



CERTIFICATE OF AUTHENTICITY

THIS IS TO CERTIFY THAT THE FOLLOWING ELECTRONIC RECORDS ARE TRUE AND ACCURATE REPRODUCTIONS OF THE ORIGINAL RECORDS OF JAMES CITY COUNTY GENERAL SERVICES DEPARTMENT- STORMWATER DIVISION; WERE SCANNED IN THE REGULAR COURSE OF BUSINESS PURSUANT TO GUIDELINES ESTABLISHED BY THE LIBRARY OF VIRGINIA AND ARCHIVES; AND HAVE BEEN VERIFIED IN THE CUSTODY OF THE INDIVIDUAL LISTED BELOW.

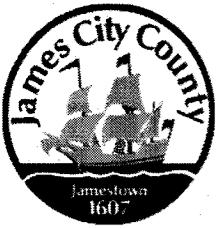
BMP NUMBER: PC003

DATE VERIFIED: May 24, 2012

QUALITY ASSURANCE TECHNICIAN: Leah Hardenbergh

Leah Hardenbergh

LOCATION: WILLIAMSBURG, VIRGINIA



Stormwater Division

MEMORANDUM

DATE: March 13, 2010
TO: Michael J. Gillis, Virginia Correctional Enterprises Document Management Services
FROM: Jo Anna Ripley, Stormwater
PO: 270712
RE: Files Approved for Scanning

General File ID or BMP ID: PC003

PIN: 3110800001C

Subdivision, Tract, Business or Owner

Name (if known):

Fox Ridge

Property Description:

Rec Area Parcel 3 Fox Ridge

Site Address:

3955 Fox Hunt Trail

(For internal use only)

Box 1

Drawer: 1

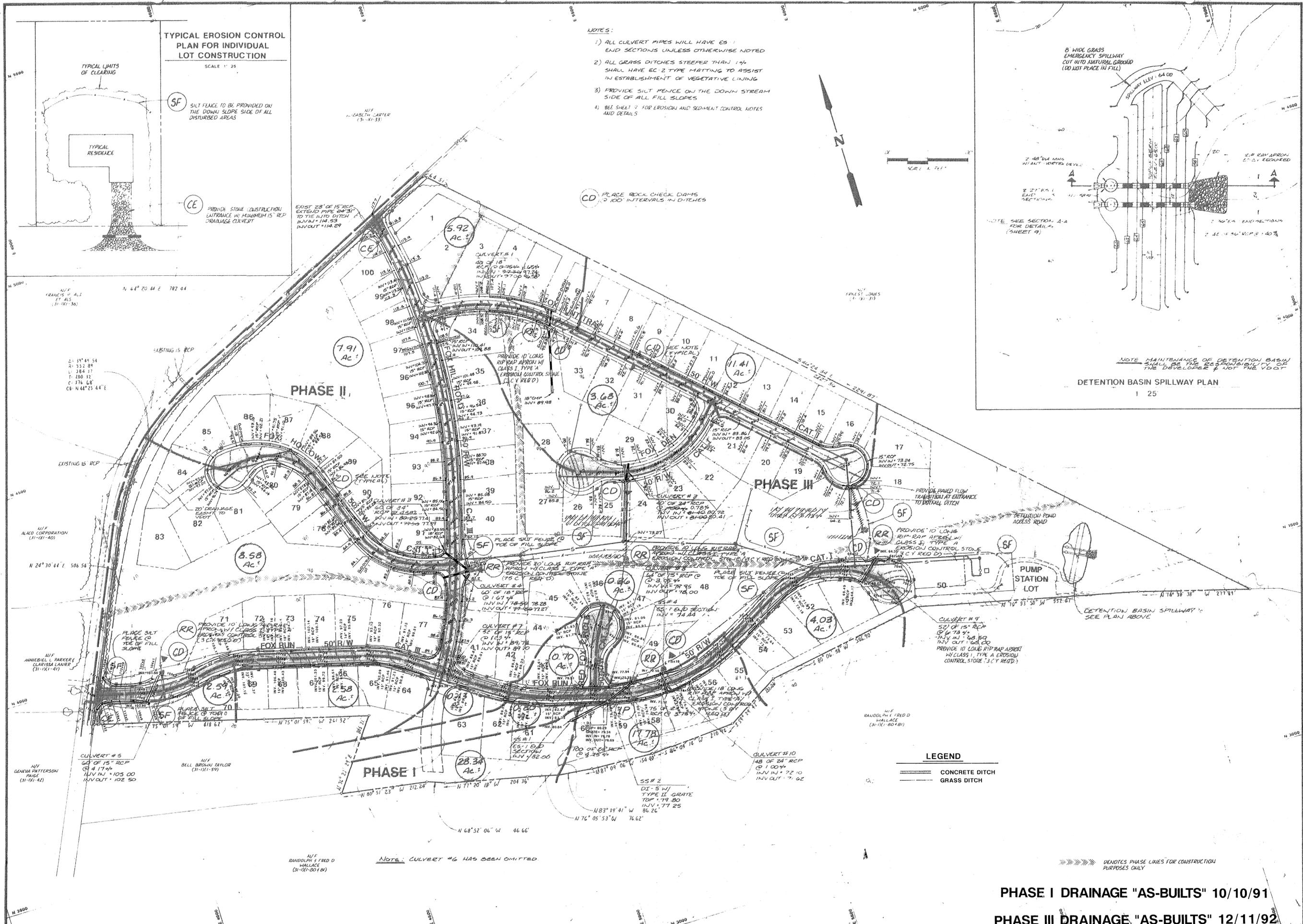
Agreements: (in file as of scan date)

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Book or Doc#:

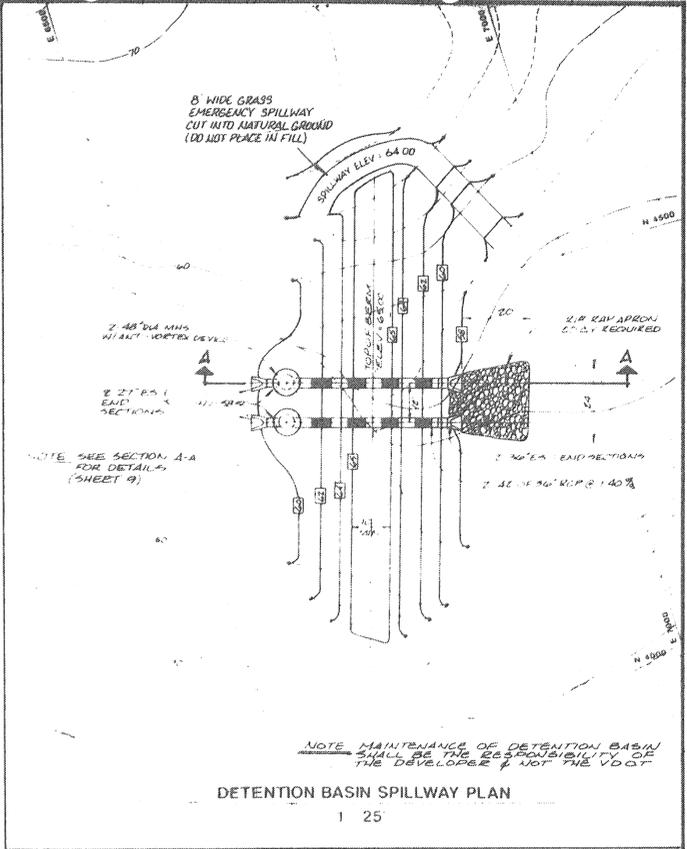
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Comments

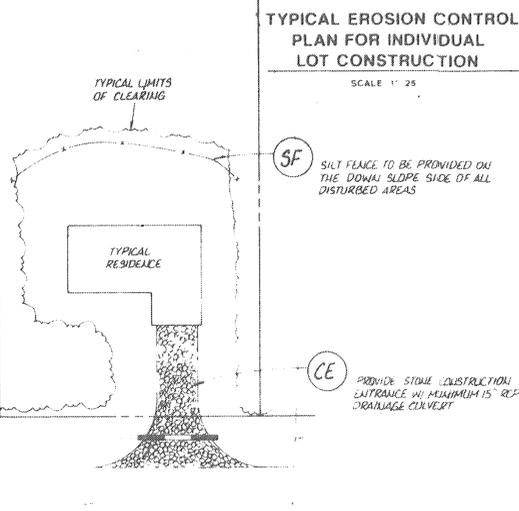


- NOTES:**
- 1) ALL CULVERT PIPES WILL HAVE END SECTIONS UNLESS OTHERWISE NOTED
 - 2) ALL GRASS DITCHES STEEPER THAN 1:4 SHALL HAVE EC-2 TYPE MATTING TO ASSIST IN ESTABLISHMENT OF VEGETATIVE LINING
 - 3) PROVIDE SILT FENCE ON THE DOWN STREAM SIDE OF ALL FILL SLOPES
 - 4) SEE SHEET 7 FOR EROSION AND SEDIMENT CONTROL NOTES AND DETAILS

CD PLACE ROCK CHECK DAMS @ 100' INTERVALS IN DITCHES



DETENTION BASIN SPILLWAY PLAN
1 25



TYPICAL EROSION CONTROL PLAN FOR INDIVIDUAL LOT CONSTRUCTION
SCALE 1" = 25'

EXISTING 15" RCP
N 64° 20' 44" E 102.44'
N 24° 30' 44" E 506.56'
N 75° 01' 57" W 241.52'
N 11° 20' 18" W 204.36'
N 68° 52' 06" W 46.66'

EXISTING 15" RCP
N 24° 30' 44" E 506.56'
N 75° 01' 57" W 241.52'

EXISTING 15" RCP
N 75° 01' 57" W 241.52'
N 11° 20' 18" W 204.36'

EXISTING 15" RCP
N 11° 20' 18" W 204.36'
N 68° 52' 06" W 46.66'

LEGEND

	CONCRETE DITCH
	GRASS DITCH

NO.	DATE	REVISION / COMMENT / NOTE
1	12/21/92	REVISED PER ACC. COMMENTS
2	12/21/92	REVISED PER ACC. COMMENTS
3	12/21/92	REVISED PER ACC. COMMENTS
4	12/21/92	REVISED PER ACC. COMMENTS
5	12/21/92	REVISED PER ACC. COMMENTS



AES, a professional corporation
1761 Jamestown Road
Williamsburg, Virginia 23185
804-253-0040
Engineering, Planning, Surveying



