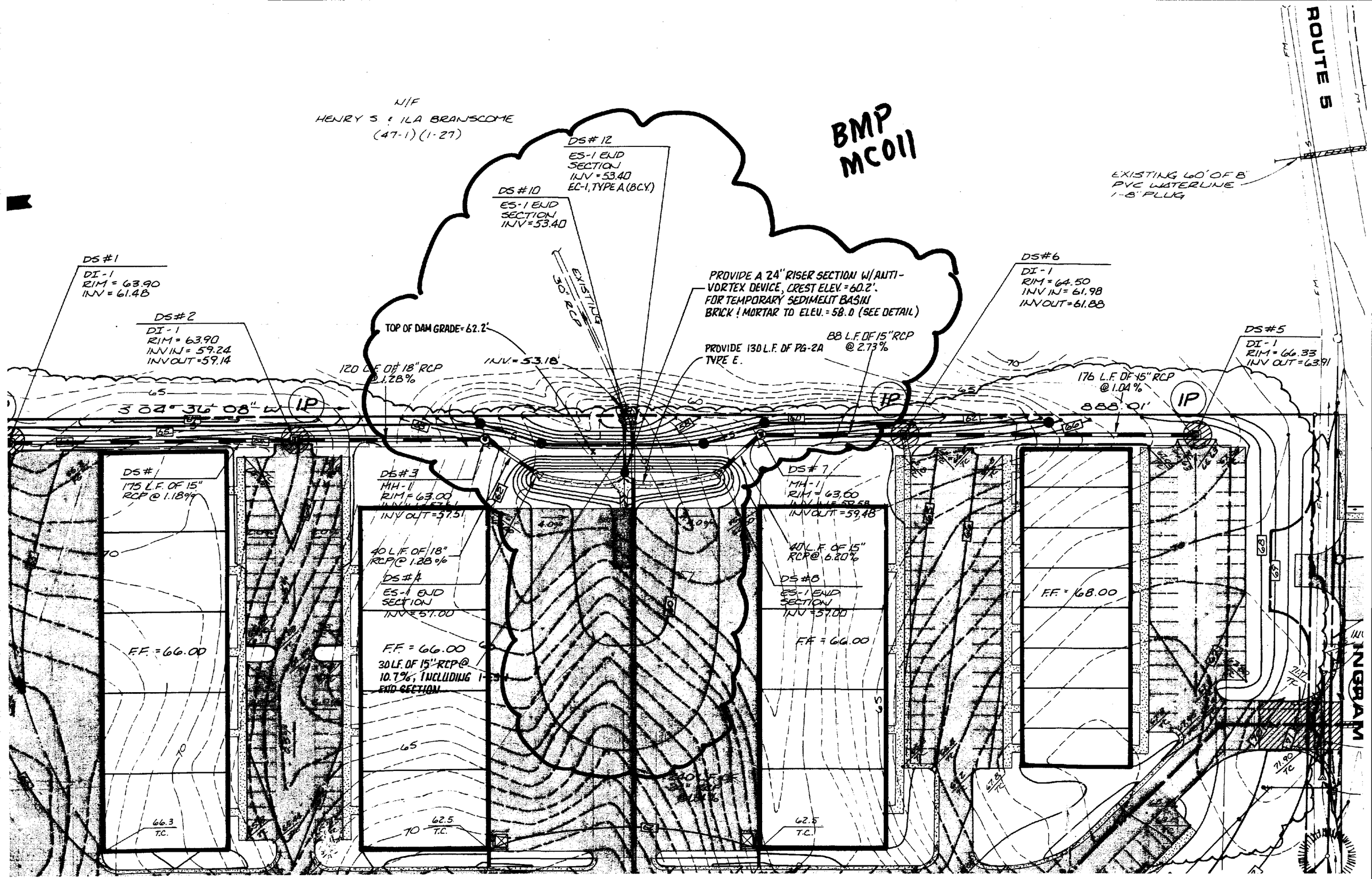
N/F ANNIE B. ARMISTEAD

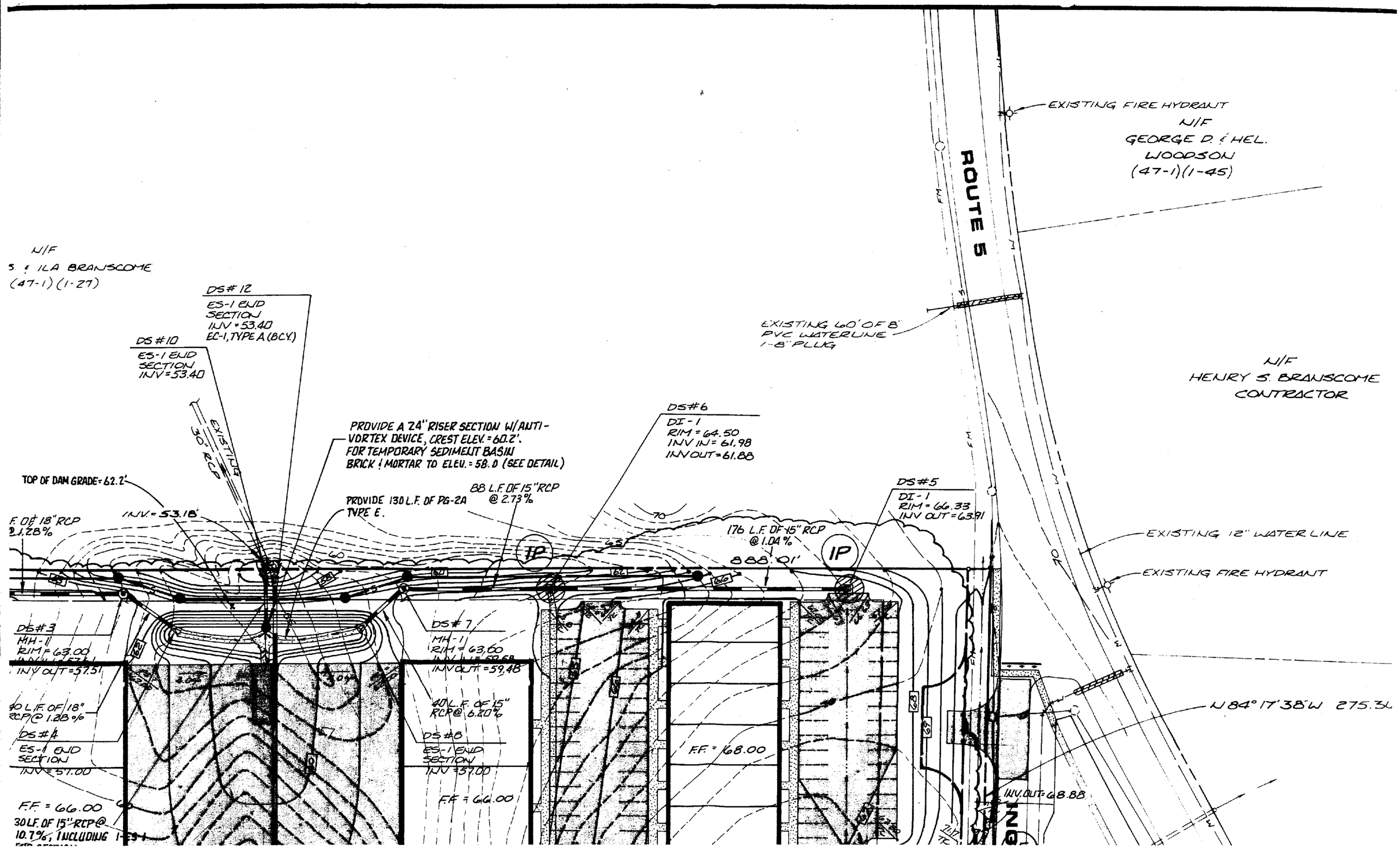
N/F SHELTON

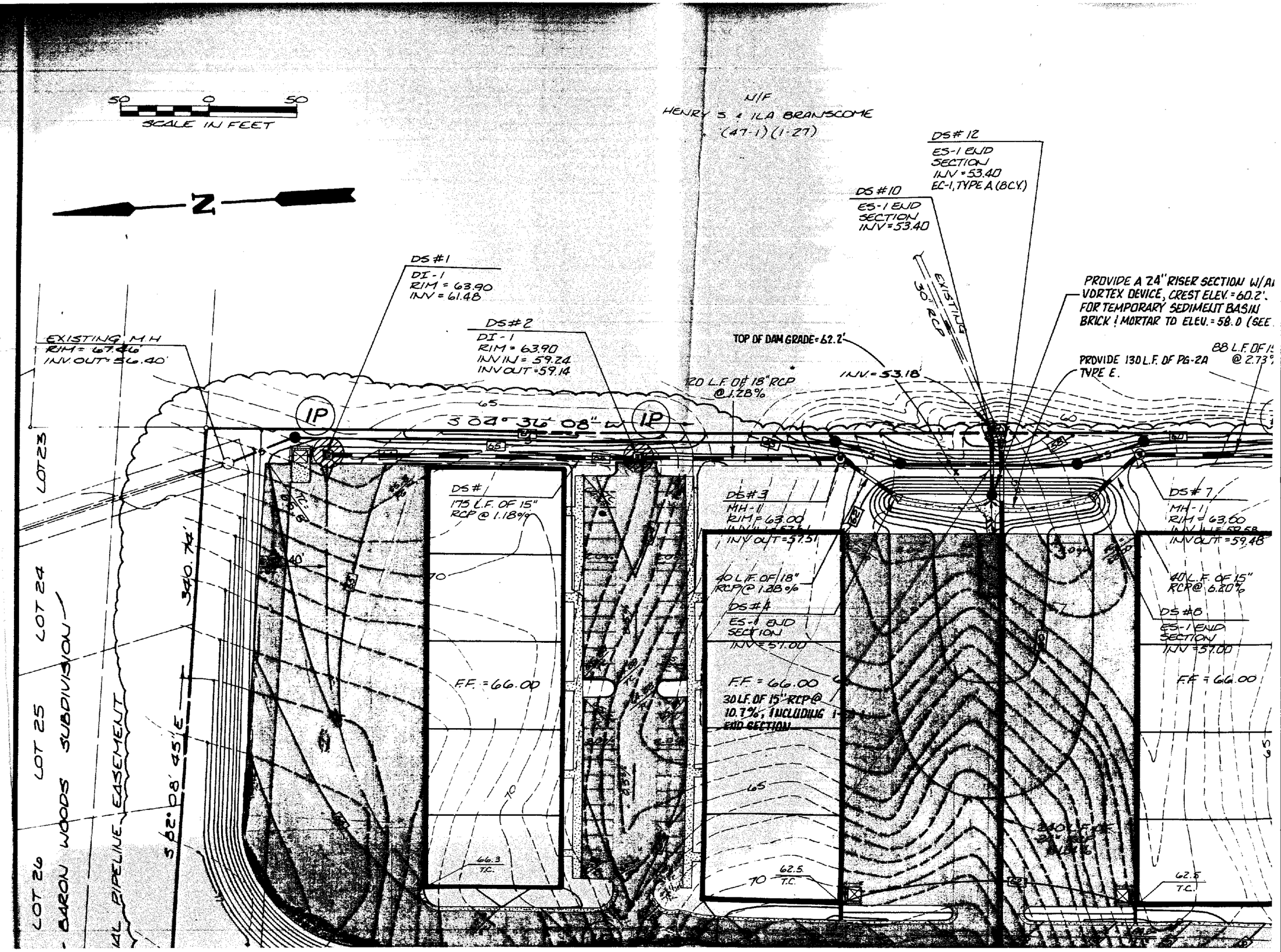
PROPOSED 2 STORY OFFICE BLDG. (6,000 S.F.)

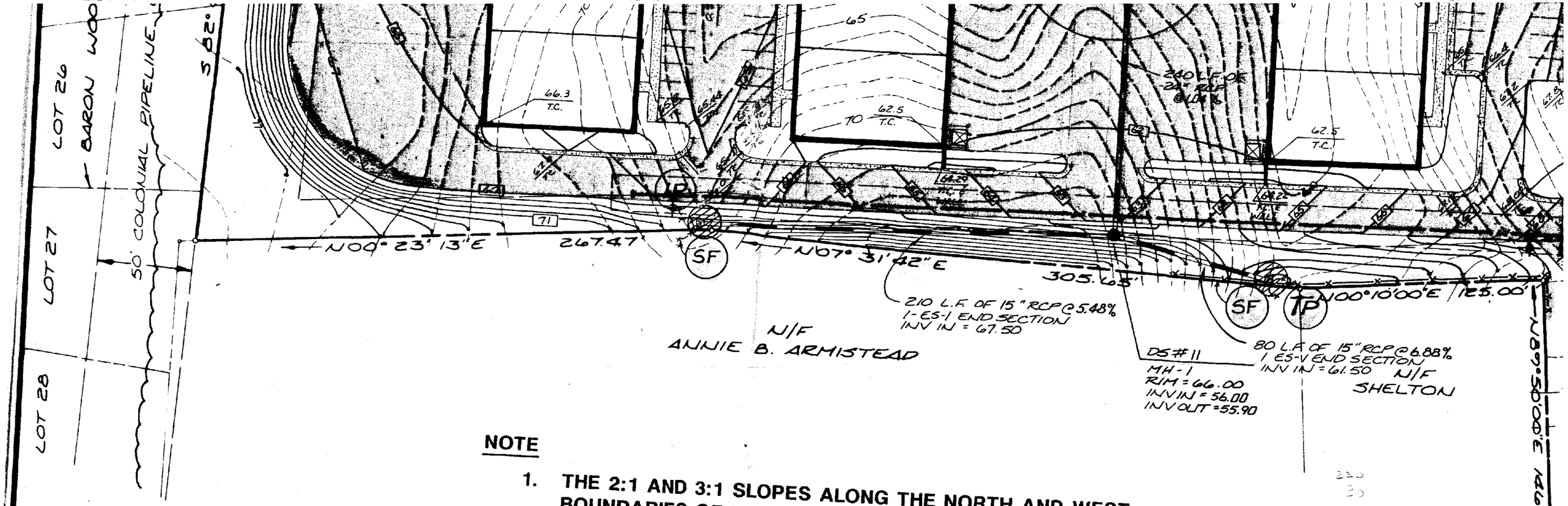
CORNER 38" PINE


EXISTING PVC WA
1-8" PLU
NOTE: SEE SHEET#6 FO
LANDSCAPE DETA
EXISTING 30" RCP
INV.=53.18'
10'x20' DRY DUMPSTER W/8' WOOD SCREEN FENCE (SEE DETAIL)
S 04° 36' 08" W
CONCRETE DITCH
888.01'
90' MIN.
50' TYPICAL
50' REAR SETBACK
35' BUFFER
S 82° 08' 45" E
8' WOOD FENCE
PROPOSED WAREHOUSE F.F.=66.0' 17,000 S.F. (WAREHOUSE) 3,000 S.F. (OFFICE SPACE)
OFFICE SPACE
HANDICAP RAMP
5' CONCRETE SIDEWALK (TYPICAL)
PROPOSED WAREHOUSE F.F.=66.0' 14,688 S.F. (WAREHOUSE) 2592 S.F. (OFFICE SPACE)
10'x20' DRY DUMPSTER W/8' WOOD SCREEN FENCE (SEE DETAIL)
HANDICAP RAMP
PROVIDE CONCRETE BUMPER BLOCKS ALONG EDGE OF PAVEMENT.
150W H.P.S. LUMINARE ON 20' POLE (TYPICAL)
PROPOSED WAREHOUSE F.F.=66.0' 14,688 S.F. (WAREHOUSE) 2592 S.F. (OFFICE SPACE)
HANDICAP RAMP
10'x20' DRY DUMPSTER W/8' WOOD SCREEN FENCE (SEE DETAIL)
PROPOSED OFFICE SPACE F.F.=68.0' 14000 S.F. OFFICE SPACE
HANDICAP RAMP
N00° 23' 13" E
267.47'
N07° 31' 42" E
305.65'
N00° 10' 00" E
125.00'
20' UTILITIES EASEMENT TO BE DEDICATED TO J.C.S.A.
20' SIDE SETBACK (TYPICAL)
20'x20' DRY DUMPSTERS W/ 8' WOOD SCREEN FENCE
N/F
PROPOSED