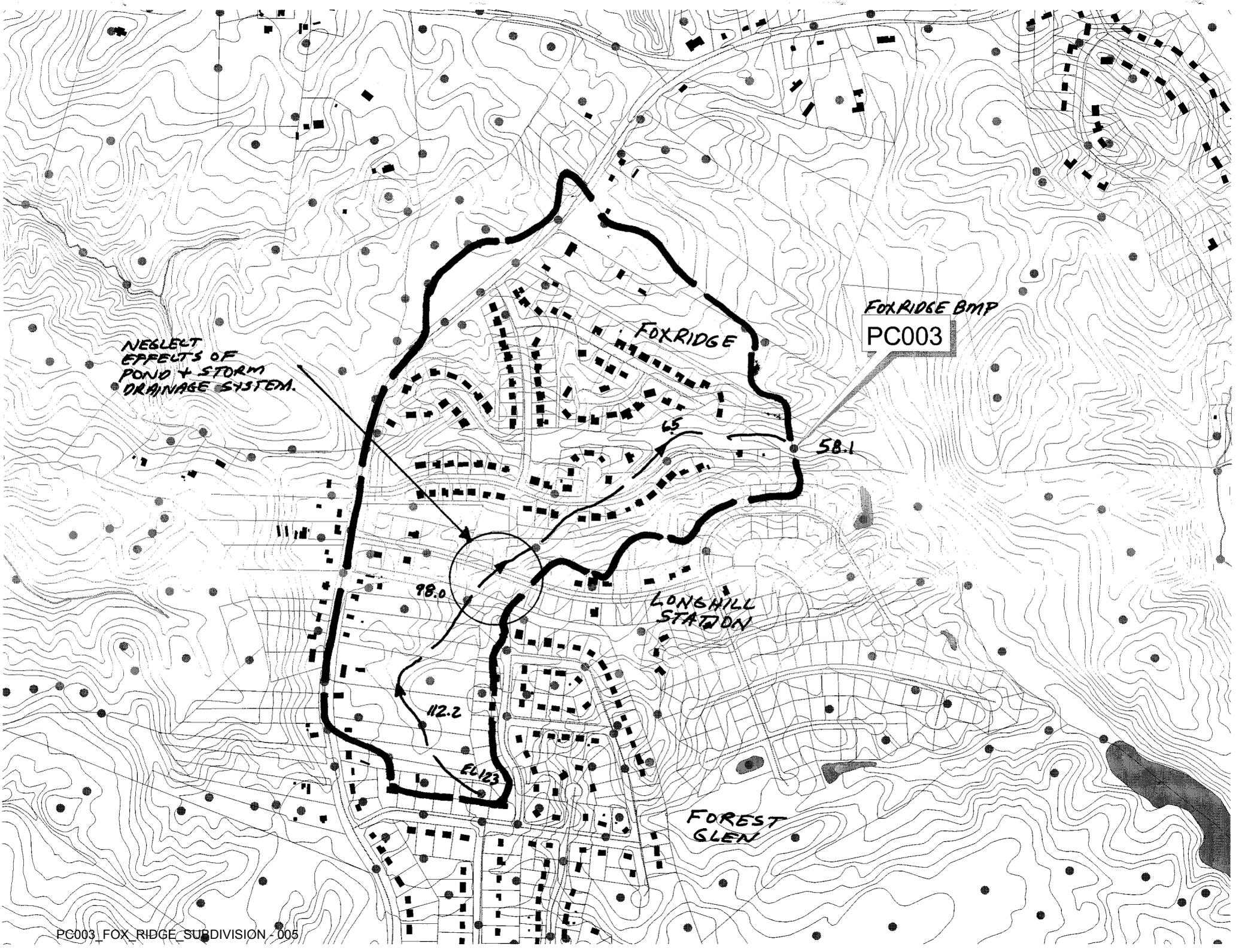
DRAINAGE AND SEDIMENT & EROSION CONTROL PLAN
FOX RIDGE
OWNER/DEVELOPER: FERRELL GENERAL CONSTRUCTION CORPORATION
JAMES CITY COUNTY, VIRGINIA

Designed HWP	Drawn AES
Scale 1"=100'	Date OCT. 1998
Project No. 7105	Drawing No. 4

PHASE I DRAINAGE "AS-BUILTS" 10/10/91
PHASE III DRAINAGE "AS-BUILTS" 12/11/92
PHASE II DRAINAGE "AS-BUILTS" 12/21/92



PC003



NEGLECT
EFFECTS OF
POND + STORM
DRAINAGE SYSTEM.

FOX RIDGE BMP
PC003

FOX RIDGE

LONGHILL
STATION

FOREST
GLEN

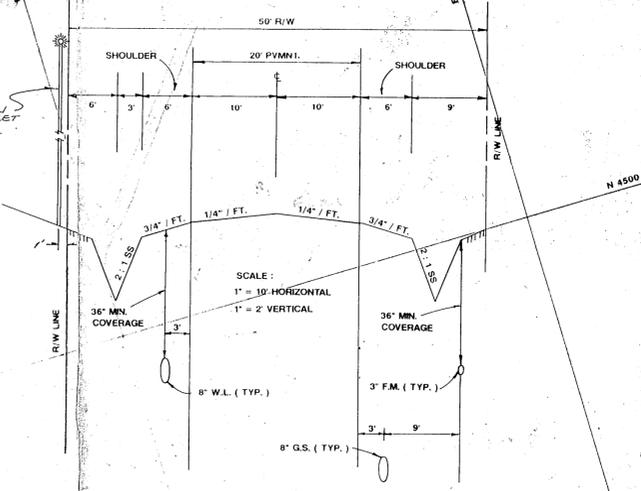
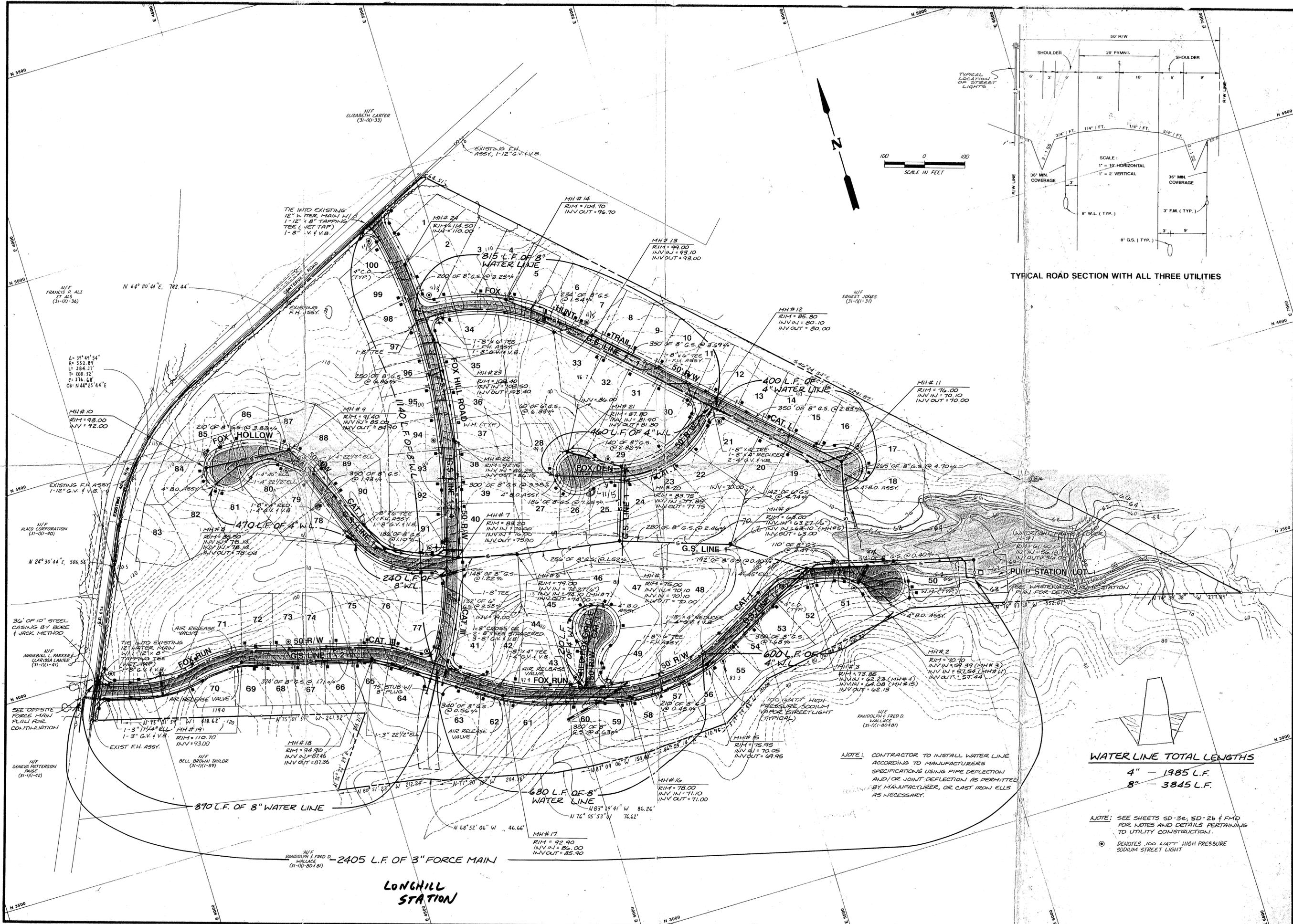
98.0

112.2

EL 123

65

58.1



TYPICAL ROAD SECTION WITH ALL THREE UTILITIES

WATER LINE TOTAL LENGTHS
 4" - 1985 L.F.
 8" - 3845 L.F.

NOTE: SEE SHEETS SD-3e, SD-26 & FMD FOR NOTES AND DETAILS PERTAINING TO UTILITY CONSTRUCTION.
 (C) DENOTES 100 WATT HIGH PRESSURE SODIUM STREET LIGHT

NOTE: CONTRACTOR TO INSTALL WATER LINE ACCORDING TO MANUFACTURERS SPECIFICATIONS USING PIPE DEFLECTION AND/OR JOINT DEFLECTION AS PERMITTED BY MANUFACTURER, OR CAST IRON ELLS AS NECESSARY.

NO.	DATE	REVISION / COMMENT / NOTE	BY
1	11/19/85	REVISED PER JCC COMMENTS	SAH
2	12/11/85	REVISED PER JCC COMMENTS	SAH
3	1/12/86	REVISED PER JCC COMMENTS	SAH

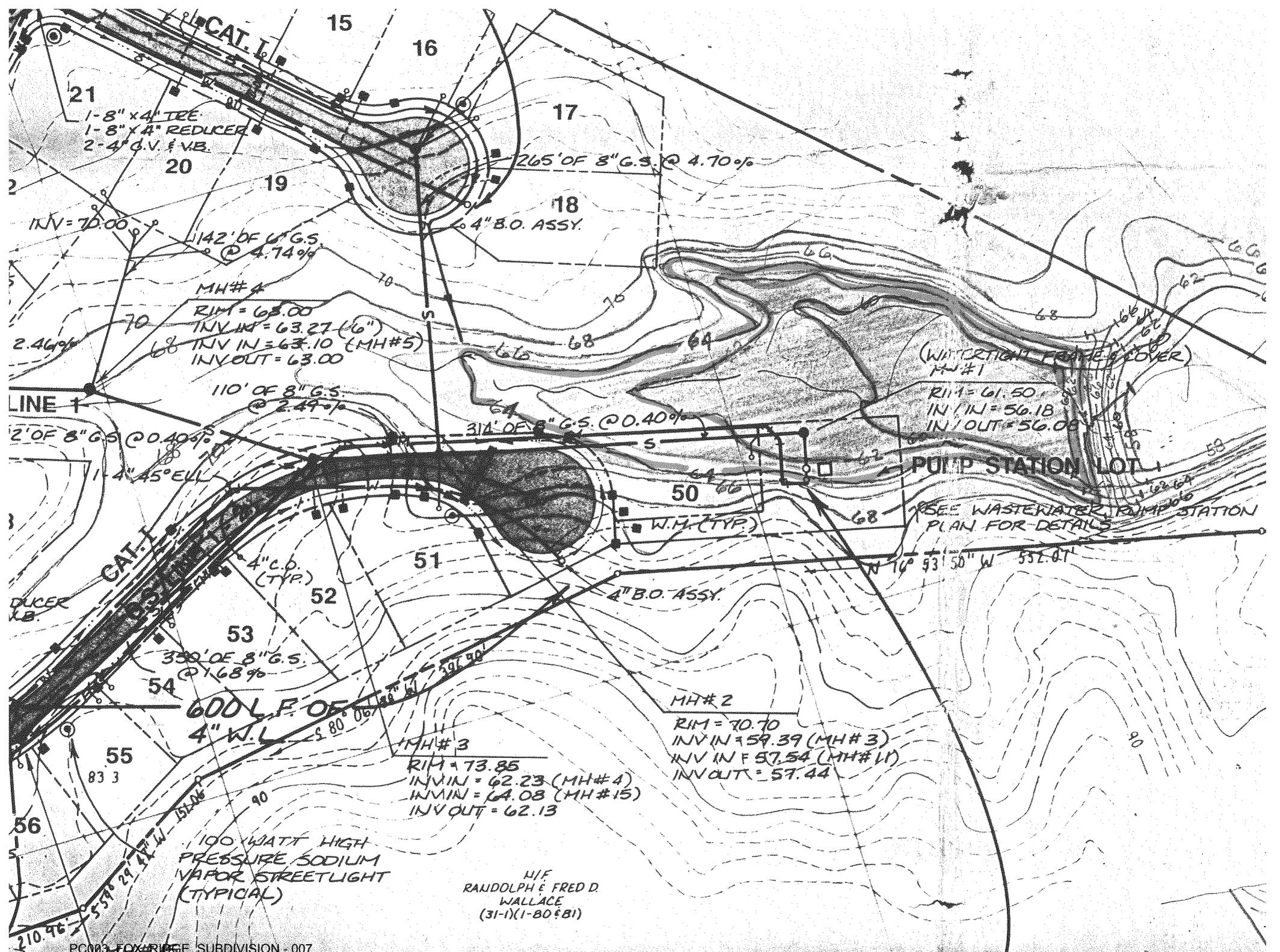


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 Engineering, Planning, Surveying



UTILITY PLAN
FOX RIDGE
 OWNER/DEVELOPER: FERRELL GENERAL CONSTRUCTION CORPORATION
 JAMES CITY COUNTY, VIRGINIA

Designed GAM/HWP	Drawn AES
Scale 1"=100'	Date OCT. 1985
Project No. 7105	Drawing No. 3



1-8" x 4" TEE
 1-8" x 4" REDUCER
 2-4" G.V. & V.B.

142' OF 6" G.S.
 @ 4.74%

MH#4
 RIM = 63.00
 INV IN = 63.27 (6")
 INV IN = 63.10 (MH#5)
 INV OUT = 63.00

110' OF 8" G.S.
 @ 2.49%

314' OF 8" G.S. @ 0.40%

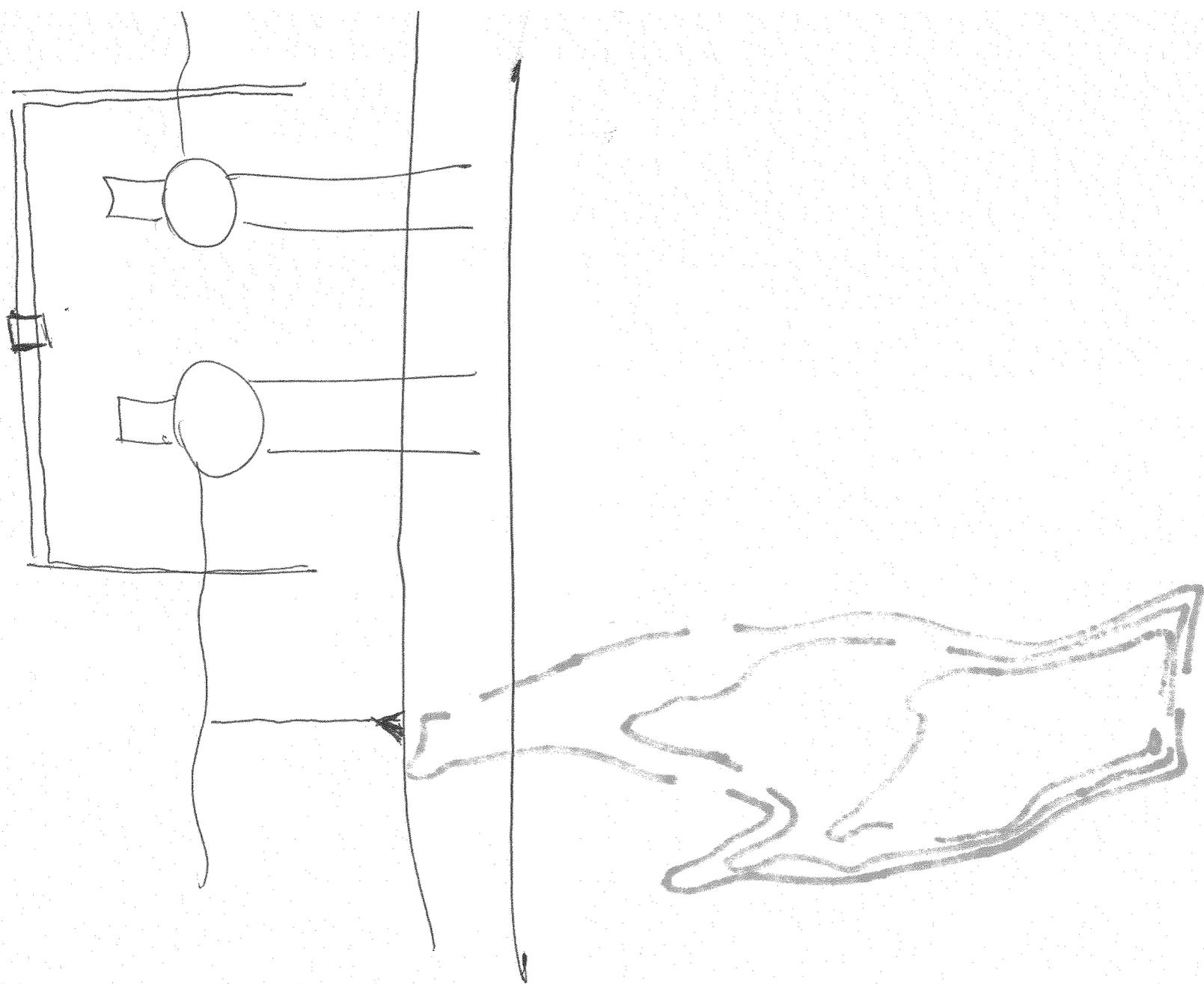
(WATERTIGHT FRAME & COVER)
MH#1
 RIM = 61.50
 INV IN = 56.18
 INV OUT = 56.08

PUMP STATION LOT 1
 SEE WASTEWATER PUMP STATION
 PLAN FOR DETAILS

600 L.F. OF
 4" W.L. @ 80' 06" W

MH#3
 RIM = 73.85
 INV IN = 62.23 (MH#4)
 INV IN = 64.08 (MH#15)
 INV OUT = 62.13

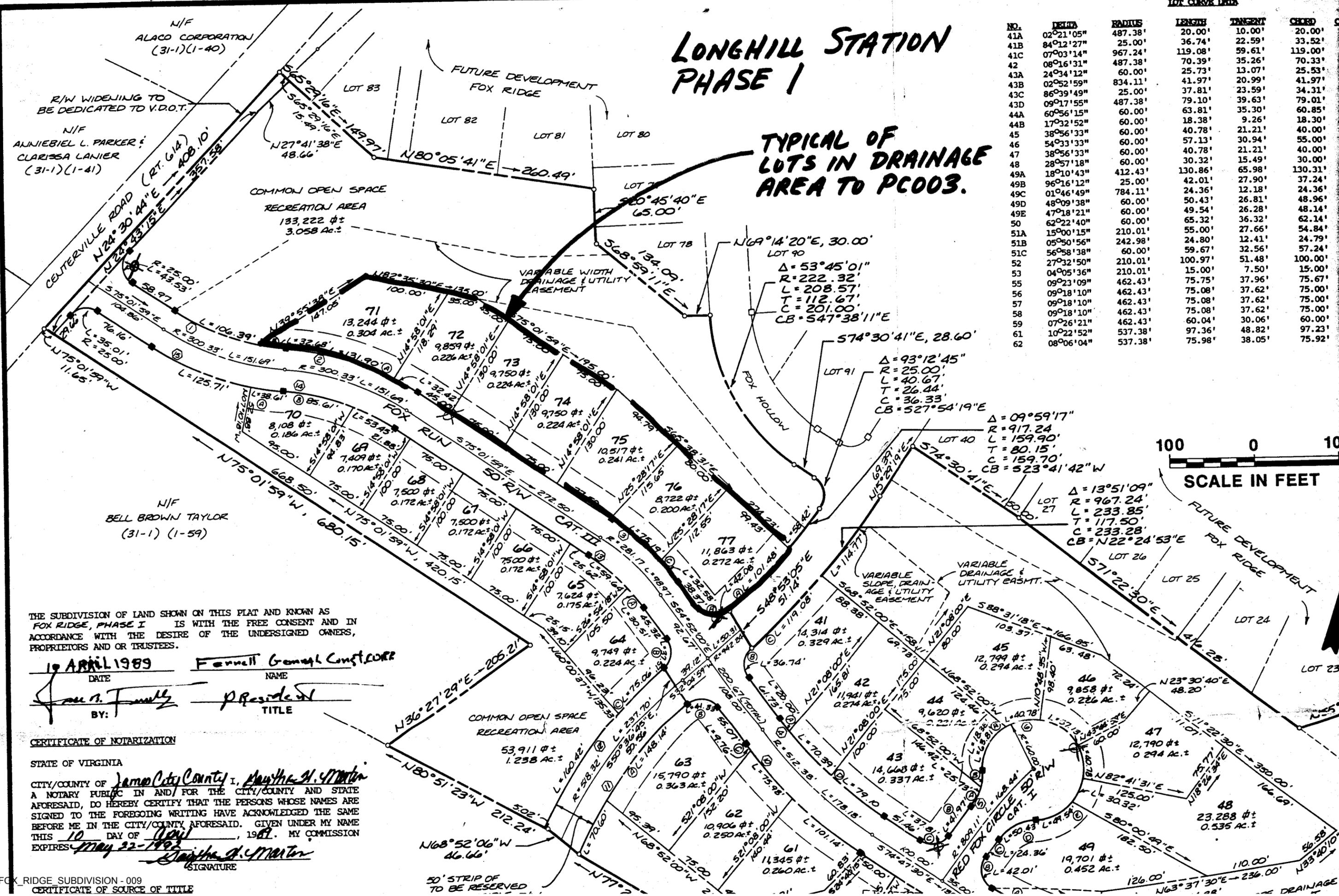
MH#2
 RIM = 70.70
 INV IN = 59.39 (MH#3)
 INV IN = 57.54 (MH#11)
 INV OUT = 57.44



LONGHILL STATION PHASE I

TYPICAL OF
LOTS IN DRAINAGE
AREA TO PC003.