N/F
HENRY S & ILA BRANSCOME
(47-1)(1-27)
BMP
MC011
ROUTE 5
EXISTING 60' OF 8" PVC WATERLINE 1-8" PLUG
DS # 12
ES-1 END SECTION
INV = 53.40
EC-1, TYPE A (B.C.Y.)
DS # 10
ES-1 END SECTION
INV = 53.40
EXISTING 30" RCP
PROVIDE A 24" RISER SECTION W/ ANTI-VORTEX DEVICE, CREST ELEV. = 60.2'. FOR TEMPORARY SEDIMENT BASIN BRICK & MORTAR TO ELEV. = 58.0 (SEE DETAIL)
DS #1
DI-1
RIM = 63.90
INV = 61.48
DS #2
DI-1
RIM = 63.90
INV IN = 59.24
INV OUT = 59.14
TOP OF DAM GRADE = 62.2'
INV. = 53.18
120 L.F. OF 18" RCP @ 1.28%
PROVIDE 130 L.F. OF PG-2A TYPE E.
88 L.F. OF 15" RCP @ 2.73%
DS #6
DI-1
RIM = 64.50
INV IN = 61.98
INV OUT = 61.88
DS #5
DI-1
RIM = 66.33
INV OUT = 63.91
176 L.F. OF 15" RCP @ 1.04%
S 02° 36' 08" W
IP
S 88° 01'
DS #
175 L.F. OF 15" RCP @ 1.18%
F.F. = 66.00
66.3
T.C.
DS # 3
MH-1
RIM = 63.00
INV OUT = 57.51
40 L.F. OF 18" RCP @ 1.28%
DS # 4
ES-1 END SECTION
INV = 57.00
F.F. = 66.00
30 L.F. OF 15" RCP @ 10.7%, INCLUDING 1-END SECTION
62.5
T.C.
DS # 7
MH-1
RIM = 63.60
INV OUT = 59.48
40 L.F. OF 15" RCP @ 6.20%
DS # 8
ES-1 END SECTION
INV = 57.00
F.F. = 66.00
62.5
T.C.
F.F. = 68.00
INGRAM

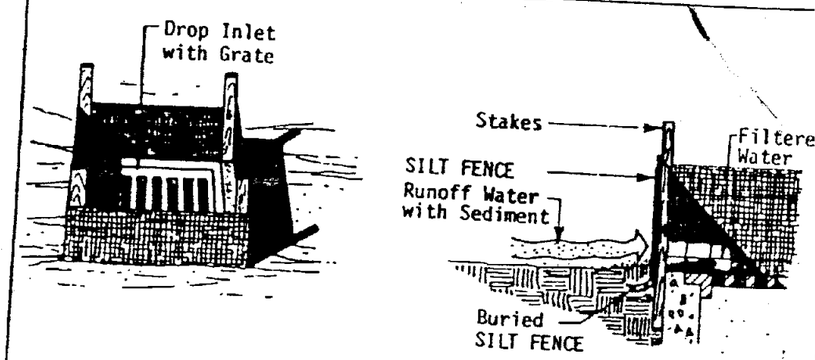

EXISTING FIRE HYDRANT
N/F
GEORGE D. & HEL.
WOODSON
(47-1)(1-45)
ROUTE 5
N/F
S. & ILA BRANSCOME
(47-1)(1-27)
DS # 12
ES-1 END
SECTION
INV = 53.40
EC-1, TYPE A (B.C.Y.)
DS # 10
ES-1 END
SECTION
INV = 53.40
EXISTING 30" RCP
PROVIDE A 24" RISER SECTION W/ ANTI-VORTEX DEVICE, CREST ELEV. = 60.2'. FOR TEMPORARY SEDIMENT BASIN BRICK & MORTAR TO ELEV. = 58.0 (SEE DETAIL)
TOP OF DAM GRADE = 62.2'
INV. = 53.18'
F. OF 18" RCP @ 1.28%
PROVIDE 130 L.F. OF PG-2A TYPE E.
88 L.F. OF 15" RCP @ 2.73%
DS # 6
DI - 1
RIM = 64.50
INV IN = 61.98
INV OUT = 61.88
DS # 5
DI - 1
RIM = 66.33
INV OUT = 63.91
176 L.F. OF 15" RCP @ 1.04%
888.01'
IP
IP
EXISTING 60' OF 8" PVC WATERLINE 1-8" PLUG
N/F
HENRY S. BRANSCOME
CONTRACTOR
EXISTING 12" WATER LINE
EXISTING FIRE HYDRANT
N 84° 17' 38" W 275.36
DS # 3
MH - 1
RIM = 63.00
INV OUT = 57.51
40 L.F. OF 18" RCP @ 1.28%
DS # 4
ES-1 END
SECTION
INV = 57.00
F.F. = 66.00
30 L.F. OF 15" RCP @ 10.7%, INCLUDING 1-ES-1 END SECTION
DS # 7
MH - 1
RIM = 63.60
INV OUT = 59.48
40 L.F. OF 15" RCP @ 6.20%
DS # 8
ES-1 END
SECTION
INV = 59.00
F.F. = 66.00
F.F. = 68.00
INV. OUT = 68.88
N


50 0 50
SCALE IN FEET
N
N/F
HENRY S. & ILA BRANSCOME
(47-1)(1-27)
DS # 12
ES-1 END
SECTION
INV = 53.40
EC-1, TYPE A (B.C.Y.)
DS # 10
ES-1 END
SECTION
INV = 53.40
DS # 1
DI-1
RIM = 63.90
INV = 61.48
DS # 2
DI-1
RIM = 63.90
INV IN = 59.24
INV OUT = 59.14
EXISTING M.H.
RIM = 67.46'
INV OUT = 56.40'
TOP OF DAM GRADE = 62.2'
INV. = 53.18'
20 L.F. OF 18" RCP
@ 1.28%
EXISTING
30" RCP
PROVIDE A 24" RISER SECTION W/A
VORTEX DEVICE, CREST ELEV. = 60.2'.
FOR TEMPORARY SEDIMENT BASIN
BRICK & MORTAR TO ELEV. = 58.0 (SEE
PROVIDE 130 L.F. OF PG-2A
TYPE E.
88 L.F. OF
@ 2.73
S 04° 36' 08" W
IP
IP
LOT 23
LOT 24
LOT 25
LOT 26
BARON WOODS SUBDIVISION
EASEMENT
PIPELINE
S 82° 08' 45" E
340.74'
DS #
175 L.F. OF 15"
RCP @ 1.18%
F.F. = 66.00
66.3
T.C.
DS # 3
MH-1
RIM = 63.00
INV OUT = 57.51
40 L.F. OF 18"
RCP @ 1.28%
DS # 4
ES-1 END
SECTION
INV = 57.00
F.F. = 66.00
30 L.F. OF 15" RCP @
10.7%, INCLUDING
END SECTION
62.5
T.C.
DS # 7
MH-1
RIM = 63.60
INV OUT = 59.48
40 L.F. OF 15"
RCP @ 6.20%
DS # 8
ES-1 END
SECTION
INV = 57.00
F.F. = 66.00
62.5
T.C.



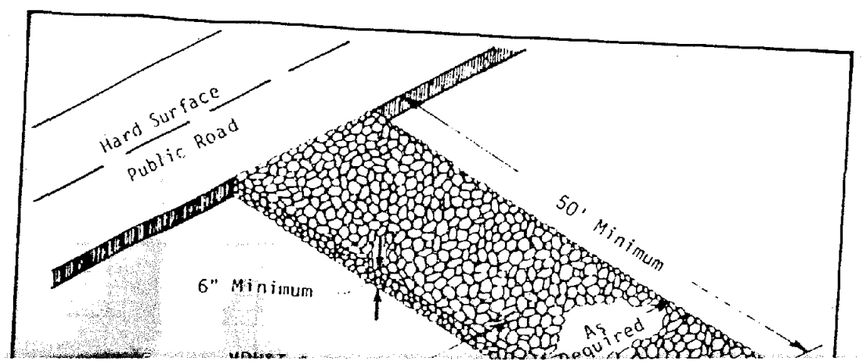
NOTE

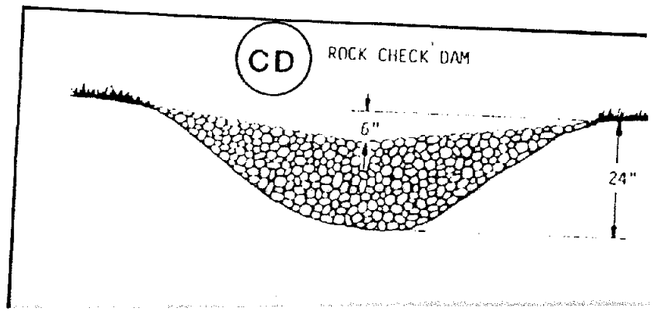
1. THE 2:1 AND 3:1 SLOPES ALONG THE NORTH AND WEST BOUNDARIES OF THE PROJECT SHALL BE STABILIZED WITH EC-2 MATERIAL (MIRAMAT OF APPROVED EQUAL) IMMEDIATELY AFTER FINAL GRADE IS ESTABLISHED.
2. ALL AREAS TO BE PAVED SHALL BE STABILIZED BY GRAVEL IMMEDIATELY AFTER FINAL GRADE IS ESTABLISHED.

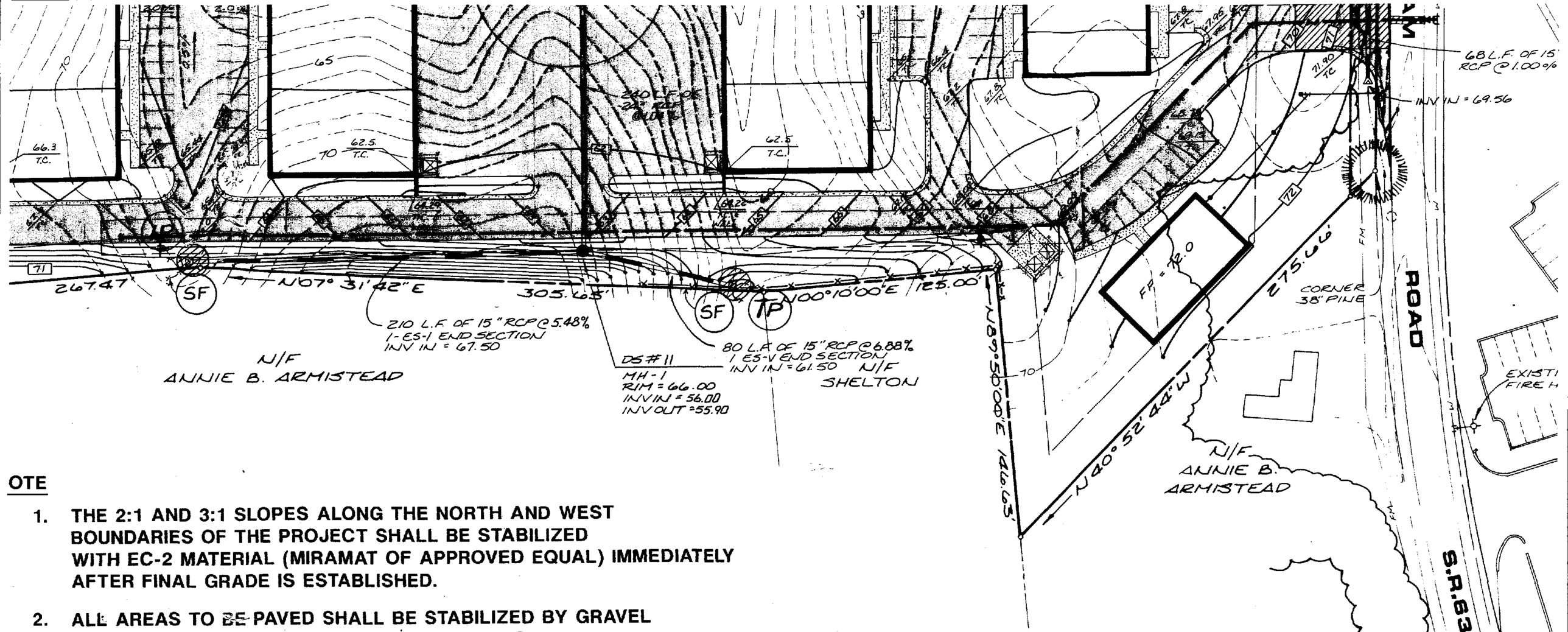


Specific Application

This method of inlet protection is applicable where the inlet drains a relatively flat area (slopes no greater than 5 percent) where sheet or overland flows (not exceeding 0.5 cfs) are typical.