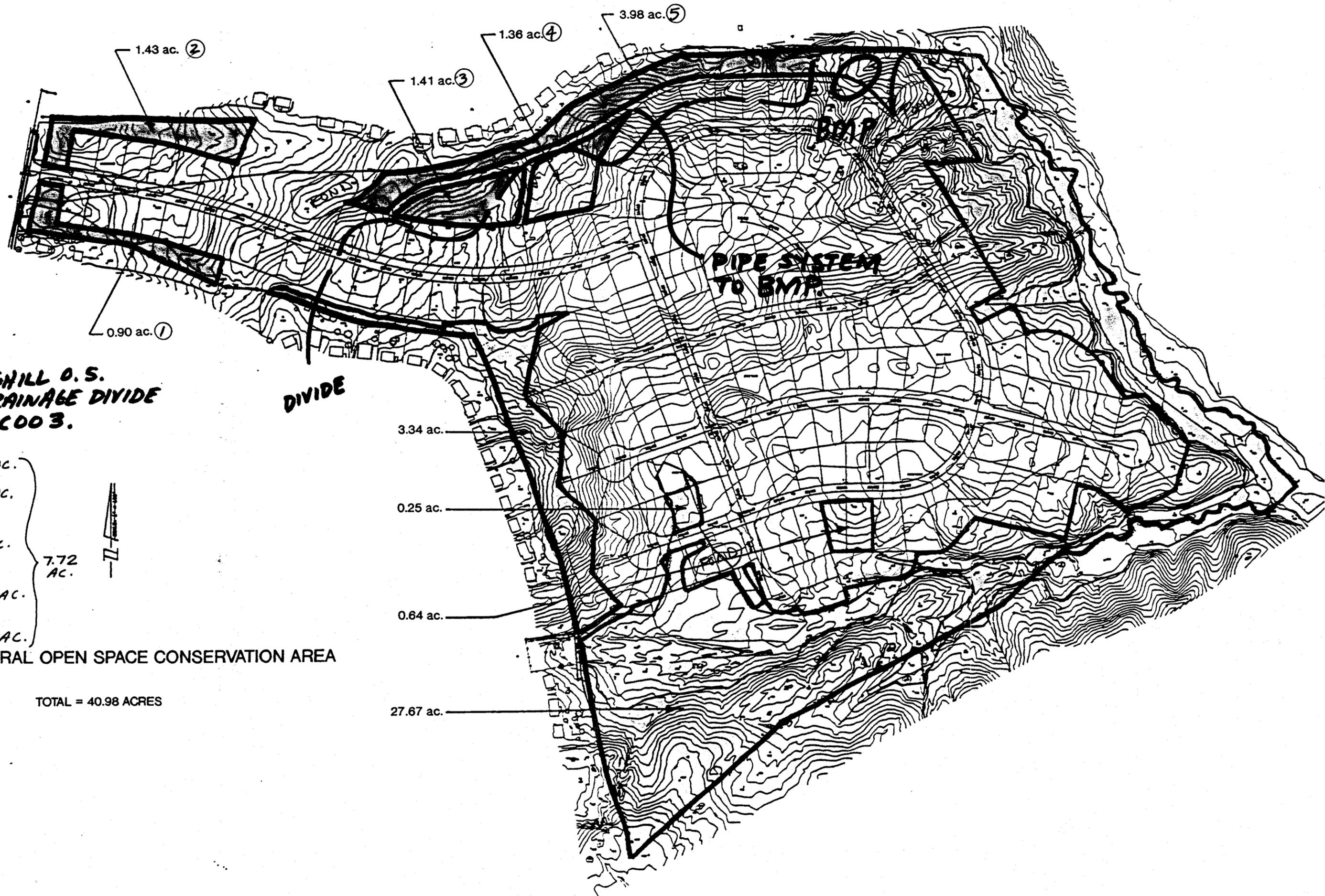
NO.	DELTA	RADIUS	LENGTH	TANGENT	CHORD
41A	02°21'05"	487.38'	20.00'	10.00'	20.00'
41B	84°12'27"	25.00'	36.74'	22.59'	33.52'
41C	07°03'14"	967.24'	119.08'	59.61'	119.00'
42	08°16'31"	487.38'	70.39'	35.26'	70.33'
43A	24°34'12"	60.00'	25.73'	13.07'	25.53'
43B	02°52'59"	834.11'	41.97'	20.99'	41.97'
43C	86°39'49"	25.00'	37.81'	23.59'	34.31'
43D	09°17'55"	487.38'	79.10'	39.63'	79.01'
44A	60°56'15"	60.00'	63.81'	35.30'	60.85'
44B	17°32'52"	60.00'	18.38'	9.26'	18.30'
45	38°56'33"	60.00'	40.78'	21.21'	40.00'
46	54°33'33"	60.00'	57.13'	30.94'	55.00'
47	38°56'33"	60.00'	40.78'	21.21'	40.00'
48	28°57'18"	60.00'	30.32'	15.49'	30.00'
49A	18°10'43"	412.43'	130.86'	65.98'	130.31'
49B	96°16'12"	25.00'	42.01'	27.90'	37.24'
49C	01°46'49"	784.11'	24.36'	12.18'	24.36'
49D	48°09'38"	60.00'	50.43'	26.81'	48.96'
49E	47°18'21"	60.00'	49.54'	26.28'	48.14'
50	62°22'40"	60.00'	65.32'	36.32'	62.14'
51A	15°00'15"	210.01'	55.00'	27.66'	54.84'
51B	05°50'56"	242.98'	24.80'	12.41'	24.79'
51C	56°58'38"	60.00'	59.67'	32.56'	57.24'
52	27°32'50"	210.01'	100.97'	51.48'	100.00'
53	04°05'36"	210.01'	15.00'	7.50'	15.00'
55	09°23'09"	462.43'	75.75'	37.96'	75.67'
56	09°18'10"	462.43'	75.08'	37.62'	75.00'
57	09°18'10"	462.43'	75.08'	37.62'	75.00'
58	09°18'10"	462.43'	75.08'	37.62'	75.00'
59	07°26'21"	462.43'	60.04'	30.06'	60.00'
61	10°22'52"	537.38'	97.36'	48.82'	97.23'
62	08°06'04"	537.38'	75.98'	38.05'	75.92'



THE SUBDIVISION OF LAND SHOWN ON THIS PLAT AND KNOWN AS
FOX RIDGE, PHASE I IS WITH THE FREE CONSENT AND IN
ACCORDANCE WITH THE DESIRE OF THE UNDERSIGNED OWNERS,
PROPRIETORS AND OR TRUSTEES.

19 APRIL 1989
DATE
Fennell General Const. Corp.
NAME
BY: *John R. Fennell* PRESIDENT
TITLE

CERTIFICATE OF NOTARIZATION
STATE OF VIRGINIA
CITY/COUNTY OF James City County, Virginia
A NOTARY PUBLIC IN AND FOR THE CITY/COUNTY AND STATE
AFORESAID, DO HEREBY CERTIFY THAT THE PERSONS WHOSE NAMES ARE
SIGNED TO THE FOREGOING WRITING HAVE ACKNOWLEDGED THE SAME
BEFORE ME IN THE CITY/COUNTY AFORESAID. GIVEN UNDER MY NAME
THIS 10 DAY OF April, 1989. MY COMMISSION
EXPIRES May 22-1992
Jantha H. Martin
SIGNATURE



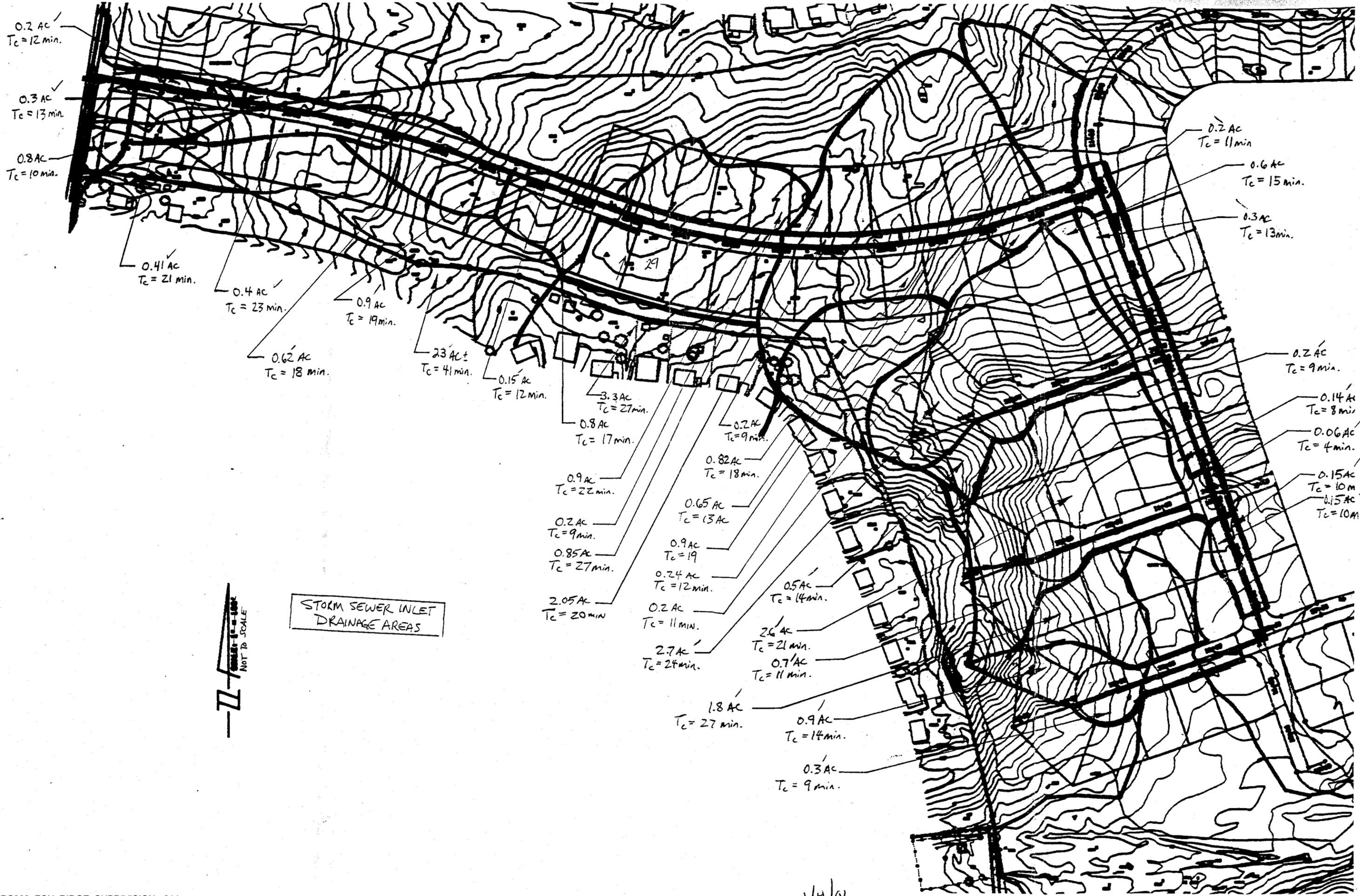
**LONGHILL O.S.
IN DRAINAGE DIVIDE
TO PC003.**

- ① 0.90 AC.
- ② 1.43 AC.
- ③ 1.41 AC.
- ④ 0.00 AC.
- ⑤ 3.98 AC.

7.72
AC.

NATURAL OPEN SPACE CONSERVATION AREA

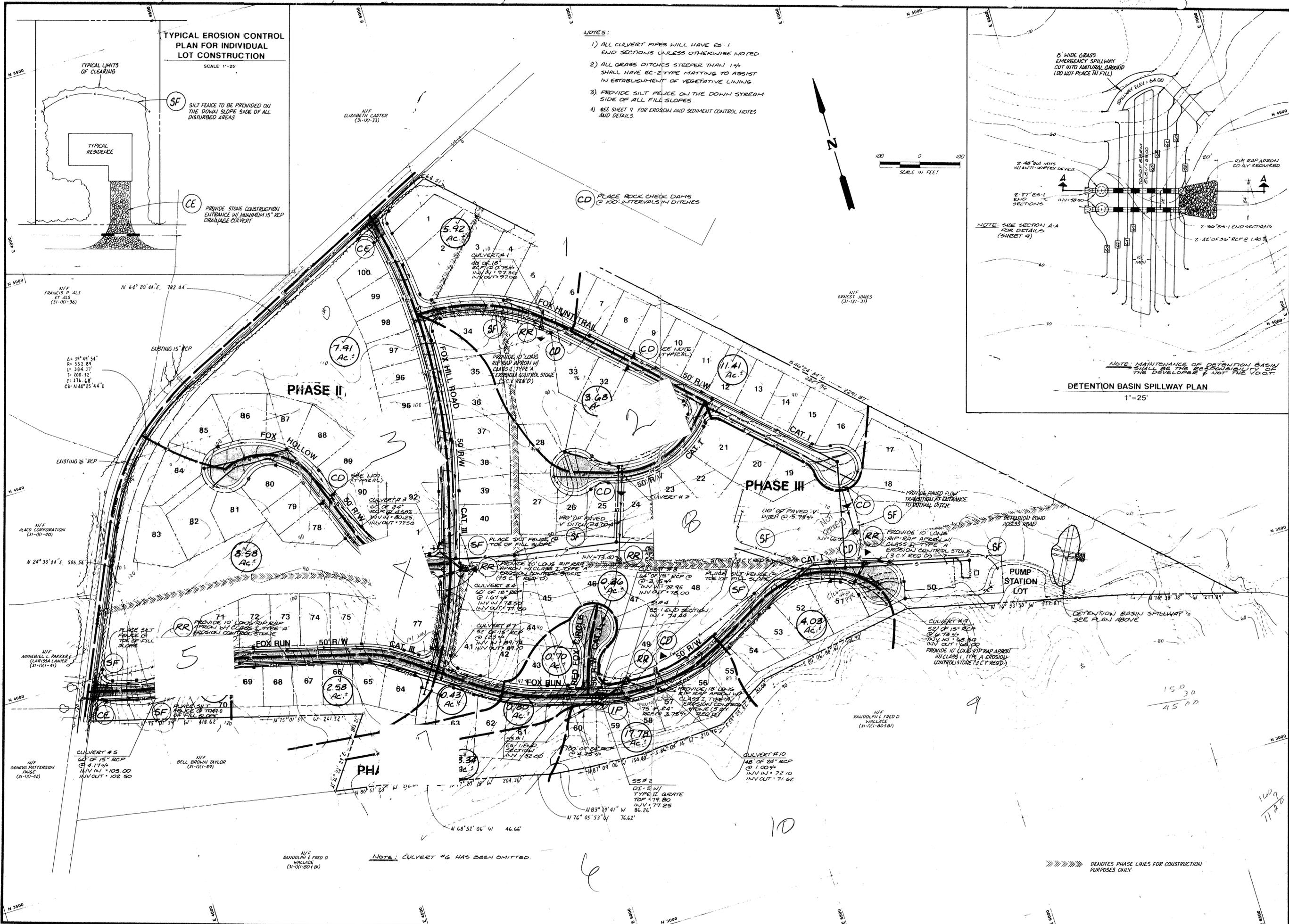
TOTAL = 40.98 ACRES



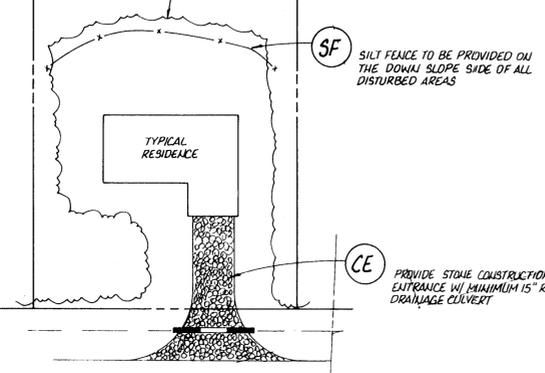


 NOT TO SCALE

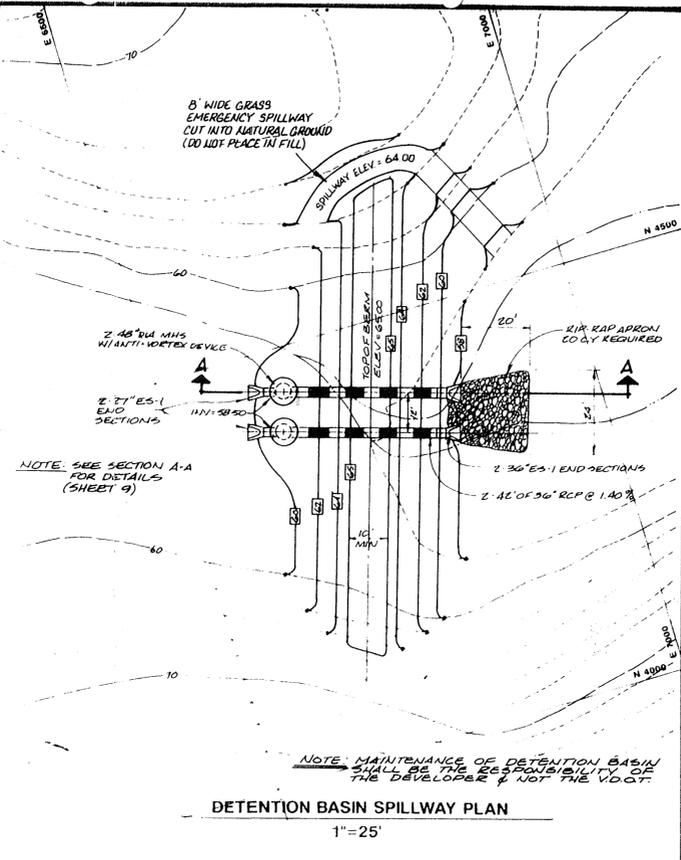
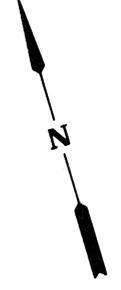
1/4/96



TYPICAL EROSION CONTROL PLAN FOR INDIVIDUAL LOT CONSTRUCTION
SCALE 1"=25'



- NOTES:**
- 1) ALL CULVERT PIPES WILL HAVE E3-1 END SECTIONS UNLESS OTHERWISE NOTED
 - 2) ALL GRASS DITCHES STEEPER THAN 1% SHALL HAVE EC-2 TYPE MATTING IN ESTABLISHMENT OF VEGETATIVE LINING
 - 3) PROVIDE SILT FENCE ON THE DOWN STREAM SIDE OF ALL FILL SLOPES
 - 4) SEE SHEET 9 FOR EROSION AND SEDIMENT CONTROL NOTES AND DETAILS



NO.	DATE	REVISION / COMMENT / NOTE	BY
1	10/1/88	REVISED PER JCC COMMENTS	EAH
2	10/1/88	REVISED PER JCC COMMENTS	EAH
3	10/1/88	REVISED PER JCC COMMENTS	EAH



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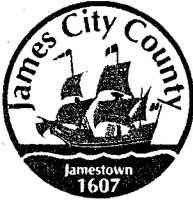


DRAINAGE AND SEDIMENT & EROSION CONTROL PLAN
FOX RIDGE
OWNER/DEVELOPER: FERRELL GENERAL CONSTRUCTION CORPORATION
JAMES CITY COUNTY, VIRGINIA

Designed HWP	Drawn AES
Scale 1"=100'	Date OCT. 1988
Project No. 7105	
Drawing No. 4	



1" = 300'



DEVELOPMENT MANAGEMENT

101-E MOUNTS BAY ROAD, P.O. BOX 8784, WILLIAMSBURG, VIRGINIA 23187-8784
(757) 253-6671 Fax: (757) 253-6850 E-MAIL: devtman@james-city.va.us

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codecomp@james-city.va.us

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environ@james-city.va.us

PLANNING
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planning@james-city.va.us

COUNTY ENGINEER
(757) 253-6678
INTEGRATED PEST MANAGEMENT
(757) 253-2620

FOX RIDGE (PC 003, 5-3-90) BMP ANALYSES

- SITE DRAINAGE AREA FROM DRAINAGE PLAN (FOX RIDGE)
 $5.92 + 11.41 + 3.68 + 7.91 + 8.58 + 2.59 + 2.58 + 0.43 + 0.36 + 0.70 + 0.80 + 28.34 + 17.78 + 4.03 = 95.11 \text{ AC.}$
- COUNTY GIS (ADJUSTED FOR STORM DRAINAGE SYSTEM IN SECTION 1B LONGHILL STATION) SEE 1" = 800' D.A. MAP
 $4,633,368.14 = 106.36 \text{ ACRES} \quad \therefore \text{USE } 106.36 \text{ AC.}$
- IN LOOKING AT THE OVERALL DRAINAGE AREA, LOTS 71-77 ARE REPRESENTATIVE OF TYPICAL LOT DENSITY & ARRANGEMENT OF DEVELOPMENT
- FROM LONGHILL STA. PHASE ONE PLAT, LOTS 71-77 THE AVERAGE LOT SIZE IS 0.24 AC OR 10,522 SF.
- FROM LONGHILL STA. PHASE I PLAT, AVG. LOT SIZE IS 10,778 SF OR 0.2478 AC.
- ASSUME OPEN SPACE IN LONGHILL CAN REMAIN UNDEVELOPED ALL OTHER COULD BECOME SIMILAR TO LONGHILL STA. LOTS, EXCEPT 7-A1 ZONED LOTS.
APPROX. O.S. WITHIN DIVIDE TO PC003 = 7.72 AC. (SEE MAP)
- USE SCS METHOD SINCE > 20 AC.

CN FACTORS

SOILS: PREDOMINANTLY 11C, 14B, 19B IN DRAINAGE AREA (SOIL MAP SHEET NO. 13)

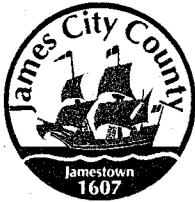
11C CRAVEN-UCHEE, 6-10%

14B EMPORIA FINE SANDY LOAM, 2-6%

19B KEMPVILLE-EMPORIA FINE SANDY LOAM, 2-6%

<u>HSG</u>
C/A
C
B/C

\therefore AVERAGE HSG IS C.