OTE

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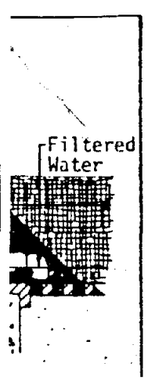
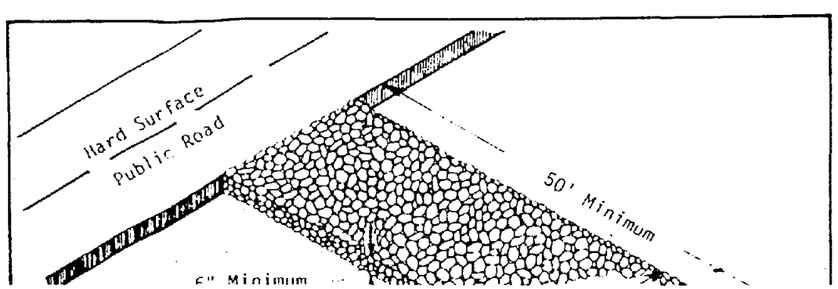
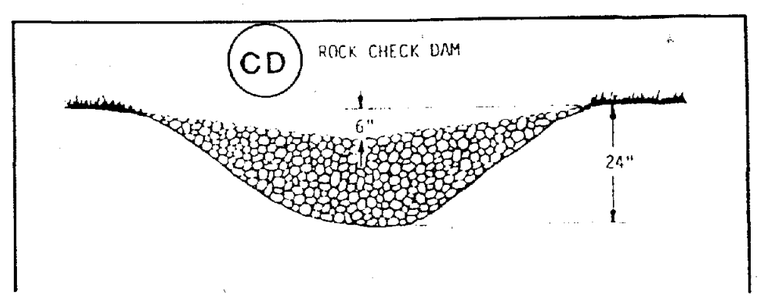
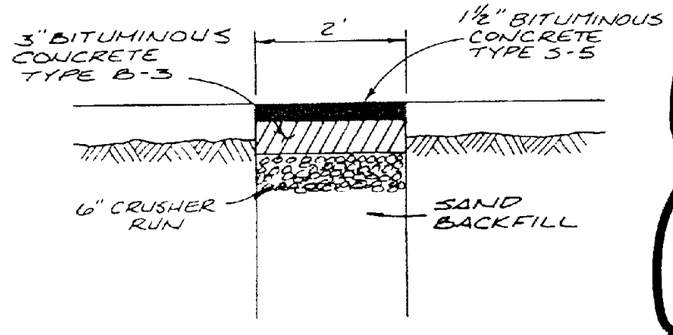


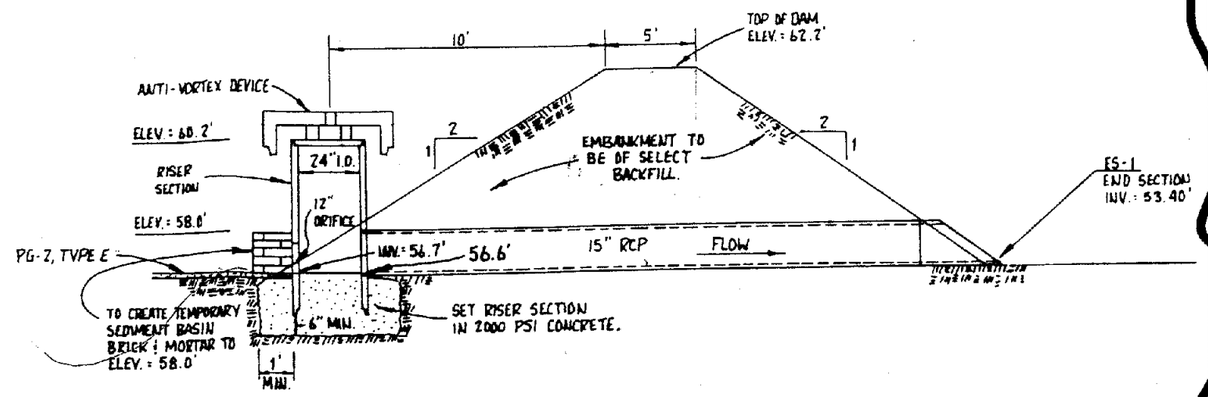




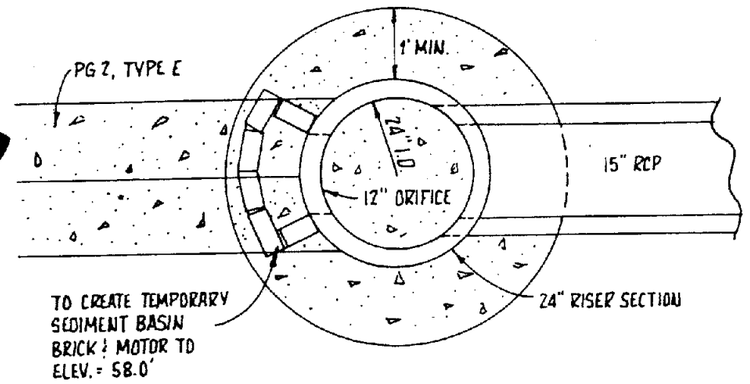


OPEN CUT RESTORATION DETAIL

N.T.S.

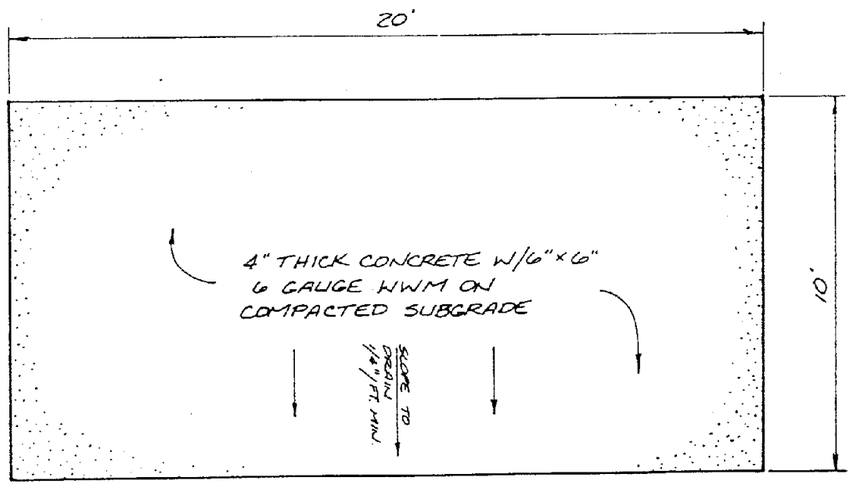


CROSS SECTION



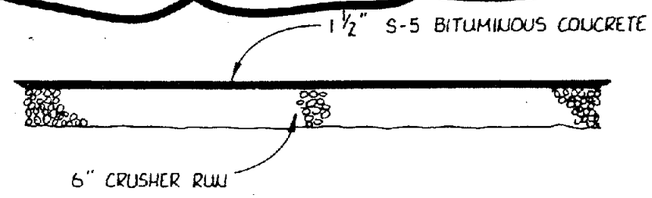
PLAN VIEW

SEDIMENT BASIN DETAIL (NTS)

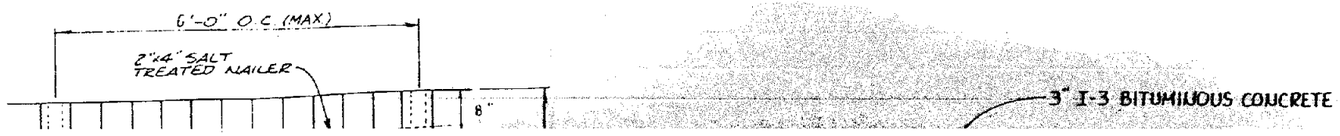


DUMPSTER PAD DETAIL

N.T.S.



PARKING AREA



No.	DATE	REVISION / COMMENT / NOTE	BY
4	8/3/90	REVISED PER JAMES CITY COUNTY COMMENTS	SOW
3	5/12/89	REVISED SAN. SEWER TO A PRIVATE SYSTEM, PER JCSA	SOW
2	4/25/89	REVISED PER JAMES CITY COUNTY COMMENTS	SOW
1	2/10/89	REVISED PER JAMES CITY COUNTY COMMENTS	SOW

COMMONWEALTH OF VIRGINIA
STEVEN O. WIGLEY
No. 019307
PROFESSIONAL ENGINEER

fessional corporation
amestown Road
urg, Virginia 23185
-253-0040
, Planning, Surveying

Erosion and Sediment Control Notes

The purpose of the erosion control measures shown on these plans shall be to preclude the transport of all waterborne sediments resulting from construction activities from entering onto adjacent properties or State waters. If field inspection reveals the inadequacy of the plan to confine sediment to the project site, appropriate modifications will be made to correct any plan deficiencies.

1. All erosion and sediment control measures shall be installed and maintained in accordance with the "Virginia Erosion and Sediment Control Handbook". The contractor shall be thoroughly familiar with all applicable measures contained therein which may be pertinent to this project.

2. All points of construction ingress and egress shall be protected by a temporary construction entrance to prevent tracking of mud onto public right-of-ways. An entrance permit from VDH&T is required prior to any construction activities within State right-of-ways.

3. Sediment basins and traps, perimeter dikes, sediment barriers and other measures intended to trap sediment on-site must be constructed as a first step in grading and be made functional before upslope land disturbance takes place. Earthen structures such as dams, dikes, and diversions must be seeded and mulched within 15 days of installation. An on-site pre-construction meeting will be held between the Department of Public Works and the contractor to identify those measures to be initially installed.

4. Maintenance of all erosion and sediment control measures shall be accomplished in accordance with the "Virginia Erosion and Sediment Control Handbook". Maintenance will include the repair of measures damaged by any subcontractor including those of the public utility companies. At the pre-construction meeting, the contractor will supply Public Works with the name of the individual who will be responsible for ensuring maintenance of installed measures on a daily basis.

5. Surface flows over cut and fill slopes shall be controlled by either redirecting flows from transversing the slopes or by installing mechanical devices to safely lower water downslope without causing erosion. A temporary fill diversion (Std. & Spec. 1.16) shall be installed prior to the end of each working day.

6. Sediment control measures may require minor field adjustments at time of construction to insure their intended purpose is accomplished. Department of Public Works approval will be required for other deviations from the approved plans.

7. The contractor shall strip and pile topsoil at the locations shown on this plan or as directed by the engineer. Silt fence shall be placed at the toe of the stockpile after stripping of topsoil is complete.

8. The contractor shall complete drainage facilities within 30 days following completion of rough grading at any point within the project. The installation of drainage facilities shall take precedence over all underground utilities. Outfall ditches from drainage structures shall be stabilized immediately after construction of same. This includes installation of erosion control stone where required. Any drainage outfalls required for a street must be completed before street grading begins.

9. Permanent or temporary soil stabilization must be applied to all denuded areas within 15 days after final grade is reached on any portion of the site. Soil stabilization must also be applied to denuded areas which may not be at final grade but will remain dormant (undisturbed) for longer than 30 days. Soil stabilization measures include vegetative establishment, mulching and the early application of gravel base material on areas to be paved.

10. No more than 300' of sanitary sewer, storm sewer, or waterlines are to be open at one time. Following installation of any portion of these items, all disturbed areas are to be immediately stabilized (i.e., the same day).

11. If disturbed area stabilization is to be accomplished during the months of December, January, or February, stabilization shall consist of mulching in accordance with Specification 1.75. Seeding will then take place as soon as the season permits.

12. The term Seeding, Final Vegetative Cover or Stabilization, on this plan shall mean the successful germination and establishment of a stable grass cover from a properly prepared seedbed containing the specified amounts of seed, lime, and fertilizer in accordance with Specification 1.66, Permanent Seeding. Irrigation shall be required as necessary to ensure establishment of grass cover.

13. All slopes steeper than 3:1 shall require the use of erosion control blankets such as excelsior blankets to aid in the establishment of a vegetative cover. Installation shall be in accordance with Specification 1.75, Mulching and Manufacturer's Instructions.

14. Inlet protection in accordance with Specification 1.08 shall be provided for all storm drain inlets as soon as practical following construction of same.

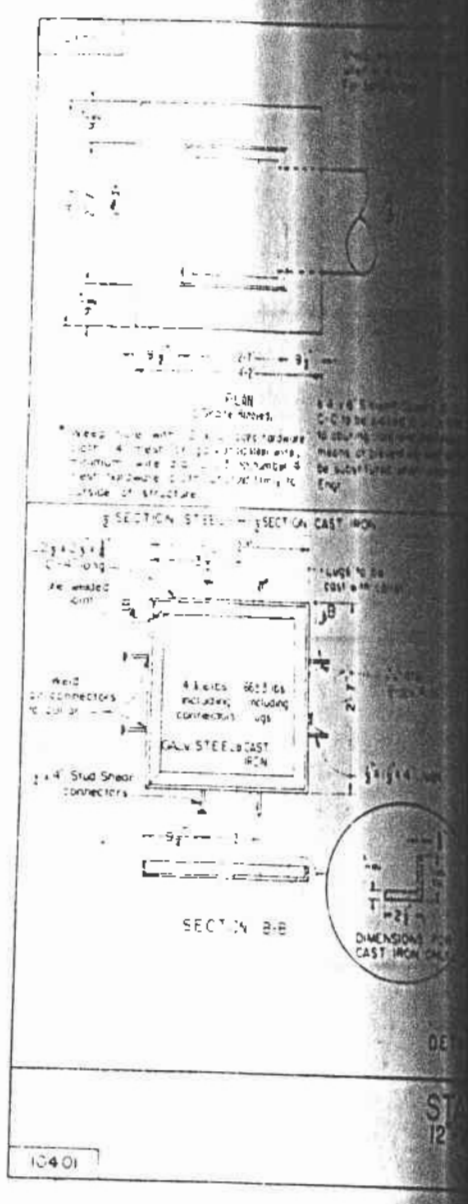
15. Temporary liners, such as polyethylene sheets, shall be provided for all paved ditches until the permanent concrete liner is installed.

16. Paved ditches shall be required wherever erosion is evident. Particular attention shall be paid to those areas where grades exceed 3%.

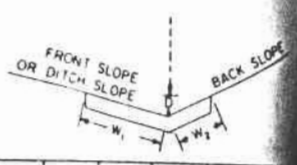
17. Temporary erosion control measures are not to be removed until all disturbed areas are stabilized. After stabilization is complete, all measures shall be removed within 30 days. Trapped sediment shall be spread and seeded.

18. Off-site waste or borrow areas shall be approved by James City County prior to the import of any borrow or export of any waste to or from the project site.

19. All paved and/or piped outfalls will be constructed before road grading and utility installation begins.

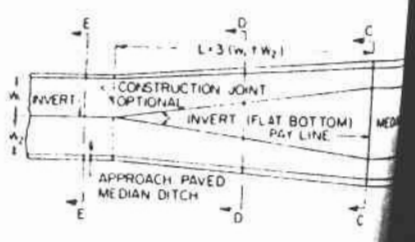


OUTSIDE ROAD DITCHES



TYPE	D	FRONT SLOPE	BACK SLOPE	W_1	W_2
A1	6"	6 1	4 1	3'-0"	2'-1"
	8"	6 1	4 1	4'-1"	2'-9"
A2	6"	6 1	3 1	3'-0"	1'-7"
	8"	6 1	3 1	4'-1"	2'-1"
A3	6"	6 1	2 1	3'-0"	1'-2"
	8"	6 1	2 1	4'-1"	1'-6"
B1	6"	4 1	4 1	2'-1"	2'-1"
	8"	4 1	4 1	2'-9"	2'-9"
B2	8"	4	3 1	2'-9"	2'-1"
	10"	1	3 1	3'-5"	2'-8"
B3	8"	4 1	2 1	2'-9"	1'-6"
	10"	4	2 1	3'-5"	1'-10"
B4	8"	4	1 1/2 1	2'-9"	1'-2"
	10"	4 1	1 1/2 1	3'-5"	1'-6"
C1	8"	3 1	2 1	2'-1"	1'-6
	10"	3 1	2 1	2'-8"	1'-6
C2	10"	1	1 1/2 1	2'-8"	1'-6
C3	10"	1	1 1	2'-8"	1'-7

PLAN FOR TRANSITIO



SPECIFICATION REFERENCE
502

SHEET 1

SITE & LANDSCAPING PLAN

DIGGES, BROTHERS OFFICE/WAREHOUSES

OWNER/DEVELOPER: T.R. O'ROURK

JAMES CITY COUNTY VIRGINIA

TAX PARCEL (47-1)(1-7) PERMIT # 92-21

NOTE: This plan is an amendment to SP-62-88 for the following reasons:

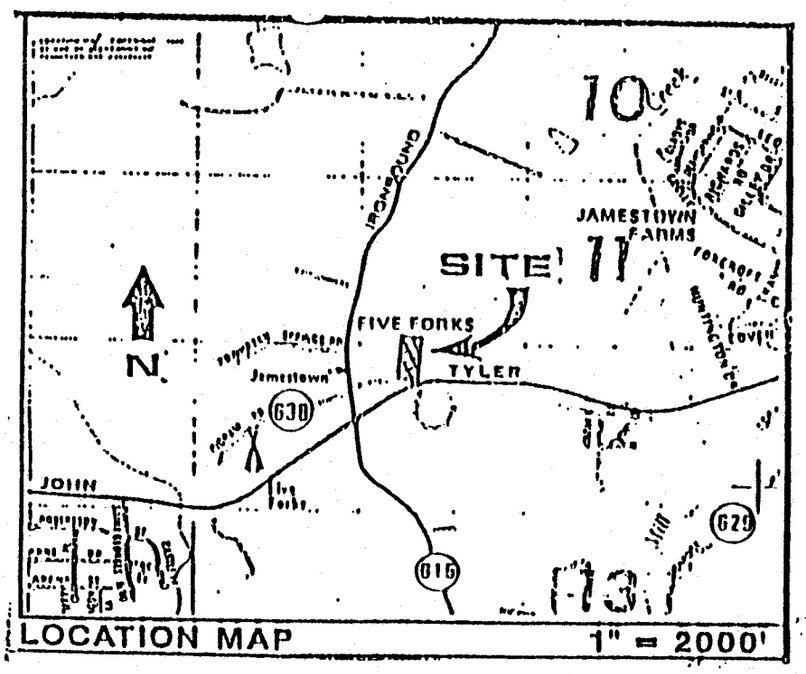
1. Move the handicap ramp to be near the center of the parking lots.
2. All exterior light fixtures are to be moved and attached to the building and to be on different circuits
3. Dumpster pads are to be located behind individual units as required by various occupant's uses.
4. Change foundation planting to accommodate various wide door ramps, A/C pads and gas meters.
5. Move the parking spaces to the north side of building #3

ZONED - B1

LAND USE DATA

	S.F.	AC.	%
LOT AREA	298,316	6.8484	100%
BUILDING COVERAGE	74,560	1.7117	25%
PAVEMENT	124,632	2.8612	42%
GREEN AREA	99,124	2.2755	33%
R/W DEDICATION	5,765	0.1323	—
	304,081	6.9807	

PARKING	REQUIRED	PROVIDED	
28,184 S.F. OFFICE SPACE	113	127	REGULAR 18' X 9'
		9	H.C. @ 18' X 12'
TOTAL		136	



COUNTY OF JAMES CITY
FINAL SITE PLAN

Received .....