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$$DA = 106.36$$

$$A1 \text{ LOTS ALONG CENTERVILLE AVG. } 202 \times 7 = 14.14 \text{ AC.}$$

$$NOS \text{ LONGHILL} = 7.72 \text{ AC.}$$

$$\text{SINGLE FAMILY DEV, } 0.25 \text{ AC AVG} = \underline{84.5 \text{ AC}}$$

USING TABLE 2.20, TR-55, RUNOFF CURVE No. 106.36 AC.

$$CN \text{ A1 LOTS, C SOIL} = 77 \quad 14.14 \text{ AC} \quad (13.29\%)$$

$$CN \text{ N.O.S, C SOIL} = 70 \quad 7.72 \text{ AC} \quad (7.26\%)$$

$$CN \text{ SF, } 0.25 \text{ AC, C SOIL} = 83 \quad 84.5 \text{ AC} \quad (79.45\%)$$

$$\text{WEIGHTED CN} = 81$$

TIME OF CONCENTRATION / FOREST GLEN TO FOX RIDGE
SLS SEGMENTAL APPROACH

- NEGLECT POND A, PC020 IN LONGHILL STATION
- NEGLECT STORM SYSTEM IN LONGHILL STA. SEC 1

$$\textcircled{1} \quad 100' \text{ OVERLAND, YARD } n=0.2, s = \frac{3}{95} = 0.0316\% = 9.8 \text{ MIN.}$$

$$\textcircled{2} \quad 267' \text{ SHALLOW CONC, SWALE, UNPAVED, } s = \frac{5}{267} = 0.0187\% = 2 \text{ MIN.}$$

$$\textcircled{3} \quad 365' \text{ SH. CONC, UNPAVED, } s = \frac{5}{367} = 0.0136\% = 3.2 \text{ MIN.}$$

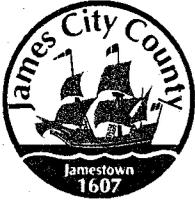
$$\textcircled{4} \quad 653' \text{ SC, UNPAVED, } s = \frac{15}{653} = 0.023\% = 4.4 \text{ MIN.}$$

$$\textcircled{5} \quad 1454' \text{ SC, UNPAVED, } s = \frac{30}{1454} = 0.0206\% = 10.5 \text{ MIN.}$$

$$\textcircled{6} \quad 722' \text{ SC UNPAVED } s = \frac{65-58.1}{722} = 0.0096\% = 7.6 \text{ MIN.}$$

$$\underline{T_C = 37.6 \text{ MIN.}}$$

$$\underline{37.6 \text{ MIN.}}$$



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HYDROLOGIC DATA

SCS Methodology

1" RAINFALL

AREA = 106.36 AC.

CN = 81

T_C = 37.6 MIN. OR 0.6267 hrs.

~~106.36~~

Runoff = 0.10 "

106.36 x 0.10 x 1/12

= 0.88 AC-FT

38,608 CF

		<u>Q PEAK</u>	<u>VOLUME</u>
1-YEAR	2.8"	87.43 CFS	10.297 AC-FT.

2-YEAR	3.5"	130.66 CFS	15.144 AC-FT.
--------	------	------------	---------------

10-YEAR	5.8"	286.51 CFS	32.813 AC-FT.
---------	------	------------	---------------

25-YEAR	6.4"	328.46 CFS	37.67 AC-FT.
---------	------	------------	--------------

100-YEAR	8.0"	441.08 CFS	50.897 AC-FT.
----------	------	------------	---------------

AVAILABLE
 VOLUME TO
 DAM CREST
 5.508 AC. FT.

EXISTING POND ROUTING, NO MODIFICATIONS.

	<u>OUTFLOW</u>	<u>ELEV.</u>
1-YR	87.43	61.56

2-YR	88.45	62.65
------	-------	-------

10-YR	208.60	64.80
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25-YR

100-YR

DAM
 OVERTOPS

DESIGNED
 FOR 10-YEAR !!
 2-YEAR PRE-
 POST CONTROL

Type.... Runoff CN-Area
Name.... CURRENT DA

File.... C:\HAESTAD\PPKW\SAMPLE\FOXRIDGE.PPW
Title... Current DA

RUNOFF CURVE NUMBER DATA

.....

Current DA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C %UC	Adjusted CN
A1 Lots, C soil	77	14.140		77.00
NOS, C soil	70	7.720		70.00
SF Lots, 1/4 ac, C soils	83	84.500		83.00
COMPOSITE AREA & WEIGHTED CN --->		106.360		81.26 (81)

.....

Type.... Tc Calcs
Name.... CURRENT DA

File.... C:\HAESTAD\PPKW\SAMPLE\FOXRIDGE.PPW
Title... Travel Path (Forest Glen, Longhill Sta, FoxRidge)

.....
TIME OF CONCENTRATION CALCULATOR
.....

Travel Path (Forest Glen, Longhill Sta, FoxRidge)

Segment #1: Tc: TR-55 Sheet
Description: Forest Glen

Mannings n .2000
Hydraulic Length 100.00 ft
2yr, 24hr P 3.5000 in
Slope .031600 ft/ft

Avg.Velocity .17 ft/sec

Segment #1 Time: .1637 hrs

Segment #2: Tc: TR-55 Shallow
Description: Natural Channel

Hydraulic Length 267.00 ft
Slope .018700 ft/ft
Unpaved

Avg.Velocity 2.21 ft/sec

Segment #2 Time: .0336 hrs

Segment #3: Tc: TR-55 Shallow
Description: Natural Channel

Hydraulic Length 365.00 ft
Slope .013600 ft/ft
Unpaved

Avg.Velocity 1.88 ft/sec

Segment #3 Time: .0539 hrs

Type.... Tc Calcs
Name.... CURRENT DA

File.... C:\HAESTAD\PPKW\SAMPLE\FOX RIDGE.PPW
Title... Travel Path (Forest Glen, Longhill Sta, FoxRidge)

Segment #4: Tc: TR-55 Shallow
Description: Natural Channel

Hydraulic Length 653.00 ft
Slope .023000 ft/ft
Unpaved

Avg.Velocity 2.45 ft/sec

Segment #4 Time: .0741 hrs

Segment #5: Tc: TR-55 Shallow
Description: Natural Channel

Hydraulic Length 1454.00 ft
Slope .020600 ft/ft
Unpaved

Avg.Velocity 2.32 ft/sec

Segment #5 Time: .1744 hrs

Segment #6: Tc: TR-55 Shallow
Description: Natural Channel

Hydraulic Length 722.00 ft
Slope .009600 ft/ft
Unpaved

Avg.Velocity 1.58 ft/sec

Segment #6 Time: .1269 hrs

=====
Total Tc: .6266 hrs
=====

Type.... Executive Summary (Nodes)
 Name.... Watershed
 File.... C:\HAESTAD\PPKW\SAMPLE\FOXRIDGE.PPW
 Storm... TypeII 24hr Tag: Pre..1

Page 2.01
 Event: 1 yr

NETWORK SUMMARY -- NODES
 (Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File, ID = JCC.RNQ JCCSCSdata

Storm Tag Name = Pre..1
 Description: 1-year event

 Data Type, File, ID = Synthetic Storm SCSTYPES.RNF TypeII 24hr
 Storm Frequency = 1 yr
 Total Rainfall Depth= 2.8000 in
 Duration Multiplier = 1
 Resulting Duration = 24.0000 hrs
 Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Node ID	Type	HYG Vol ac-ft	Trun.	Qpeak hrs	Qpeak cfs	Max WSEL ft
BMP AREA	AREA	10.297		12.3000	87.43	
Outfall BMP OUTFALL	JCT	10.297		12.3000	87.43	

Type.... Executive Summary (Nodes)
 Name.... Watershed
 File.... C:\HAESTAD\PPKW\SAMPLE\FOXRIDGE.PPW
 Storm... TypeII 24hr Tag: Pre..2

Page 2.02
 Event: 2 yr

NETWORK SUMMARY -- NODES
 (Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File, ID = JCC.RNQ JCCSCSdata

Storm Tag Name = Pre..2
 Description: 2-year storm event

 Data Type, File, ID = Synthetic Storm SCSTYPES.RNF TypeII 24hr
 Storm Frequency = 2 yr
 Total Rainfall Depth= 3.5000 in
 Duration Multiplier = 1
 Resulting Duration = 24.0000 hrs
 Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Node ID	Type	HYG Vol ac-ft	Qpeak Trun. hrs	Qpeak cfs	Max WSEL ft
BMP AREA	AREA	15.144	12.3000	130.66	
Outfall BMP OUTFALL	JCT	15.144	12.3000	130.66	

Type.... Executive Summary (Nodes)
 Name.... Watershed
 File.... C:\HAESTAD\PPKW\SAMPLE\FOXRIDGE.PPW
 Storm... TypeII 24hr Tag: Pre.10

Page 2.03
 Event: 10 yr

NETWORK SUMMARY -- NODES
 (Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File, ID = JCC.RNQ JCCSCSdata

Storm Tag Name = Pre.10
 Description: 10-year storm event

 Data Type, File, ID = Synthetic Storm SCSTYPES.RNF TypeII 24hr
 Storm Frequency = 10 yr
 Total Rainfall Depth= 5.8000 in
 Duration Multiplier = 1
 Resulting Duration = 24.0000 hrs
 Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Node ID	Type	HYG Vol ac-ft	Qpeak Trun. hrs	Qpeak cfs	Max WSEL ft
BMP AREA	AREA	32.813	12.2500	286.51	
Outfall BMP OUTFALL	JCT	32.813	12.2500	286.51	

Type.... Executive Summary (Nodes)
 Name.... Watershed
 File.... C:\HAESTAD\PPKW\SAMPLE\FOXRIDGE.PPW
 Storm... TypeII 24hr Tag: Pre.25

Page 2.04
 Event: 25 yr

NETWORK SUMMARY -- NODES
 (Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File, ID = JCC.RNQ JCCSCSdata

Storm Tag Name = Pre.25
 Description: 25-year storm event

 Data Type, File, ID = Synthetic Storm SCSTYPES.RNF TypeII 24hr
 Storm Frequency = 25 yr
 Total Rainfall Depth= 6.4000 in
 Duration Multiplier = 1
 Resulting Duration = 24.0000 hrs
 Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Node ID	Type	HYG Vol ac-ft	Qpeak Trun. hrs	Qpeak cfs	Max WSEL ft
BMP AREA	AREA	37.670	12.2500	328.46	
Outfall BMP OUTFALL	JCT	37.670	12.2500	328.46	

Type.... Executive Summary (Nodes)
 Name.... Watershed
 File.... C:\HAESTAD\PPKW\SAMPLE\FOXRIDGE.PPW
 Storm... TypeII 24hr Tag: Pre100

Page 2.05
 Event: 100 yr

NETWORK SUMMARY -- NODES
 (Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File, ID = JCC.RNQ JCCSCSdata

Storm Tag Name = Pre100
 Description: 100-year storm event

 Data Type, File, ID = Synthetic Storm SCSTYPES.RNF TypeII 24hr
 Storm Frequency = 100 yr
 Total Rainfall Depth= 8.0000 in
 Duration Multiplier = 1
 Resulting Duration = 24.0000 hrs
 Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Node ID	Type	HYG Vol ac-ft	Qpeak Trun. hrs	Qpeak cfs	Max WSEL ft
BMP AREA	AREA	50.897	12.2500	441.06	
Outfall BMP OUTFALL	JCT	50.897	12.2500	441.06	

POINT	PLUS	HI	MINUS	ELEV	NOTES
pump sta step				65.7	assumed
	6.9	72.6			
tp / gate			3.75	68.85	
tp/dam			7.33	65.27	
tp/dam	4.63	69.9			
		69.9			
south wall top of dam		69.9	4.63	65.27	
		69.9		69.9	19.5'
sw toe of slope		69.9	11.28	58.62	
		69.9		69.9	16'
sw corner		69.9	10.67	59.23	
		69.9		69.9	28' swcor to nwcor
south invert in		69.9	11.8	58.1	
		69.9		69.9	
south inlet top		69.9	6.83	63.07	
		69.9		69.9	
north inlet in		69.9	11.76	58.14	
		69.9		69.9	
north inlet top		69.9	6.99	62.91	
		69.9		69.9	
northwest corner		69.9	11.48	58.42	
		69.9		69.9	18'
n/wall toe of slope		69.9	11.62	58.28	
		69.9		69.9	17'
north wall top of dam		69.9	4.98	64.92	
		69.9		69.9	
emergency spillway		69.9	6.2	63.7	
		69.9		69.9	
north inv out		69.9	13.2	56.7	
		69.9		69.9	
south inv out		69.9	13.32	56.58	
tp/gate	1.5	70.35		68.85	
		70.35			
transformer base		70.35	4.98	65.37	
		70.35		70.35	
ne cor lot 50		70.35	8.76	61.59	

= 58
 + 3

 61

= 65
 = 24

= 65.4
 61.6

FOX RIDGE RETROFIT

PER ASBVILT SURVEY

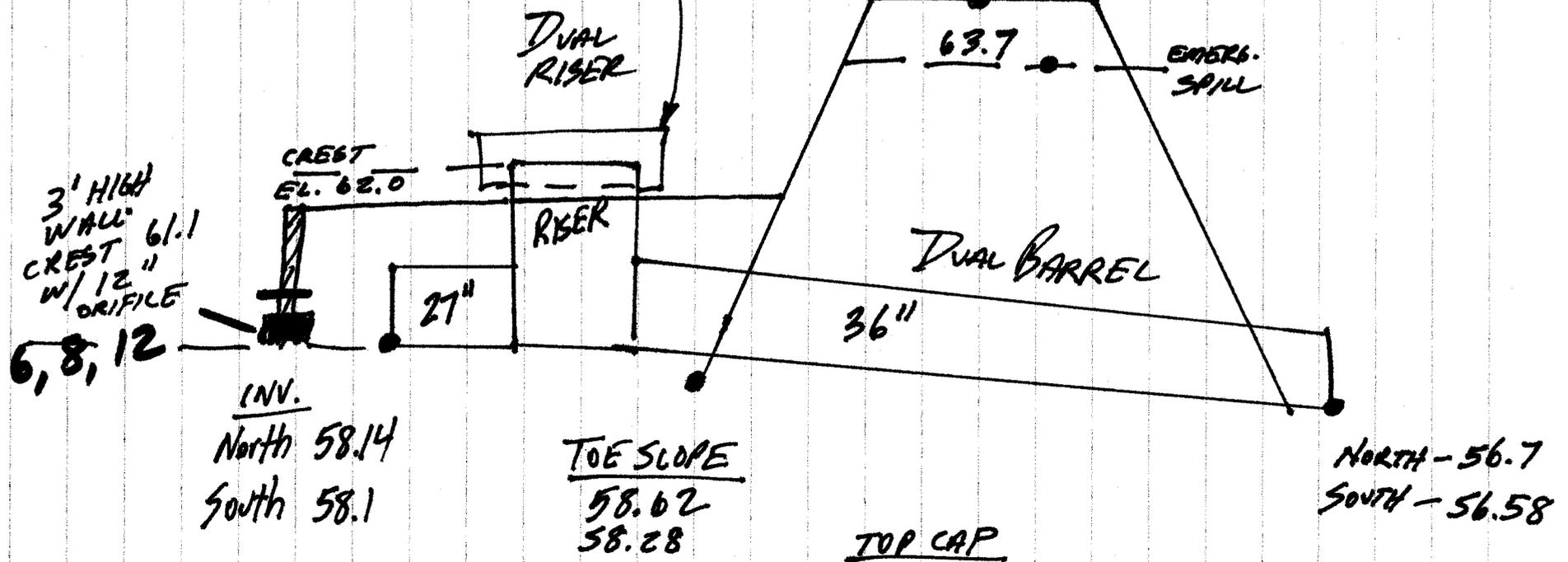
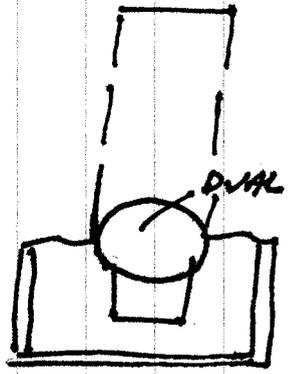
Area

58.1	0
60.0	0.41 AC. (17859 SF)
62.0	0.92 AC. (40,075 SF)
64.0	1.46 AC. (63,597 SF)
65.0 ^{TOP} _{DAM}	1.73 AC. (75,358 SF)*

* ESTIMATED.

RIVER TOP
 North - 62.91
 South - 63.07

TOP DAM
 65.27
 65.27
 64.92 (LOW) SAY 65.0

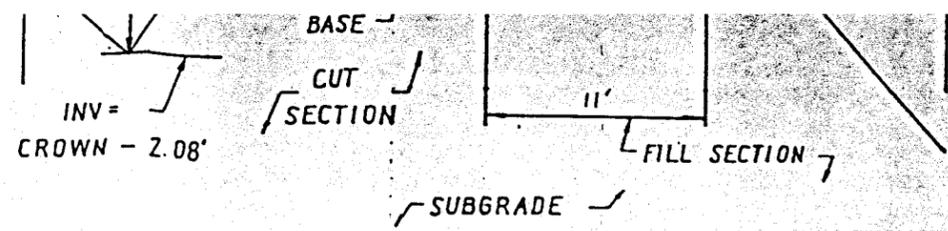


$$\frac{3}{1} = \frac{x - 0.92}{0.54}$$

TOP CAP
 48" TOP
 84" CAP
 2' DEEP } EL 61.50
 ±0.5' } USE EL 62.0

CATEGORY I

1. PROVIDE 2-11' LANES, PAVEMENT WIDTH - 20'
2. PROVIDE 6" SHOULDER WIDTH, SLOPE - 3/4" / FOOT
3. SUBGRADE - CBR OF 10 OR GREATER
4. BASE - 10" OF AGGREGATE BASE MATERIAL (TYPE I OR TYPE II) OR 4" OF BITUMINOUS CONCRETE (TYPE B-1)
5. SURFACE - 1 1/2" BITUMINOUS CONCRETE (S-5) OR PRIME AND DOUBLE SEAL
6. SIDE SLOPES - 6" PER FT. (2:1) MAXIMUM



TYPICAL SECTION - CATEGORY I, 50' R/W

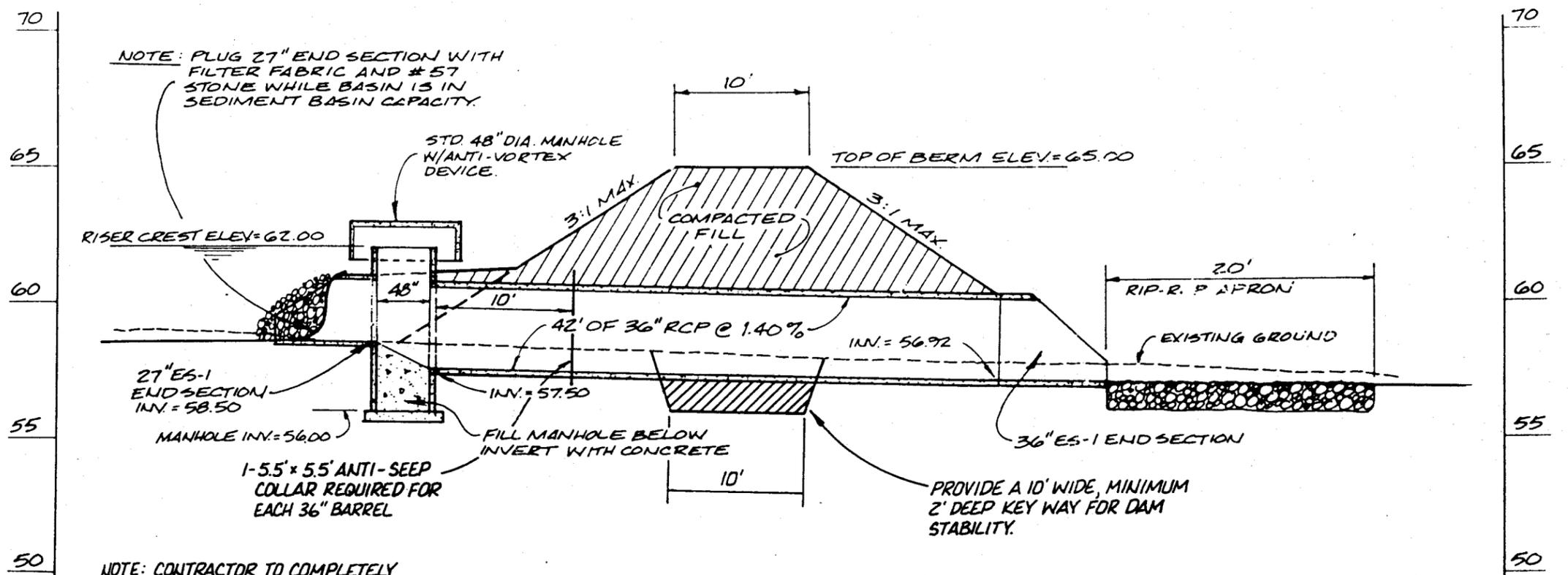
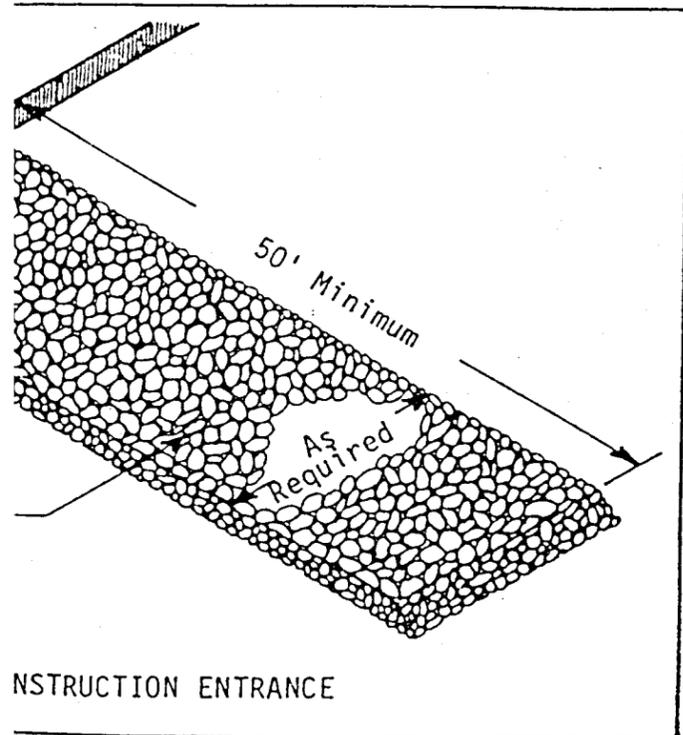
SCALE 1" = 10' HORIZONTAL, 1" = 3' VERTICAL

1. PROVIDE 2-10' LANES, PAVEMENT WIDTH - 20', SLOPE - 1/4" / FOOT.
2. PROVIDE 6" SHOULDER WIDTH, SLOPE - 3/4" / FOOT.
3. SUBGRADE - CBR OF 10 OR GREATER.
4. BASE 6" OF AGGREGATE, BASE MATERIAL (TYPE I OR TYPE II) OR 4" OF CRUSHER RUN, & 3" OF BITUMINOUS CONCRETE (TYPE B-1)
5. SURFACE - 1 1/2" BITUMINOUS CONCRETE (S-5) OR PRIME AND DOUBLE SEAL.
6. SIDE SLOPES - 6" PER FOOT (2:1) MAXIMUM.

RISER CAP
 RISER 48"
 CAP 84"
 2' DEPTH
 + 0.5' = EL. 61.50
 USE CREST EL. 62.0

ication
 n is applicable where the inlet
 es no greater than 5 percent) where
 ing 0.5 cfs) are typical. The
 ceiving concentrated flows, such

T SEDIMENT FILTER



NOTE: PLUG 27" END SECTION WITH FILTER FABRIC AND #57 STONE WHILE BASIN IS IN SEDIMENT BASIN CAPACITY.