APPROVALS	DATE
Fire Dept.	
Health Dept.	
Highway Dept.	
Plan. Comm.	7/8/97
Pub. Wks. Dept.	
Zoning Adm.	7/8/97
JCSA	
Other	
Other	

RECEIVED
JUN 1997
PLANNING DEPARTMENT

SP-67-97

SHEET II

40 G

35' BUFFER

LIGHT LUMEN CURB

42'

PROPOSED BLDG III

TRUCK LOADING AREA

58'

LIGHT LUMEN CURB

42'

UNITS 9-16

BLDG. II

REVISED LOADING DOCK

SEDIMENT POND

TAX PARCEL (47-1) (1-7)
ZONED B-1

PERMIT #92-21
SCALE 1" = 30'

PLANT LIST

MARK	COMMON NAME	SIZE
A	Red Maple	6' to 8' High Container Grown
B	Yellow Pine	4' to 6' High Container Grown
C	Yaupon Holly	2 Gallon Pots Min. 18" High
D	Foster Holly	2 Gallon Pots Min. 18" High
E	Forsythia	2 Gallon Pots Min. 18" High
G	Leland Cypress	6' B&B

PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY:

- ALL REQUIRED LANDSCAPING MUST BE INSTALLED
- ALL HANDICAPPED PARKING IMPROVEMENTS MUST BE INSTALLED
- ALL TRASH FACILITIES MUST BE SCREENED BY FENCING AND/OR LANDSCAPING
- ANY FURTHER CHANGES TO THIS PLAN WILL REQUIRE A SITE PLAN AMENDMENT

UNITS 1-8

BLDG. I

108 INGRAM RD.

MC 011
SP-62-88

DIGGES BROS WHSE

1. Pre Dev.

1. TOTAL D_A = 5.71 ac + 6.15 + 6.9 = 18.74 ac

T_c = 25 min

C = .3

Q_2 = 3.0 x .3 x 18.74 = 16.9 cfs

Q_{10} = 4.02 x .3 x 18.74 = 22.6 cfs

2. Post Devel

C = .65 x 6.9 + .3 x 11.86 = 8.04 / 18.74 = .43

T_c = 25 min

Q_2 = 3 x .43 x 18.74 = 24.17 cfs

Q_{10} = 4.02 x .43 x 18.74 = 32.4 cfs

3. 2-yr Design

C_D = .43

D_A = 18.74

t_c = 25 min

q_o = 16.9 cfs

Q_o = 26.91

T_c = 20.5 min

V_s = 13,800 ft^3 = .32 ac ft.

4. 10 yr design

q_o = 22.6

T_c = 24 min

Q_o = 33.0

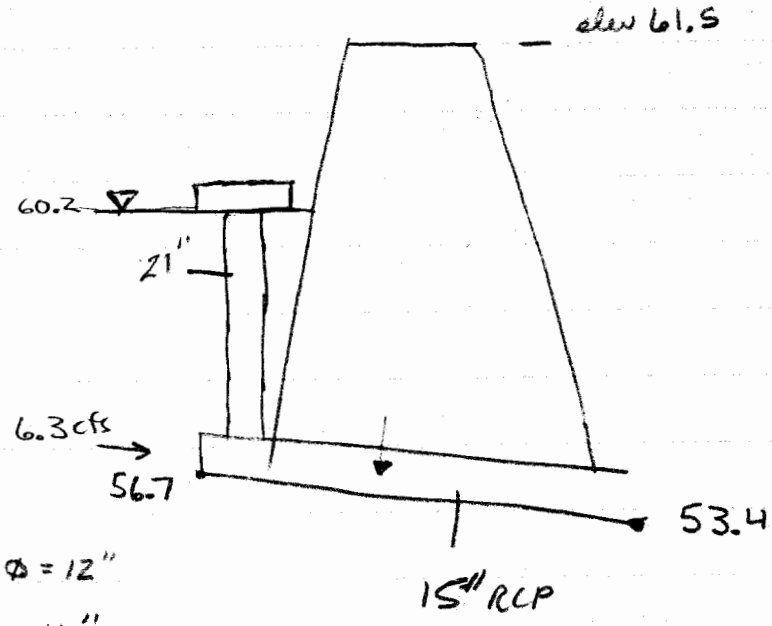
V_s = 18,300 ft^3 = .42 ac ft

Diggers Whse

5. $Q_{2OUT} = 6.3$ cfs

– elev 60.3'

inv. out =



$Q_o = CA\sqrt{2gH}$

$A = \dfrac{Q_o}{C\sqrt{2gH}}$

$= \dfrac{6.3}{.62\sqrt{2\times32.2\times4}}$

Assume $\phi = 12''$

$A = .633\ ft^2 \quad \phi = .9' = 11''$

for $\phi = 12'' \quad A = .785$

$Q_o = .62\times\sqrt{64.4\times4}\times.785 = 7.8$ cfs

$h = 3' \quad Q_o = .62\times.785\times\sqrt{64.4\times3} = 6.7$ cfs ✓

6. $Q_{10} = 9.7$ cfs

$Q_{orifice} = .62\times.785\sqrt{2\times32.2\times4.5}$

$= 8.3$ cfs

$Q_{riser} = 9.7 - 8.3 = 1.4$ cfs

$Q_{15''\,riser} = 4$ cfs @ 0.5' head

15" barrel = $H = 7' \quad Q_o = 1.18\times14.4 = 17$ cfs

12" Barrel = $\cancel{2}\times1.22\times8.5 = 10.4$ cfs

7. $Q_{25} = .65\times6.9\times6''/hr = 26.9$ cfs $\qquad Q_o = 15.4\times1.18 = 18.2$ cfs

$\dfrac{Q_o}{Q_c} = \dfrac{18.2}{26.9\text{ cfs}} = 0.676$

$\dfrac{V_s}{V_r} = .22 \qquad V_r = 5.4''\times6.9\text{ ac}\times\frac{1}{12} = 3.06$ ac ft

$V_s = .673$ ac ft $= 29{,}325\ ft^3$

Detention Pond Calc.s

Code Compliance

Pre-development

A = 6.88 Ac. — Study
C = 0.30 (Woodlands)

L = 600 ft. ΔElev. = 78 - 53.18 = 24.82 Slope = 4.14%

T_c = 24 min ⇒ I_2 = 3.07 6.34 cfs
I_{10} = 4.10 8.46 cfs

I_2 = 3.8 in/hr. , Q_2 = 7.84 cfs
I_{10} = 4.7 in/hr. , Q_{10} = 9.70 cfs

Post-development

Composite "C" Value

	A	C	CA
Pvmnt & Roof	4.75	0.85	4.04
Green Area	2.13	0.20	0.43
	6.88		4.47

C = 0.65

L = 420' ΔElev. = 72.5 - 64.3 = 8.2 S = 2%
T_c = 14 min

Richmond Constants
a_2 = 130.3 a_{10} = 189.2 a_{100} = 360.0
b_2 = 18.5 b_{10} = 22.1 b_{100} = 28.0

Critical Storm Duration For 2yr. Post-Dev

$$T_c = \sqrt{\frac{2CAa(b - t_c/4)}{q_o}} - b$$

given: C = 0.65
A = 6.88 Ac.
t_c = 14 min
a_2 = 130.3
b_2 = 18.5
q_o = 7.84 cfs

$$T_c = \sqrt{\frac{2(0.65)6.88(130.3)(18.5 - 14/4)}{7.84}} - 18.5$$

T_c = 28.7 min. ✓

for q_o = 6.34
Q_o = 11.1 cfs

Peak Inflow

$$Q_o = CA\left(\frac{a}{b + T_c}\right) = 0.65(6.88)\left(\frac{130.3}{18.5 + 28.7}\right) = 12.3 \text{ cfs}$$ ✓ V_s = 14,500 ft³ = .33 ac-ft

Required Storage For Critical 2yr. Storm

$$V = \left(Q_o T_c + \frac{Q_o t_c}{4} - \frac{q_o T_c}{2} - \frac{3 q_o t_c}{4}\right)60$$

$$V = \left(12.3(28.7) + \frac{12.3(14)}{4} - \frac{7.84(28.7)}{2} - \frac{3(7.84)14}{4}\right)60$$

V = 12074.2 ft³ = 0.28 Ac.-ft. ✓

Critical Storm Duration For 10yr Post Dev

$$T_c = \sqrt{\frac{2(0.65)(6.88)(189.2)/(22.1 + 14/2)}{9.70}} - 22.1$$

$= 34.9$ min ✓

given: $C = 0.65$
$A = 6.88$
$t_c = 14$ min.
$a_{10} = 189.2$
$b_{10} = 22.1$
$q_o = 9.70$ cfs

Peak Inflow

$$Q_o = 0.65(6.88)\left(\frac{189.2}{22.1 + 34.9}\right) = 14.8 \text{ cfs}$$ ✓

however for $q_o = 8.46$
$Q_o = 13.9$ cfs
$T_c = 38.9$ min
$V = 20{,}100$ ft^3
.46 ac ft

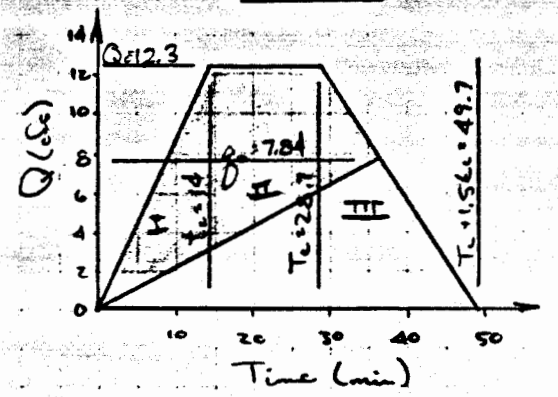
Required Storage for Critical 10yr Storm

$$V = \left(14.8(34.9) + \frac{14.8(14)}{4} - \frac{9.7(34.9)}{2} - \frac{3(9.70)14}{4}\right)60$$

$= 17832.3$ ft^3 $= 0.41$ Ac.-ft. ✓

check:

2YR.



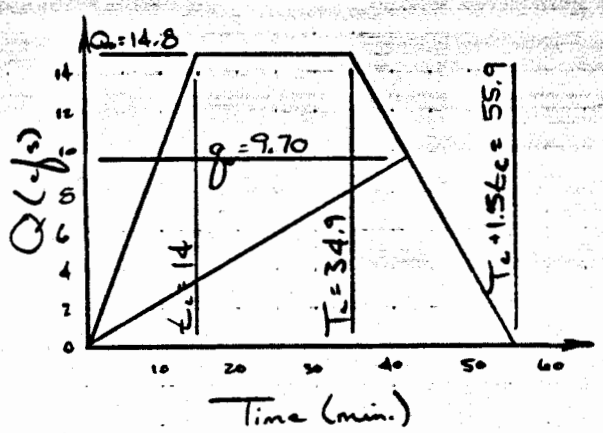
Area

$\frac{1}{2}(12.3)14(60) = 5166.0$
$12.3(14.7)60 = 10848.6$
$\frac{1}{2}(12.3)21(60) = 7749.0$
23763.6

$\frac{1}{2}(7.84)49.7(60) = -11689.4$

$V = 12074.2$ ft^3 ✓

10YR



Area

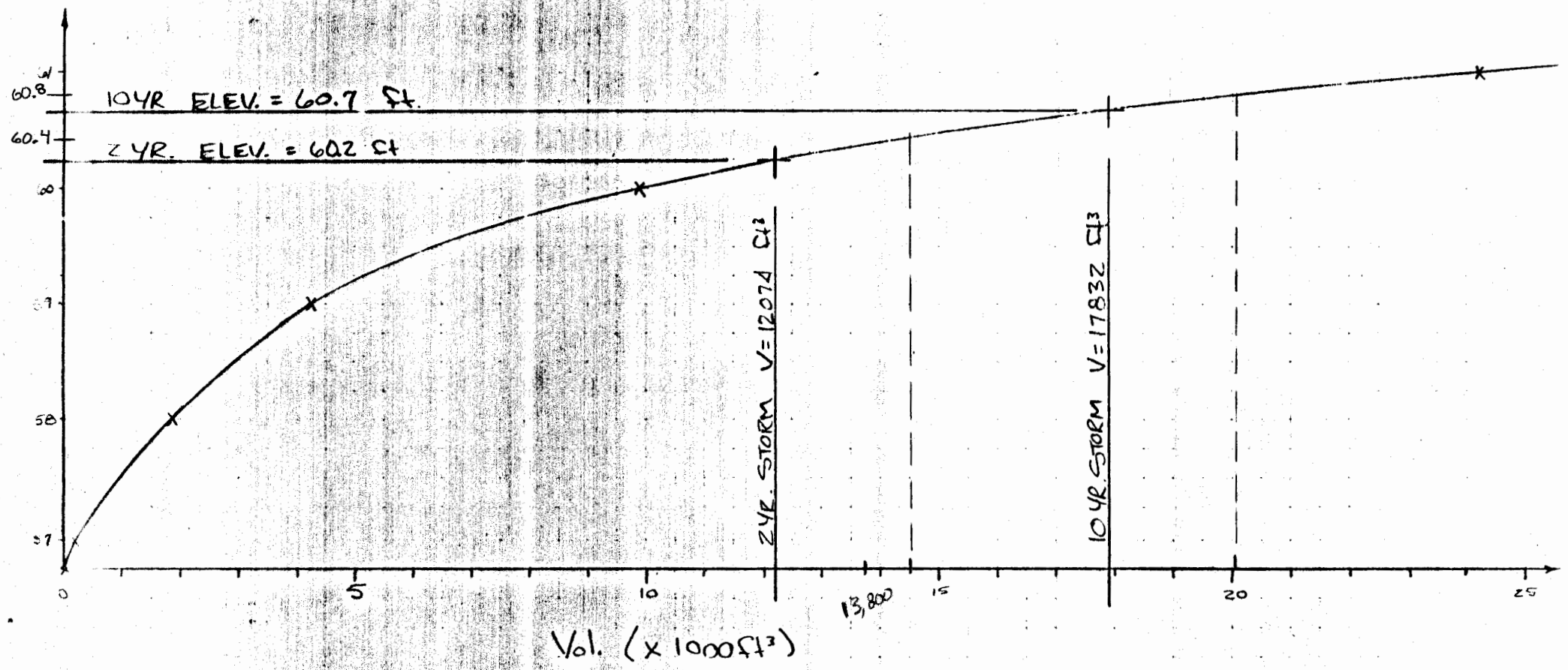
$\frac{1}{2}(14.8)14(60) = 6216.0$
$14.8(20.9)60 = 18559.2$
$\frac{1}{2}(14.8)21(60) = 9324.0$
34099.2

$\frac{1}{2}(9.7)55.9(60) = -16266.9$

$V = 17832.3$ ft^3 ✓

Elev.	Area	Avg. Area	Depth	Inc. Vol.	Vol.
56.7	0				0
		695.5	0.3	209	
57.0	1391				209
		1669	1	1669	
58.0	1947				1878
		2356	1	2356	
59.0	2765				4234
		5650.	1	5650	
60.0	8535				9884
		14332.	1	14332	
61.0	20129				24216

elev = 60.2 12,500 – Vs for Sed Basin



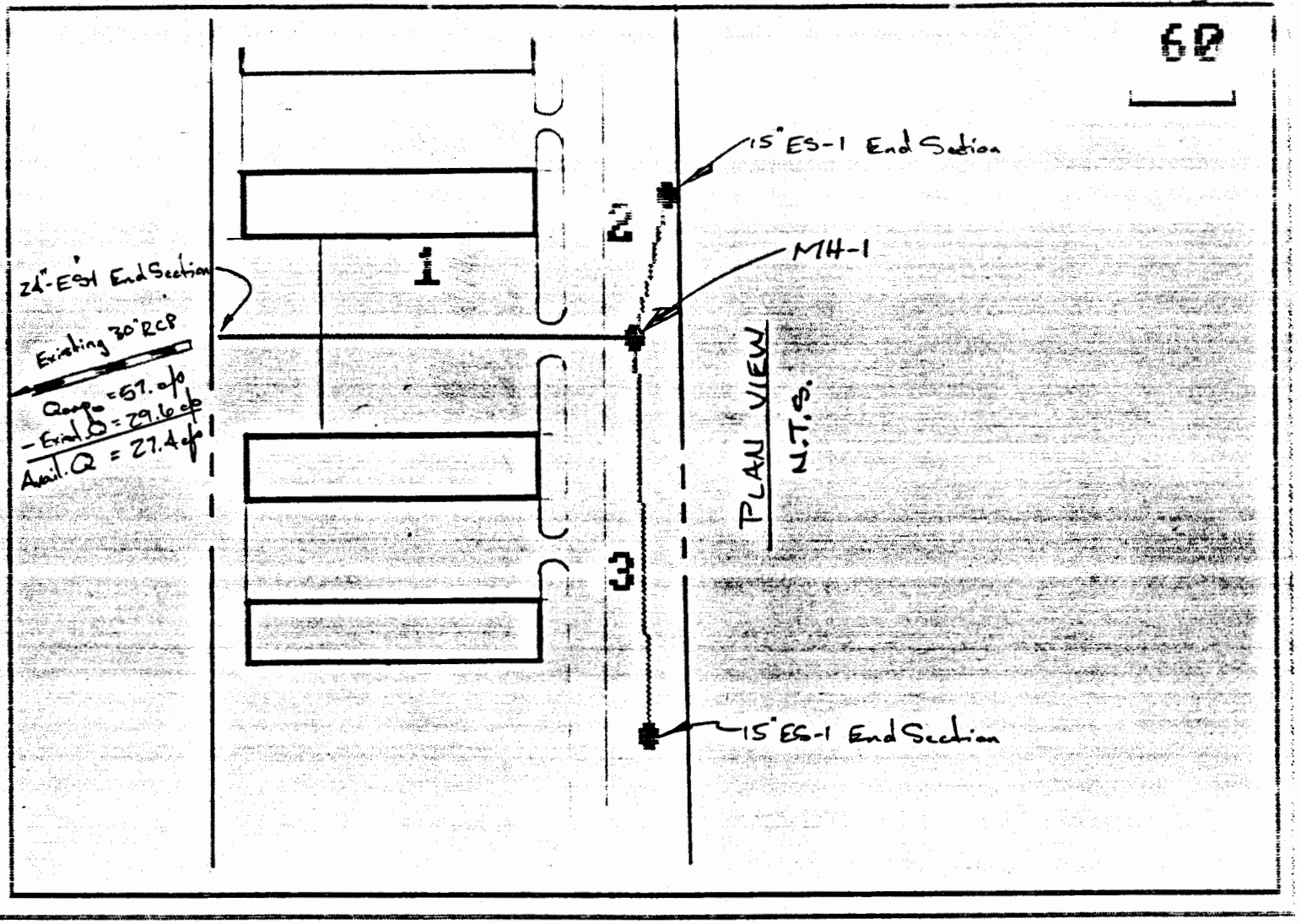
Outlet Structure

Outlet structure to be 36 L.F. of 12" RCP, including 2, ES-1 End Sections.

Barrel Flow (Plate 1.26b) Use a 1 " RCP

Inv. In ("Outlet Pipe) = 56.70
24YR. Elev. = 60.2' Q_{P2} = 7.84 cfs
10YR. Elev. = 60.7' Q_{P10} = 9.70 cfs

Storm	Head (ft.)	Q (cfs) 12" RCP		
24YR.	3.5	7.14	< 7.84	✓
10YR.	4.0	7.65	< 9.70	✓



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[PgUp] Enlarge [PgDn] Reduce [Esc] Exit

HYDRAULIC ANALYSIS

Run date: 02-13-1989
File:

Return Period = 10 Yrs
Rainfall file: JAMES CITY COUNTY

DS #11

LINE 1 / Q = 14.0 / HT = 24 / WID = 24 / N = .013 / L = 240 / JLC = 1.2

DIGS O..... / Outfall

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	54.82	16.99	53.40	5.88	55.35	21.82	-2	2.38
UPSTRM	57.32	17.05	55.90	5.86	57.85	21.77	8.09	2.39

Drainage area (ac) =	0	Slope of invert (%) =	1.042
Runoff coefficient =	0	Slope energy grade line (%) =	1.042
Time of conc (min) =	26	Critical depth (in) =	16.99
Inlet time (min) =	0	Req'd length curb inlet (ft) =	0.0
Intensity (in/hr) =	3.93	Req'd grate area (sf) =	0.0
Cumulative C*A =	3.6	Depth at inlet opening (in) =	0
Flow contrib (cfs) =	0	Confluence angle (deg) =	0
Default Q (cfs) =	0	Natural ground elev (ft) =	66
Line capac. (cfs) =	23.1	Line storage (cuft) =	572

LINE 2 / Q = 6.9 / HT = 15 / WID = 15 / N = .013 / L = 80 / JLC = .5

DIGS O..... / DNLN = 1

Right to DS #11

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	57.96	15.00	56.00	5.62	58.45	0.00	8.75	1.23
UPSTRM	62.88	13.06	61.50	6.08	63.45	10.08	-1.25	1.13

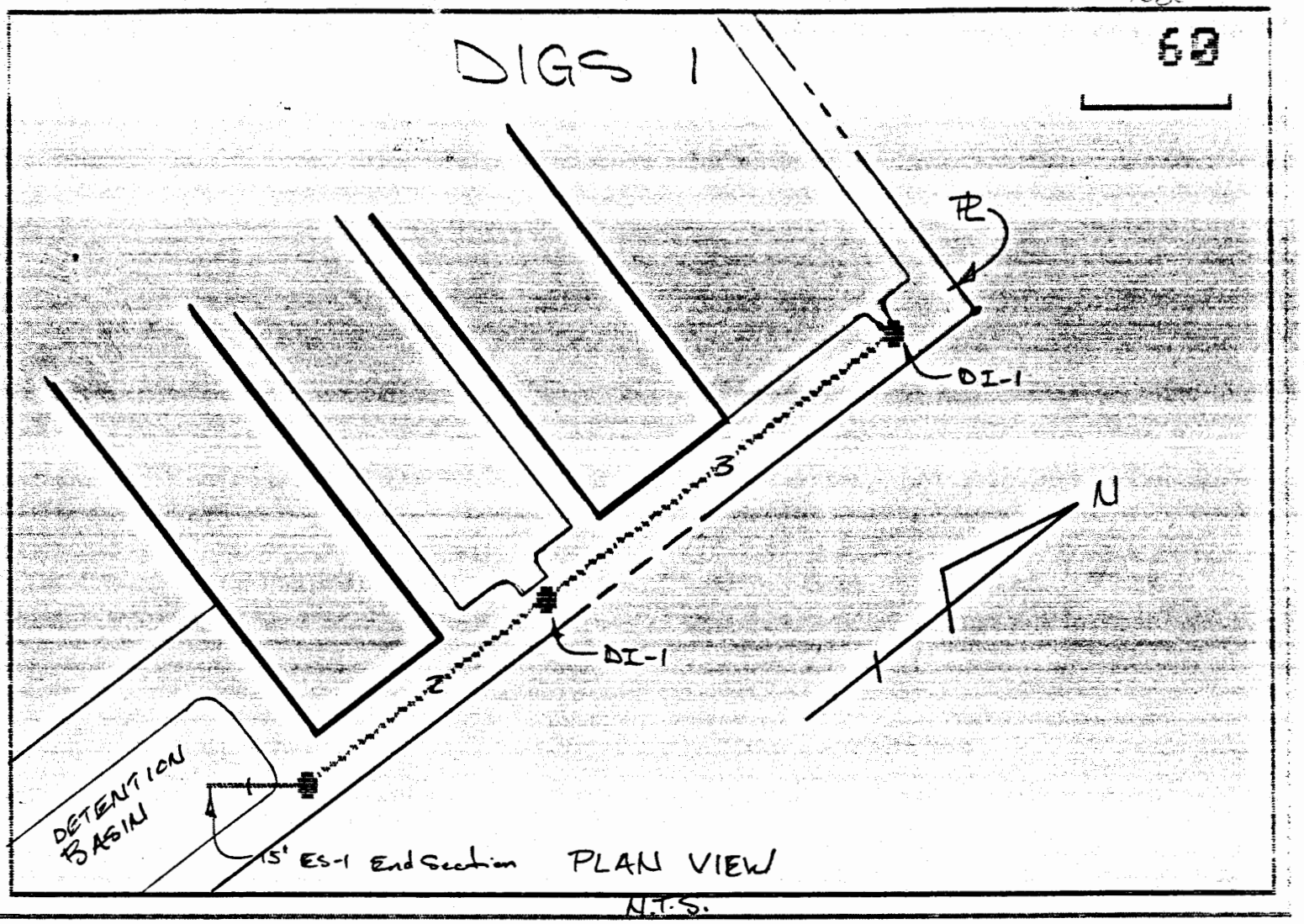
Drainage area (ac) =	5.71	Slope of invert (%) =	6.875
Runoff coefficient =	.3	Slope energy grade line (%) =	6.248
Time of conc (min) =	25	Critical depth (in) =	13.03
Inlet time (min) =	25	Req'd length curb inlet (ft) =	0.0
Intensity (in/hr) =	4.03	Req'd grate area (sf) =	0.0
Cumulative C*A =	1.7	Depth at inlet opening (in) =	0
Flow contrib (cfs) =	6.8	Confluence angle (deg) =	-77
Default Q (cfs) =	0	Natural ground elev (ft) =	61.5
Line capac. (cfs) =	16.9	Line storage (cuft) =	94

LINE 3 / Q = 7.4 / HT = 15 / WID = 15 / N = .013 / L = 210 / JLC = .5

DIGS 0..... / DNLN = 1 Left to PS #11

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	57.96	15.00	56.00	6.05	58.53	0.00	8.75	1.23
UPSTRM	68.93	13.30	67.50	6.46	69.58	9.51	-1.25	1.15

Drainage area (ac)	= 6.15	Slope of invert (%)	= 5.476
Runoff coefficient	= .3	Slope energy grade line (%)	= 5.108
Time of conc (min)	= 25	Critical depth (in)	= 13.28
Inlet time (min)	= 25	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 4.03	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.8	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 7.4	Confluence angle (deg)	= 88
Default Q (cfs)	= 0	Natural ground elev (ft)	= 67.5
Line capac. (cfs)	= 15.1	Line storage (cuft)	= 250



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HYDRAULIC ANALYSIS

Run date: 02-09-1989
File:

Return Period = 2 Yrs
Rainfall file: JAMES CITY COUNTY

LINE 1 / Q = 7.9 / HT = 15 / WID = 15 / N = .013 / L = 40 / JLC = 1.2

DIGS 1..... / Outfall

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	58.33	15.00	57.00	6.43	58.97	0.00	-1.25	1.23
UPSTRM	58.92	15.00	57.51	6.43	59.57	0.00	4.24	1.23

Drainage area (ac)	= 0	Slope of invert (%)	= 1.275
Runoff coefficient	= 0	Slope energy grade line (%)	= 1.492
Time of conc (min)	= 12	Critical depth (in)	= 13.51
Inlet time (min)	= 0	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 4.28	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.8	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 0	Confluence angle (deg)	= 0
Default Q (cfs)	= 0	Natural ground elev (ft)	= 63
Line capac. (cfs)	= 7.3	Line storage (cuft)	= 49

LINE 2 / Q = 8.1 / HT = 15 / WID = 15 / N = .013 / L = 120 / JLC = .9

DIG 1..... / DNLN = 1

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	59.69	15.00	57.61	6.59	60.37	0.00	4.13	1.23
UPSTRM	61.58	15.00	59.14	6.59	62.25	0.00	3.51	1.23

Drainage area (ac)	= 1.21	Slope of invert (%)	= 1.275
Runoff coefficient	= .7	Slope energy grade line (%)	= 1.568
Time of conc (min)	= 11	Critical depth (in)	= 13.73
Inlet time (min)	= 10	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 4.39	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.8	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 3.7	Confluence angle (deg)	=-35
Default Q (cfs)	= 0	Natural ground elev (ft)	= 63.9
Line capac. (cfs)	= 7.3	Line storage (cuft)	= 147

LINE 3 / Q = 4.5 / HT = 15 / WID = 15 / N = .013 / L = 176 / JLC = .9

DIG 1..... / DNLN = 2 DS #1

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	62.18	15.00	59.24	3.69	62.40	0.00	3.41	1.23
UPSTRM	63.24	15.00	61.48	3.69	63.45	0.00	1.17	1.23

Drainage area (ac) =	1.42	Slope of invert (%) =	1.273
Runoff coefficient =	.7	Slope energy grade line (%) =	0.493
Time of conc (min) =	10	Critical depth (in) =	10.62
Inlet time (min) =	10	Req'd length curb inlet (ft) =	0.0
Intensity (in/hr) =	4.56	Req'd grate area (sf) =	0.0
Cumulative C*A =	1.0	Depth at inlet opening (in) =	0
Flow contrib (cfs) =	4.5	Confluence angle (deg) =	0
Default Q (cfs) =	0	Natural ground elev (ft) =	63.9
Line capac. (cfs) =	7.3	Line storage (cuft) =	216

HYDRAULIC ANALYSIS

Run date: 02-09-1989
File:

Return Period = 10 Yrs
Rainfall file: JAMES CITY COUNTY

LINE 1 / Q = 10.4 / HT = 18 / WID = 18 / N = .013 / L = 40 / JLC = 1.2

DIGS 1..... / Outfall

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	58.33	15.93	57.00	6.26	58.94	11.48	-1.5	1.65
UPSTRM	58.79	15.36	57.51	6.45	59.44	12.74	3.99	1.61

Drainage area (ac)	= 0	Slope of invert (%)	= 1.275
Runoff coefficient	= 0	Slope energy grade line (%)	= 1.248
Time of conc (min)	= 12	Critical depth (in)	= 15.32
Inlet time (min)	= 0	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 5.63	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.8	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 0	Confluence angle (deg)	= 0
Default Q (cfs)	= 0	Natural ground elev (ft)	= 63
Line capac. (cfs)	= 11.9	Line storage (cuft)	= 65

LINE 2 / Q = 10.6 / HT = 18 / WID = 18 / N = .013 / L = 120 / JLC = .9

DIG 1..... / DNLN = 1 DS#2

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	59.57	18.00	57.61	5.99	60.12	0.00	3.88	1.77
UPSTRM	60.78	18.00	59.14	5.98	61.34	0.00	3.26	1.77

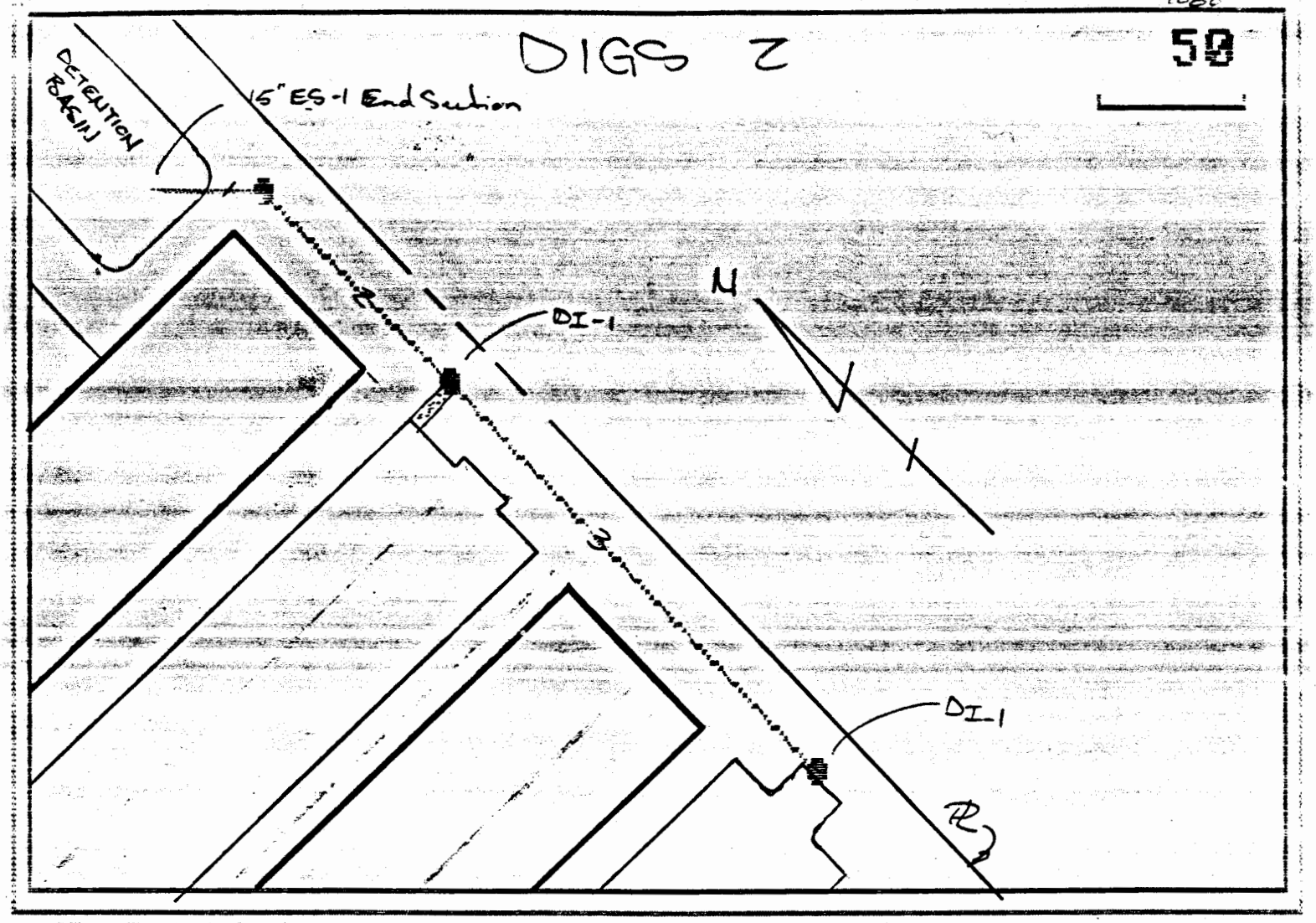
Drainage area (ac)	= 1.21	Slope of invert (%)	= 1.275
Runoff coefficient	= .7	Slope energy grade line (%)	= 1.015
Time of conc (min)	= 11	Critical depth (in)	= 15.32
Inlet time (min)	= 10	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 5.74	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.8	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 4.8	Confluence angle (deg)	=-35
Default Q (cfs)	= 0	Natural ground elev (ft)	= 63.9
Line capac. (cfs)	= 11.9	Line storage (cuft)	= 212

LINE 3 / Q = 5.9 / HT = 15 / WID = 15 / N = .013 / L = 176 / JLC = .9

DIG 1..... / DNLN = 2 DS#1

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	61.28	15.00	59.24	4.80	61.64	0.00	3.41	1.23
UPSTRM	62.97	13.70	61.48	5.01	63.36	8.43	1.17	1.18

Drainage area (ac) =	1.42	Slope of invert (%) =	1.273
Runoff coefficient =	.7	Slope energy grade line (%) =	0.779
Time of conc (min) =	10	Critical depth (in) =	12.21
Inlet time (min) =	10	Req'd length curb inlet (ft) =	0.0
Intensity (in/hr) =	5.93	Req'd grate area (sf) =	0.0
Cumulative C*A =	1.0	Depth at inlet opening (in) =	0
Flow contrib (cfs) =	5.8	Confluence angle (deg) =	0
Default Q (cfs) =	0	Natural ground elev (ft) =	63.9
Line capac. (cfs) =	7.3	Line storage (cuft) =	211



[← ↑ → ↓] Move [Spc] #'s [Home] Reset

[PgUp] Enlarge [PgDn] Reduce [Esc] Exit

HYDRAULIC ANALYSIS

Run date: 02-09-1989
File:

Return Period = 2 Yrs
Rainfall file: JAMES CITY COUNTY

LINE 1 / Q = 5.2 / HT = 15 / WID = 15 / N = .013 / L = 40 / JLC = 1.2

DIGS 2..... / Outfall DS#7

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	58.07	13.03	57.00	4.55	58.41	10.13	-1.25	1.13
UPSTRM	60.42	11.32	59.48	5.19	60.84	12.91	1.27	0.99

Drainage area (ac)	= 0	Slope of invert (%)	= 6.200
Runoff coefficient	= 0	Slope energy grade line (%)	= 6.084
Time of conc (min)	= 16	Critical depth (in)	= 11.29
Inlet time (min)	= 0	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 3.65	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.4	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 0	Confluence angle (deg)	= 0
Default Q (cfs)	= 0	Natural ground elev (ft)	= 62
Line capac. (cfs)	= 16.1	Line storage (cuft)	= 43

LINE 2 / Q = 5.2 / HT = 15 / WID = 15 / N = .013 / L = 88 / JLC = .9

DIGS 2..... / DNLN = 1 DS#6

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	60.92	15.00	59.58	4.26	61.21	0.00	1.16	1.23
UPSTRM	62.95	11.64	61.98	5.12	63.36	12.51	1.27	1.02

Drainage area (ac)	= 1.12	Slope of invert (%)	= 2.727
Runoff coefficient	= .7	Slope energy grade line (%)	= 2.443
Time of conc (min)	= 16	Critical depth (in)	= 11.61
Inlet time (min)	= 15	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 3.70	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.4	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 2.9	Confluence angle (deg)	= 44
Default Q (cfs)	= 0	Natural ground elev (ft)	= 64.5
Line capac. (cfs)	= 10.7	Line storage (cuft)	= 99

LINE 3 / Q = 2.4 / HT = 15 / WID = 15 / N = .013 / L = 176 / JLC = .9

DIGS 2..... / DNLN = 2 DS#5

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	63.32	14.84	62.08	1.96	63.38	2.86	1.16	1.23
UPSTRM	64.75	7.76	63.91	3.76	64.97	14.99	1.17	0.64

Drainage area (ac)	= .9	Slope of invert (%)	= 1.040
Runoff coefficient	= .7	Slope energy grade line (%)	= 0.795
Time of conc (min)	= 15	Critical depth (in)	= 7.72
Inlet time (min)	= 15	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 3.82	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 0.6	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 2.4	Confluence angle (deg)	= 0
Default Q (cfs)	= 0	Natural ground elev (ft)	= 66.33
Line capac. (cfs)	= 6.6	Line storage (cuft)	= 164

HYDRAULIC ANALYSIS

Run date: 02-09-1989
File:

Return Period = 10 Yrs
Rainfall file: JAMES CITY COUNTY

LINE 1 / Q = 6.9 / HT = 15 / WID = 15 / N = .013 / L = 40 / JLC = 1.2

DIGS 2..... / Outfall DS#7

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	58.09	13.03	57.00	6.14	58.67	10.13	-1.25	1.13
UPSTRM	60.37	13.06	59.48	6.13	61.15	10.08	1.27	1.13

Drainage area (ac)	= 0	Slope of invert (%)	= 6.200
Runoff coefficient	= 0	Slope energy grade line (%)	= 6.201
Time of conc (min)	= 16	Critical depth (in)	= 13.03
Inlet time (min)	= 0	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 4.91	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.4	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 0	Confluence angle (deg)	= 0
Default Q (cfs)	= 0	Natural ground elev (ft)	= 62
Line capac. (cfs)	= 16.1	Line storage (cuft)	= 45

LINE 2 / Q = 7.0 / HT = 15 / WID = 15 / N = .013 / L = 88 / JLC = .9

DIGS 2..... / DNLN = 1 DS#6

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	61.27	15.00	59.58	5.73	61.78	0.00	1.16	1.23
UPSTRM	63.07	13.06	61.98	6.20	63.67	10.08	1.27	1.13

Drainage area (ac)	= 1.12	Slope of invert (%)	= 2.727
Runoff coefficient	= .7	Slope energy grade line (%)	= 2.146
Time of conc (min)	= 16	Critical depth (in)	= 13.03
Inlet time (min)	= 15	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 4.98	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 1.4	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 3.9	Confluence angle (deg)	= 44
Default Q (cfs)	= 0	Natural ground elev (ft)	= 64.5
Line capac. (cfs)	= 10.7	Line storage (cuft)	= 104

LINE 3 / Q = 3.2 / HT = 15 / WID = 15 / N = .013 / L = 176 / JLC = .9

DIGS 2..... / DNLN = 2 DS #5

	HGL	DEPTH	INVERT	VEL	EGL	T WID	COVER	AREA
DNSTRM	63.61	15.00	62.08	2.62	63.71	0.00	1.16	1.23
UPSTRM	64.91	9.24	63.91	4.06	65.17	14.59	1.17	0.79

Drainage area (ac)	= .9	Slope of invert (%)	= 1.040
Runoff coefficient	= .7	Slope energy grade line (%)	= 0.695
Time of conc (min)	= 15	Critical depth (in)	= 9.20
Inlet time (min)	= 15	Req'd length curb inlet (ft)	= 0.0
Intensity (in/hr)	= 5.11	Req'd grate area (sf)	= 0.0
Cumulative C*A	= 0.6	Depth at inlet opening (in)	= 0
Flow contrib (cfs)	= 3.2	Confluence angle (deg)	= 0
Default Q (cfs)	= 0	Natural ground elev (ft)	= 66.33
Line capac. (cfs)	= 6.6	Line storage (cuft)	= 178

Diggers Bros Warehouses
Drainage Calculations

#7080
5-4-88
S.O.W.

Pre-development

Existing flow to 30" culvert (includes Diggers & others) (Calcs per Five Forks Commercial Park calcs dated 1-27-88)

D.A. = 17.68 C = 0.30 T_c = 30 min.

I_2 = 2.7 in/hr. , Q_2 = 14.3 cfs
I_{10} = 3.6 in/hr. , Q_{10} = 19.1 cfs

— Inlet Control —

H_w/D = 0.87 HW = 2.18'

— Outlet Control —

$H_w = H + h_o - LS_o$ H = 1.3 (chart 9) $h_o = \frac{1.5+2.5}{2} = 2$ LS_o = 10.50
H_w = 1.3 + 2 - 10.5 = -7.2 N.A. Inlet controls

Capacity of existing 30" RCP (from VDOT Fig 3.9.3.7)
Q_{cap} = 57 cfs

Flow into 30" Culvert contributed by Fiveforks Commercial Park (See plans by AES dated July '87)

SS#1	Q_2 = 4.87 cfs	Q_{10} = 6.35 cfs
SS#2	Q_2 = 3.20 cfs	Q_{10} = 4.17 cfs
	Sub-Total	= 10.52 cfs
		19.10 cfs

Existing Total Flow in 30" RCP = 29.62 cfs

Existing Available Capacity in 30" RCP = 57 - 29.6 = 27.4 cfs

Digges Brothers Office / Warehouses

#7080
5-18-88
SOW

On-site Drainage

Post

Intesity – L = 420' (DA6) ΔElev. = 72.5 − 64.3 = 8.2 S = 2%

Composite "C" Value

	A	C	CA
Pvmt & Roof	4.75	0.85	4.04
Green Area	2.13	0.20	0.43
	6.88		4.47

C = 0.65

.9×80×10 =
.1×10×10 =

Tc = 14 min

I_2 = 3.7 in/hr.
I_{10} = 5.8 in/hr.

a = 130.3
b = 18.5

$$V = \left[Q_o T + \frac{Q_o t_c}{4} - \frac{q_o T}{2} - \frac{3 q_o t_c}{4}\right] 60$$

$$T_c = \sqrt{\frac{2CA_a(b - t_c/4)}{q_o}} - b$$

Drainage Area Contributions –

DA3 DA = 1.14 Ac
Q_2 = 2.7 cfs
Q_{10} = 4.3 cfs

6000 ft³

DA7 DA = 0.36 Ac.
Q_2 = 0.9 cfs
Q_{10} = 1.4 cfs

DA4 DA = 0.73 Ac
Q_2 = 1.7 cfs
Q_{10} = 2.7 cfs

DA8 DA = 1.57 Ac. .65 ; .3 T_c = 30 min
Q_2 = 3.8 cfs 1.7 cfs $\left(\frac{80 \times 160}{2}\right) \times .67 = 4300$
Q_{10} = 5.9 cfs 5000 CF

DA5 DA = 1.05 cfs
Q_2 = 2.5 cfs
Q_{10} = 4.0 cfs

DA9 DA = 1.41 Ac.
Q_2 = 3.4 cfs 4500 1500 ft³
Q_{10} = 5.3 cfs

DA6 DA = 1.42 Ac.
Q_2 = 3.4 cfs
Q_{10} = 5.4 cfs

Total	
Q_2	= 18.4 cfs
Q_{10}	= 29.0 cfs

Off-site Drainage

Pre

DA1 DA = 5.71 Ac. C = 0.3 (Woodlands)
L = 550 ft ΔElev. = 83 − 65 = 18 ft S = 3.3%
Tc = 25 min
I_2 = 2.7 in/hr. Q_2 = 4.6 cfs 18" RCP @ 1.94%
I_{10} = 4.5 in/hr. Q_{10} = 7.7 cfs Qcap = 14 cfs OK

DA2 DA = 6.15
Q_2 = 5.0
Q_{10} = 8.3

Total	
Q_2	= 9.6 cfs
Q_{10}	= 16.0 cfs

18" RCP @ 3.6%
Qcap = 18 cfs OK

#7080
5-18-88
SAW

Capacity of 30" RCP @ Branscome's Q = 57 cfs
Contributors: 1.) Five Forks 10.5 cfs
2.) Diggers Warehouse 29.0 cfs
3.) Off-site residual drainage 16.0 cfs

1.5 cfs

Therefore the 30" RCP is adequate to convey the proposed drainage flow of the Diggers Brothers Office/Warehouse project.

On-site Pipe Design

Diggers Bro.s Office / Warehouse

5-18-88
#7080

T_c = 14 min. I_{10} = 5.8 in/hr. C = 0.65
I_2 = 3.7 in/hr.

FROM Pt.	To Pt.	Area Drained (Ac.)	Runoff Coeff. C	Inlet Time (min.)	I_2 (in/hr.)	I_{10} in/hr.	Q_2 (cfs)	Q_{10} (cfs)	Acc Q_{10} (cfs)	Pipe ~~Adequate~~ Design ~~Exist~~ Pipe (in)	S (%)	Qcap (cfs)	Use Pipe (in)	Qcap (cfs)
DA 1	DS4	5.71	0.3	25	2.7	4.5	4.6	7.7	7.7	—	1.94	—	18	14
DA2	DS4	6.15	0.3	25	2.7	4.5	5.0	8.3	16	—	3.6	—	18	18
DS4	DS3	11.86	0.3	25	2.7	4.5			16	—	2.0	—	24	30
DS3	DS2	1.14	0.65	14	3.7	5.8	2.7	4.3	20.3	—	2.0	—	24	30
DS2	DS1	0.73	0.65	14	3.7	5.8	1.7	2.7	23	—	2.0	—	24	30

On-Site
~~ROADSIDE~~ DITCH DESIGN

PROJECT Diggers Bros Office/W-hse
DATE 5-18-88
PROJ. # 7080

Tc = 14 min I_{10} = 5.8 in/hr C = 0.65
I_2 = 3.7 in/hr

NAME OF STREET N/A

FROM STATION (DIRECTION	TO STATION OF FLOW)	~~LEFT~~ OR ~~RIGHT~~	A (Ac.)	$Q_2 = CI_2A$ CFS	CUMULA. Q_2 CFS	AVG SLOPE FT/FT	ACTUAL VEL_2 FPS	ALLOWABLE VEL FPS	LINING TYPE	$Q_{10} = CI_{10}A$ CFS	CUM Q_{10} CFS	ACT. Vel_{10} FPS	DEPTH OF $FLOW_{10}$ IN	REMARKS L.F	Ditch Type	SS	Width	D
DA5	DA6	—	1.05	2.5	2.5	1.7%	4.7	—	Conc.	4.0	4.0	5.5	7.4	190'	PG-1E	2:1	3'	[illegible]
DA6	DS#1	—	1.78	4.3	6.8	1.7%	6	—	Conc.	6.8	10.8	7	11	160	PG-1E	2:1	3'	[illegible]
		Use	350 L.F. of PG-1E @ 1.7%															
DA8	DA9	—	1.57	3.8	3.8	0.56%	4	—	Conc.	5.9	5.9	4.5	9.8	180	PG-1E	2:1	3'	[illegible]
DA9	DS#1	—	1.41	3.4	7.2	2.9%	8	—	Conc.	5.3	11.2	9	10	210	PG-1E	2:1	3'	[illegible]
		Use	390 L.F. of PG-1E @ 2.9%															

WATERSHED	MC	MAINTENANCE PLAN	No	CTRL STRUC DESC	RCP Riser
BMP ID NO	011	SITE AREA acre	6.98	CTRL STRUC SIZE inches	21
PLAN NO	SP-62-88	LAND USE	General Businees	OTLT BARRL DESC	RCP Barrel
TAX PARCEL	(47-01)(01-07)	old BMP TYP	Dry Pond	OTLT BARRL SIZE inch	15
PIN NO	4710100007	JCC BMP CODE			
CONSTRUCTION DATE	1/1/1990	POINT VALUE	4	EMERG SPILLWAY	No
PROJECT NAME	Digges Brothers Office & Warehouse			DESIGN HW ELEV	61.5
FACILITY LOCATION	108 Ingram Road (Five Forks)			PERM POOL ELE	na
CITY-STATE	Williamsburg, Va. 23188	SVC DRAIN AREA acres	18.74	2-YR OUTFLOW cfs	26.91
CURRENT OWNER	Dr. T.R. O'Rourk (c/o JJ Digges)			10-YR OUTFLOW cfs	33.00
OWNER ADDRESS	512 South Henry Street			REC DRAWING	No
OWNER ADDRESS 2		SERVICE AREA DESCRI	Onsite Building, Parking & offsite area		
CITY-STATE-ZIP CODE	Williamsburg, Va. 23185	IMPERV AREA acres	4.75	CONSTR CERTI	No
OWNER PHONE	229-2045	RECV STREAM	UT of Mill Creek		
MAINT AGREEMENT	No	EXT DET-WQ-CTRL	No	LAST INSP DATE	12/12/2000
EMERG ACTION PLAN	No	WTR QUAL VOL acre-ft	0	INTERNAL RATING	3
		CHAN PROT CTRL	No	MISC/COMMENTS	
		CHAN PROT VOL acre-ft	0	Third Bldg SP-67-97. DHW estimated. 12" low flow orif.	
		SW/FLOOD CONTROL	Yes		
		GEOTECH REPORT	No		

Get Last BMP No

Return to Menu

CODE COMPLIANCE PLAN REVIEW COMMENTS
DIGGES BROTHERS OFFICE/WAREHOUSES
PLAN NO. SP-62-88 DEC
February 23, 1989

1. A Land Disturbing Permit and Siltation Agreement, with surety, are required for this project.

2. As-built drawings shall be provided for the detention basin upon completion.

3. A preconstruction conference shall be held on-site between the County, the Developer, the Project Engineer and the Contractor prior to issuance of a Land Disturbing Permit.

4. The Land Disturbing Permit when issued will require construction of the sediment basin prior to any other construction activities on the remainder of the project.

5. Recommend that the Development Review Committee require a streetlight at the project entrance on Ingram Road. The light shall be a standard Virginia Power, 5,000 lumen, 30-foot high streetlight installed by Virginia Power at a 45 degree angle to the entrance.

6. Provide inlet protection for all storm drain inlets.

7. Revise the pipe sizes for the drainage line from DS-1 to DS-4. As presented in the calculations from DS-1 to DS-2, the pipe diameter should be 15-inches and the rest of the pipe to DS-4 should be 18 inches in diameter.

8. Revise the design of the detention basin by providing a 21-inch riser to elevation 60.2 and a 15-inch barrel pipe. Provide a 12-inch orifice opening in the bottom of the riser and provide a paved ditch in the bottom of the basin from both outfall pipes to the orifice opening. Provide a trash rack for the riser. Also, provide a paved flume and ditch from the low point in the parking lot to the 12-inch orifice.

9. Add a note to the plan to provide a small diameter riser from the 12" orifice to elevation 58.0 in place of the stone filter over the orifice. This will allow the basin to serve as a sediment basin during the construction phase.

10. Provide elevations and other basin details on the basin cross-section shown on Sheet 5. Top of the dam shall be at least elevation 62.0.

11. Provide a permanent barricade or bumper blocks along edge of the paving adjacent to the basin as a safety feature.

0367C
4419c

MC011_DIGGERS_BROTHERS_OFFICE_AND_WAREHO

o As-Builts Required, ENV DIV Comment #2
Feb 23 1989

James City County
VIRGINIA
Jamestown 1607

TRANSMITTAL SHEET

ENGINEERING & RESOURCE PROTECTION → STORMWATER

Project: Diggs Brothers Office + Warehouse

County Plan No. SP-62-88

Assigned BMP No.: MC011

BMP Type: Dry Pond

RECEIVED ON
SEP 27 '12
Stormwater Division

Information Enclosed:

- ☐ Record Drawings (Asbuilts)
- ☐ Construction Certification
- ☐ Computations
- ☒ Other: 24 year old project being closed out. Creating makeshift file for MS4 post construction purposes. See memo for explanation.

If no BMP file exists for MC011, use this. If it does add this to the file.

Name: Scott J. Thomas

Date: 7/31/12

Signature: Scott J Thomas

Thomas R O'Rourke

Send to

Sasha Diggs
3617 Ironbound Rd

Memorandum to File

Case: SP-62-88 - Digges Brothers Office/Warehouse
Location: 108 Ingram Road; GPIN 4710100007
Date: July 31, 2012
From: Scott J. Thomas, Director
Engineering and Resource Protection

This particular case was brought to the attention of the Division director as a long-outstanding case in the system. The Division director fully reviewed the project's case file information and known records in the County's land records system.

The case was first reviewed by the Division in 1988 (over 24 years ago) under County case No. SP-62-88. The site is an approximate 7 acre site, zoned B-1. Our Division issued comments on the plan and it received approval from us on June 1, 1989. Review comments did indicate the need for onsite stormwater management (detention); however, it appeared that the primary reason for the onsite stormwater basin was for temporary site erosion and sediment control. Notes during plan review called for asbuilt drawings for the sediment basin. Plan sheet information shows the basin structure but no indication or label as a permanent SWM facility. There were no comments during plan review for a Declaration of Covenants, Inspection-Maintenance agreement. At that time this may not have been a Division protocol.

A siltation agreement was required and was obtained on September 10, 1991. Land-disturbing permit # 92-21 was issued for the project on September 13, 1991 and the permit was released in February 2000. Current amount of siltation surety being held (check) is $18,000. This was the original amount, never reduced.

It appears that siltation surety was being held for 2 primary reasons. One was the Code Compliance had made note that although C.O. were being issued, there were deficiencies in the completion of work approved by the plan. Mainly landscaping, parking lot, dumpster pad, fences incomplete. However, C.O. was issued. Second, no asbuilt information was ever received. Although no detailed information was found on plans or in computation to show the stormwater basin area was to serve as a permanent dry pond BMP structure, there was indication in plan review comments indicating need for onsite stormwater management and comments requiring asbuilts. No asbuilt information was provided.

Further research in records shows that although notes about site deficiencies were made by Codes Compliance, following the date of those notes (April 1995) over 21 additional building permits have been issued on the subject site since 1995. Also, although the onsite detention basin was issued a County BMP ID number (MC011) the level of control is very basic and minimum in nature. The area is a basic basin in dry pond detention form. Onsite storm piping is 15, 18 and 24 inch RCP. Primary reliance for stormwater control is the receiving downstream storm piping system – 30 inch size and greater which ends up in the County regional basin MC045.

It is the decision of the program administrator given the time passage, subsequent issuance of additional building permits at the site without regard to past concerns, and lack of clarity about the onsite basin's nature – temporary sediment control versus permanent BMP control that our Division will proceed with the release of the $18,000 surety for the project. A make-shift file will be set up for the onsite basin and forwarded to Stormwater Division as a BMP number already exists and continued tracking of the simple basin is warranted under the MS4 program.

07-31-12

POOR QUALITY

ORIGINAL(S) FOLLOW

THIS IS THE BEST COPY AVAILABLE

VCE DOCUMENT CONVERSION

FIELD CORRECTION NOTICE

LOCATION INGRAM RD. SITE PLAN

ISSUED TO DIGGES BROS. OFFICE WAREHOUSE PERMIT NO. 62-88
PERMIT HOLDER AND/OR ALL RESPONSIBLE PARTIES.

NOTICE DELIVERED TO DOUGLAS PAYNE

Upon inspection, violations of the _____ Sec. _____ were in evidence.

The following orders are hereby issued for their correction: A TEMPORARY C.O. WAS GRANTED FOR THIS SITE IN A SPIRIT OF COOPERATION. THE PORTION OF THE SITE THAT IS IN USE MUST COME INTO COMPLIANCE WITH THE APPROVED SITE PLAN WITHIN A REASONABLE PERIOD OF TIME. LANDSCAPING, PARKING AREAS, SIGNAGE, DUMPSTER PADS & FENCES, ETC. NEED TO BE COMPLETED BY JULY 1 & PARKING LOT BEHIND EXISTING BUILDING MUST BE PAVED by JUNE 1, 1995.

PLEASE CALL FOR INSPECTION WHEN CORRECTIONS HAVE BEEN COMPLETED. ACCEPTANCE AND APPROVAL BY AN INSPECTOR OF THIS DEPARTMENT IS REQUIRED. ALL CORRECTIONS MUST BE MADE ON OR BEFORE SEE ABOVE

DATE APRIL 25, 1995 BY [signature]
INSPECTOR

FILE COPY

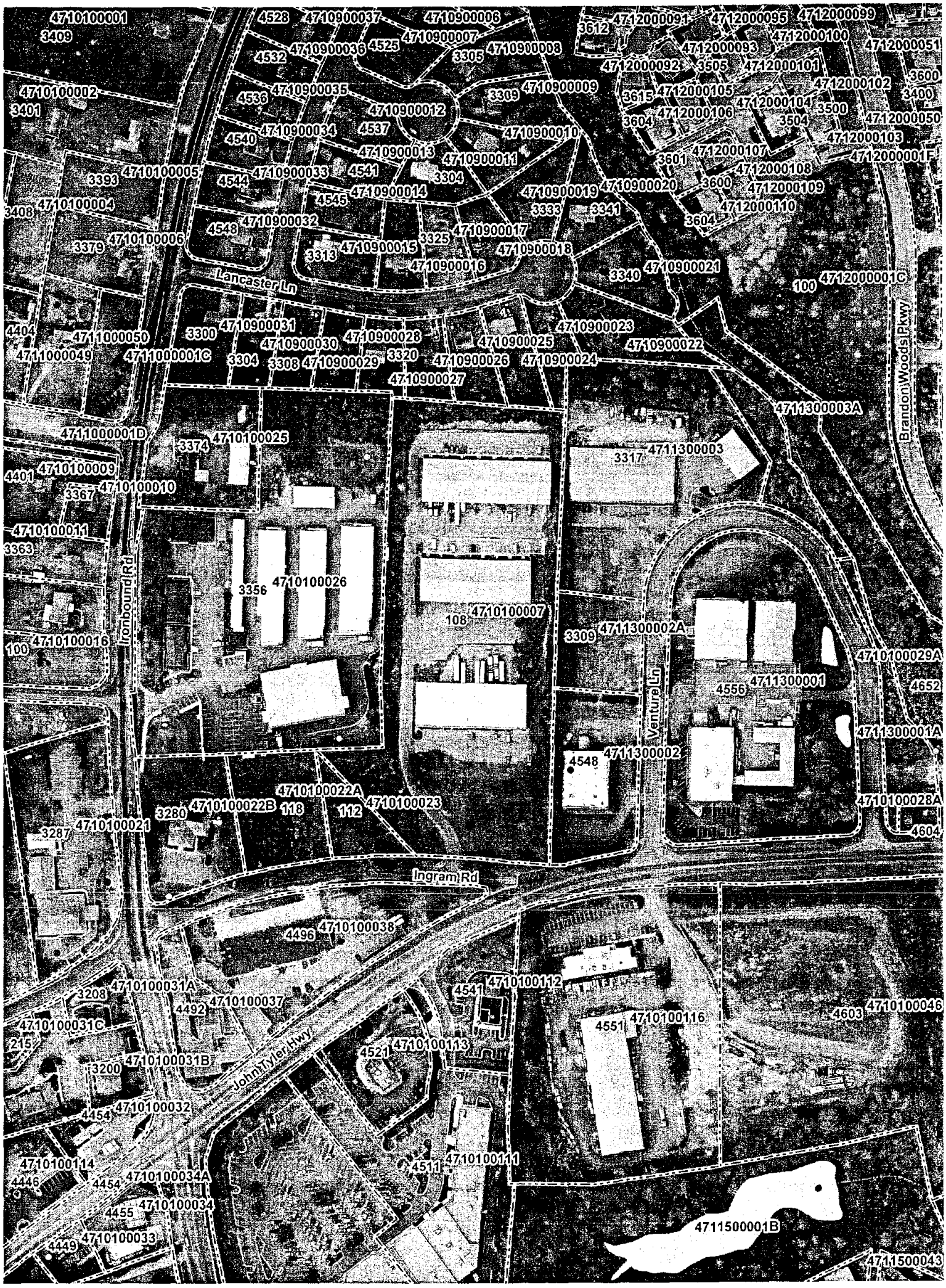
1 SIT
2 UT
3 DR
4 UT
5 NC
6 LA
-2b ST
-3e ST

sional corporatio
estown Road
,Virginia 23185
53-0040
lanning, Surveying

since issuance of not in April 1995 of site deficiencies (BS+P), the following additional Building Permits + approvals have been issued after.

BP-00-035	Bakery-kitchen
BP-00-0584	Gas Meter
BP-00-1146	Fire Suppression - HVAC
BP-00-2873	Interior
BP-91-2139	Warehouse Shell
BP-93-0053	Alterations
BP-93-0635	Alterations
BP-93-0894	Sewer addition
BP-93-2309	Storage Unit #2
BP-94-0638	Office Storage Alteration
BP-95-0320	Retail-alteration
BP-95-1911	Office Storage Alteration
BP-96-0413	Warehouse
BP-96-3647	Commercial-alteration
BP-97-0526	Commercial - alteration

↓ List continues over 21 total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4712000095
4712000099
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Lancaster Ln
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4712000001C
4404
4710900031
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4710900023
4711000050
4710900028
4710900025
4710900022
4711000049
4711000001C
3304
3308
4710900029
3320
4710900026
4710900024
4710900027
4711300003A
Brandon Woods Pkwy
4711000001D
3374
4710100025
3317
4711300003
4401
4710100009
3367
4710100010
4710100011
3363
Ironbound Rd
3356
4710100026
108
4710100007
3309
4711300002A
4710100016
100
4710100029A
4556
4711300001
4652
Venture Ln
4711300001A
4548
4711300002
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4710100028A
3280
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118
112
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3287
4710100021
4604
Ingram Rd
4496
4710100038
4710100031A
3208
4710100112
4541
4492
4710100037
4603
4710100046
4710100031C
4551
4710100116
215
4521
4710100113
3200
4710100031B
John Tyler Hwy
4454
4710100032
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4511
4710100111
4446
4454
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4455
4710100034
4711500001B
4449
4710100033
4711500043

TRANSMITTAL

DATE: October 2, 1990

TO: CODE COMPLIANCE, Darryl Cook
HEALTH
REAL ESTATE ASSESSMENTS*

FIRE
JCSA
VDOT

RECEIVED 10-3-90

FROM: Mike Freda, Planner

SUBJECT: Case No. SP-62-88. Digges Brothers Office & Warehouse

ITEMS ATTACHED: Approved site plan for your files
*Case file

INSTRUCTIONS: *Forward case file to Records Management

RETURN REQUIRED BY: n/a

AGENCY'S COMMENTS:

MC 011
SP-62-88

EROSION CONTROL REVIEW COMMENTS
DIGGES BROTHERS OFFICE/WAREHOUSE DEC
SITE PLAN NO. SP-62-88

1. A Land Disturbing Permit and Siltation Agreement, with surety, are required for this project.

2. Remove the note on Sheet 3 that states that Stormwater Management is to be met by construction of a detention basin on Baron Woods. This is incorrect. Provide Stormwater Management on the site through a combination of parking lot storage and detention storage in the area adjacent to the existing 30-inch pipe.

3. Provide a temporary riser over the end of the existing 30-inch pipe draining the site to allow the depressed area to serve as a sediment basin during construction.

4. Provide a note stating that the areas to be paved will be stabilized by gravel immediately after final grade is reached.

5. Provide a note on Sheet 3 specifying that the 2:1 and 3:1 slopes along the north and west boundaries of the project must be stabilized with EC-2 material immediately after final grade is established.

6. Recommend that the Site Plan Review Committee require a streetlight at the project entrance on Ingram Road. The light shall be a standard Virginia Power, 5000 lumen, 30-foot high streetlight installed by Virginia Power at a 45 degree angle to the intersection.

DEC/jw
3993c

FIELD CORRECTION NOTICE

LOCATION INGRAM Rd. SITE PLAN

ISSUED TO DIGGES BROS. OFFICE WAREHOUSE ~~PERMIT NO.~~ 62-88
PERMIT HOLDER AND/OR ALL RESPONSIBILE PARTIES.

NOTICE DELIVERED TO DOUGLAS PAYNE

Upon inspection, violations of the _____ Sec. _____ were in evidence.

The following orders are hereby issued for their correction: A TEMPORARY C.O. WAS GRANTED FOR THIS SITE IN A SPIRIT OF COOPERATION. THE PORTION OF THE SITE THAT IS IN USE MUST COME INTO COMPLIANCE WITH THE APPROVED SITE PLAN WITHIN A REASONABLE PERIOD OF TIME. LANDSCAPING, PARKING AREAS, SIGNAGE, DUMPSTER PADS & FENCES, ETC. NEED TO BE COMPLETED by JULY 1 & PARKING LOT BEHIND EXISTING BUILDING MUST BE PAVED by JUNE 1, 1995.

PLEASE CALL FOR INSPECTION WHEN CORRECTIONS HAVE BEEN COMPLETED. ACCEPTANCE AND APPROVAL BY AN INSPECTOR OF THIS DEPARTMENT IS REQUIRED. ALL CORRECTIONS MUST BE MADE ON OR BEFORE SEE ABOVE

DATE APRIL 25, 1995 BY [signature]
INSPECTOR

FILE COPY

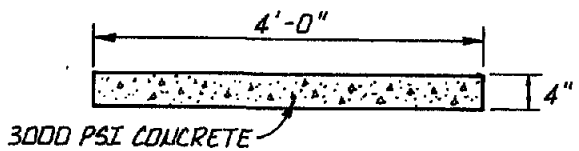
1 SIT
2 UT
3 DR
4 UT
5 NC
6 LA
-2b ST
-3e ST

ssional corporatio
nestown Road
g,Virginia 23185
253-0040
Planning, Surveying

LOT 28

GENERAL NOTES

1. SITE IS ZONED B-1, GENERAL BUSINESS.
2. TAX MAP PARCEL NO. (47-1)(1-7).
3. TOTAL ACREAGE OF SITE = 6.9807 ACRES.
4. ALL UTILITY WIRES ARE TO BE INSTALLED UNDERGROUND.
5. SITE IS TO BE SERVED WITH BOTH PUBLIC WATER AND SEWER.
6. PLAT FOR THE CONVEYANCE OF UTILITY EASEMENTS SHALL BE SUBMITTED TO JAMES CITY COUNTY UPON FIELD LOCATIONS.
7. ALL PARKING SPACES ARE TO BE PROVIDED WITH 4 INCHES PAINTED STRIPING.
8. ALL HANDICAP SPACES ARE TO BE PROVIDED WITH A "HANDICAPPED PARKING SPACE" SIGN, A MINIMUM OF 4 FEET IN HEIGHT.
9. LIGHTING TO BE PHOTOCELL ACTIVATED.
10. DEVELOPER AGREES TO CONFORM WITH THE CURRENT JAMES CITY COUNTY ORDINANCE REQUIREMENTS CONCERNING SIGNAGE.
11. BUILDINGS ARE NOT TO EXCEED A MAXIMUM HEIGHT OF 24 FEET.
12. BUILDINGS ARE CLASSIFIED AS USE GROUP B/S1 AND ARE TO BE OF CONSTRUCTION TYPE 3B.
13. FREESTANDING ROAD SIGN IS NOT TO EXCEED A MAXIMUM HEIGHT OF 20 FEET NOR A SIZE OF 32 SQUARE FEET PER FACE.



TYPICAL CONCRETE SIDEWALK DETAIL

SCALE: 1/2" = 1'-0"

1. TREES REQUIR
TREES PROVID

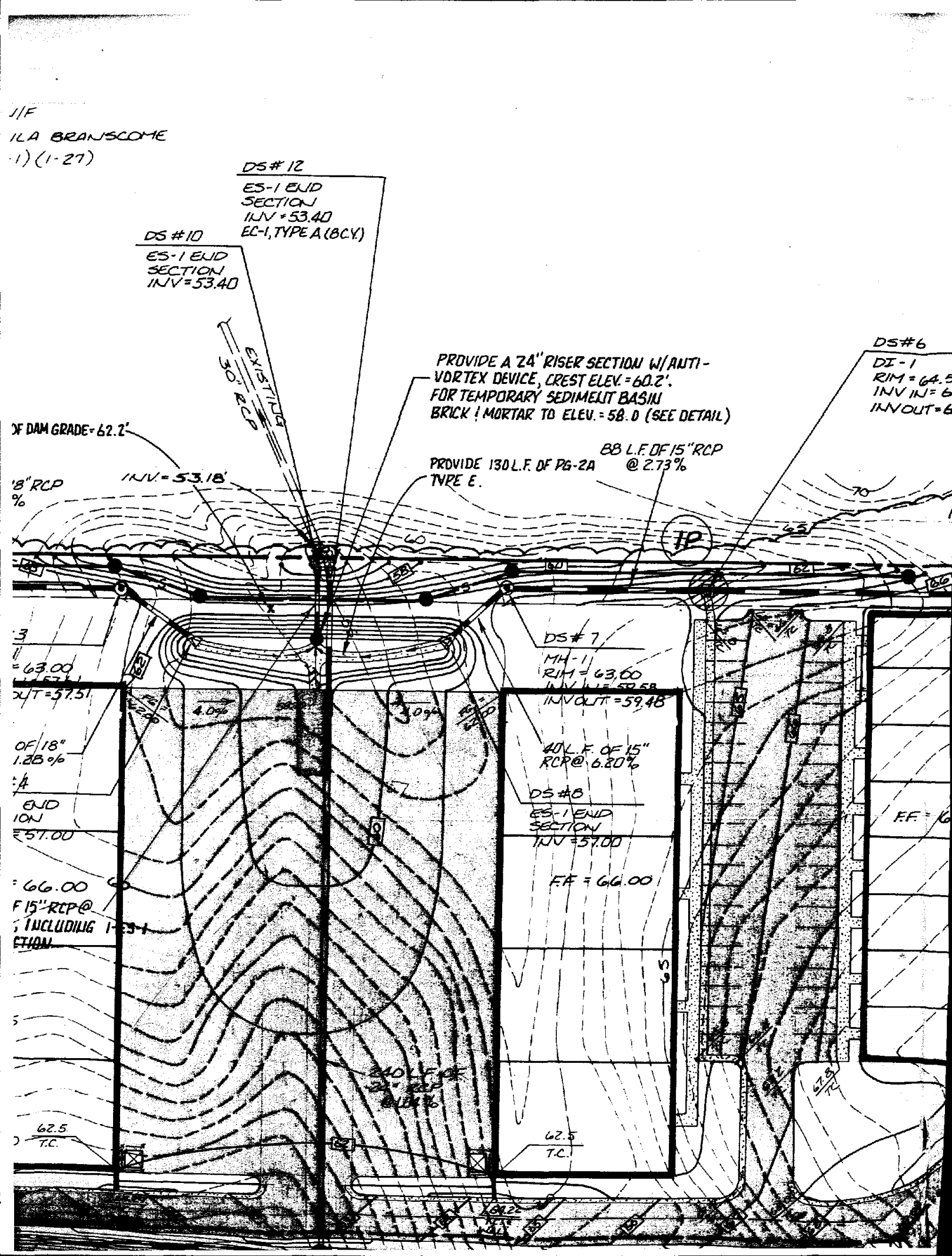
2. PERIMETER TR
PERIMETER LE
TREES REQUIR
TREES PROVID

3. THE SITE CC
OAKS) 8" - 1
IN DIAMETER,

4. LANDSCAPING
FREESTANDING

5. FRONT FOUNDA
ARE TO BE PF
FACING INGRA

MARK	COMMON MAN
*A	RED MAPLE
B	WILLOW OAK


DS # 12
ES-1 END SECTION
INV = 53.40
EC-1, TYPE A (8 C.Y.)
DS # 10
ES-1 END SECTION
INV = 53.40
EXISTING 30" RCP
PROVIDE A 24" RISER SECTION W/ ANTI-VORTEX DEVICE, CREST ELEV. = 60.2'. FOR TEMPORARY SEDIMENT BASIN BRICK & MORTAR TO ELEV. = 58.0 (SEE DETAIL)
DS # 6
DI-1
RIM = 64.5
INV IN = 6
INV OUT = 6
OF DAM GRADE = 62.2'
INV. = 53.18'
PROVIDE 130 L.F. OF PG-2A TYPE E.
88 L.F. OF 15" RCP @ 2.73%
TP
DS # 7
MH-1
RIM = 63.60
INV OUT = 59.48
40 L.F. OF 15" RCP @ 6.20%
DS # 8
ES-1 END SECTION
INV = 57.00
F.F. = 66.00
F.F. = 6
240 L.F. OF 24" RCP @ 1.04%
62.5 T.C.
62.5 T.C.
4.0%
3.0%
1.28%
67.8 T.C.