NOTE: CONTRACTOR TO COMPLETELY CLEAR, GRUB AND REMOVE UNSUITABLE MATERIAL FROM DAM AREA PRIOR TO PLACING FILL MATERIAL.

DETENTION BASIN SPILLWAY SECTION A - A

SCALE:
 1" = 5' VERTICAL
 1" = 10' HORIZONTAL

NOTE: DUAL INSTALLATION REQUIRED (SEE PLAN VIEW)

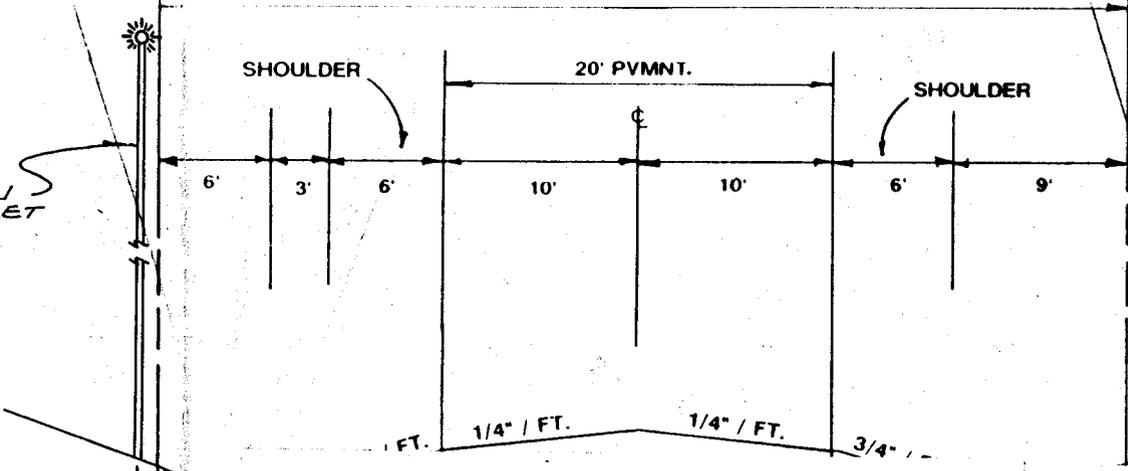
$$\begin{pmatrix} 62 & 0.92 \\ 64 & 1.46 \\ 65 & x \end{pmatrix}$$

$$\frac{65-62}{64-62} = \frac{x-0.92}{1.46-0.92}$$

$$\frac{3}{2} = \frac{x-0.92}{0.54}$$

$$2x - 1.84 = 1.62$$

$$x = 1.73 \text{ ac.}$$



N 4500

JCC. & VDOT. COMMENTS
 S. VDOT. & VDOT.H. COMMENTS

REVISION / COMMENT / NOTE

ELLEV	AREA	VOLUME
36' CC	0.43 0.41 0.41	(111V = 59) = 0.2 Ac Ft
	0.41	0.41 Ac x 1/12" = 0.2 Ac Ft
62	0.92 0.93	0.92 Ac x 2 Ft = 1.8 Ac Ft
64	1.46 1.46	1.46 Ac x 2 Ft = 2.9 Ac Ft
		TOTAL = 5 Ac Ft
		2.1 Ac x 2 Ft = 4 Ac Ft
		DR - SHC DITCH @ 68 -

TYPIC

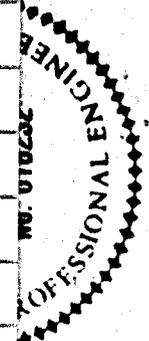
REMARKS ONLY SPILLWAY - 64.

TOP OF DAM @ 5

pumping sta. FF @ 66

66. 2.13
 2.01 2.1

AS BUILT INFO SHOWS CUR -
 S.S. MH TOP @ 62 WTF @



Surveying

File.... C:\HAESTAD\PPKW\SAMPLE\FOXRIDGE.PPW
Title... Exist PC 003

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
58.10	-----	.0000	.0000	.000	.000
60.00	-----	.4100	.4100	.260	.260
62.00	-----	.9200	1.9442	1.296	1.556
64.00	-----	1.4600	3.5390	2.359	3.915
65.00	-----	1.7300	4.7793	1.593	5.508

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2}-\text{EL1}) * (\text{Area1} + \text{Area2} + \text{sq.rt.}(\text{Area1}*\text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
Area1,Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

Type.... Executive Summary (Nodes)
 Name.... Watershed
 File.... C:\HAESTAD\PPKW\JCCPROJECTS\FOX RIDGE.PPW
 Storm... TypeII 24hr Tag: A....1

Page 3.01
 Event: 1 yr

NETWORK SUMMARY -- NODES
 (Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File, ID = JCC.RNQ JCCSCSdata

Storm Tag Name = A....1
 Description: 1-year event

 Data Type, File, ID = Synthetic Storm SCSTYPES.RNF TypeII 24hr
 Storm Frequency = 1 yr
 Total Rainfall Depth= 2.8000 in
 Duration Multiplier = 1
 Resulting Duration = 24.0000 hrs
 Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Node ID		Type	HYG Vol ac-ft	Qpeak Trun. hrs	Qpeak cfs	Max WSEL ft
-----		-----	-----	-----	-----	-----
Outfall	CURRENT DA	AREA	10.297	12.3000	87.43	
	OUTFALL	JCT	10.297	12.6500	47.56	
	POND	IN POND	10.297	12.3000	87.43	
	POND	OUT POND	10.297	12.6500	47.56	61.56

Type.... Executive Summary (Nodes)
 Name.... Watershed
 File.... C:\HAESTAD\PPKW\JCCPROJECTS\FOXTRIDGE.PPW
 Storm... TypeII 24hr Tag: A....2

Page 3.03
 Event: 2 yr

NETWORK SUMMARY -- NODES
 (Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File, ID = JCC.RNQ JCCSCSdata

Storm Tag Name = A....2
 Description: 2-year storm event

 Data Type, File, ID = Synthetic Storm SCSTYPES.RNF TypeII 24hr
 Storm Frequency = 2 yr
 Total Rainfall Depth= 3.5000 in
 Duration Multiplier = 1
 Resulting Duration = 24.0000 hrs
 Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Node ID		Type	HYG Vol ac-ft	Qpeak Trun. hrs	Qpeak cfs	Max WSEL ft
-----		---	---	---	---	-----
	CURRENT DA	AREA	15.144	12.3000	130.66	
Outfall	OUTFALL	JCT	15.144	12.5500	88.45	
	POND	IN POND	15.144	12.3000	130.66	
	POND	OUT POND	15.144	12.5500	88.45	62.65

Type.... Executive Summary (Nodes)
 Name.... Watershed
 File.... C:\HAESTAD\PPKW\JCCPROJECTS\FOXTRIDGE.PPW
 Storm... TypeII 24hr Tag: A...10

Page 3.05
 Event: 10 yr

NETWORK SUMMARY -- NODES
 (Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File, ID = JCC.RNQ JCCSCSdata

Storm Tag Name = A...10
 Description: 10-year storm event

 Data Type, File, ID = Synthetic Storm SCSTYPES.RNF TypeII 24hr
 Storm Frequency = 10 yr
 Total Rainfall Depth= 5.8000 in
 Duration Multiplier = 1
 Resulting Duration = 24.0000 hrs
 Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Node ID	Type	HYG Vol ac-ft	Qpeak Trun. hrs	Qpeak cfs	Max WSEL ft
-----	-----	-----	-----	-----	-----
Outfall CURRENT DA	AREA	32.813	12.2500	286.51	
OUTFALL	JCT	32.813	12.5000	208.60	
POND	IN POND	32.813	12.2500	286.51	
POND	OUT POND	32.813	12.5000	208.60	64.80

VOLUME
(INV \approx 59)

$$0.4 \text{ Ac} \times \frac{1}{12} = 0.2 \text{ Ac Ft}$$

$$0.92 \text{ Ac} \times 2 \text{ FT} = 1.8 \text{ Ac Ft}$$

$$1.46 \text{ Ac} \times 2 \text{ FT} = \underline{2.9 \text{ Ac Ft}}$$

$$\text{TOTAL} \approx \underline{5 \text{ Ac Ft}}$$

$$2.1 \text{ Ac} \times 2 \text{ FT} = \underline{4 \text{ Ac Ft}}$$

DIE-SHED DITCH @ 68 -

ELEV	INV	AREA
60	0.43	
	0.41	
	0.41	

0.41

62	0.92	
	0.90	
	0.93	

0.92

EMERGENCY SPILLWAY - 64.

64	1.46	
	1.46	

1.46

TOP OF DAM @ 5

Pumping Sta FF @ 66 -

66	2.13	
	2.01	

2.1

AS BUILT INFO SHOWS CAN -

SS MH TOP @ 62 WTF @

DITCH CALCULATIONS

STA.	D.A. (FE.)	Q ₂ (LPS)	SLOPE (%)	V _L (FPS)	LINING TYPE	R ₁₀ (LPS)	D ₁₀ (IN)	DESCRIPTION
24+70 LT	0.12	0.200	1.00	0.85	GRASS	0.20	4.60	GRASS V-DITCH 4/2' 1.53, 12" DEEP
25+00 RT	5.00	4.05	1.00	1.85	GRASS	5.25	15.00	TRANSITION
				4.50	PAVED		9.00	GRASS V-DITCH 4/2' 1.53, 12" DEEP TO PAVED V-DITCH 4/2' 1.53, 12" DEEP
30+50 LT	0.31	0.49	4.00	1.80	GRASS	0.65	5.00	GRASS V-DITCH 4/2' 1.53, 12" DEEP
30+50 RT	3.16	3.32	4.00	3.00	GRASS	4.20	10.00	GRASS V-DITCH 4/2' 1.53, 12" DEEP
30+70 LT	0.21	0.33	3.00	1.40	GRASS	0.43	4.60	GRASS V-DITCH 4/2' 1.53, 12" DEEP
30+70 RT	0.87	0.91	3.00	1.90	GRASS	1.18	6.70	GRASS V-DITCH 4/2' 1.53, 12" DEEP
RD. F Red Fox Circle								
10+20 RT	0.21	0.33	2.50	1.45	GRASS	0.44	4.80	GRASS V-DITCH 4/2' 1.53, 12" DEEP
10+20 LT	0.55	0.87	2.50	1.80	GRASS	1.10	6.90	GRASS V-DITCH 4/2' 1.53, 12" DEEP

DITCH CALCULATIONS

STA.	D.A. (ft.)	Q ₂ (CFS)	SLOPE (%)	V _L (FPS)	LINING TYPE	R ₁₀ (CFS)	D ₁₀ (IN)	DESCRIPTION
RD "D"								
12+00 RT	2.81	2.95	1.00 276	1.70 6.25	GRASS PAVED	3.80	12.50 6.60	TRANSITION GRASS V-DITCH W/2:1SS, 12" DEEP PAVED V-DITCH W/2:1SS, 12" DEEP
RD "E"	Fox RW							
11+70 LT	0.12	0.18	<u>6.53</u>	4.25	PAVED	0.25	2.00	PAVED V-DITCH W/2:1SS, 12" DEEP
11+70 RT	0.96	1.01	<u>6.53</u>	6.50	PAVED	1.30	3.80	PAVED V-DITCH W/2:1SS, 12" DEEP
11+80 LT	0.12	0.20	1.20	0.95	GRASS	0.26	4.30	GRASS V-DITCH W/2:1SS, 12" DEEP
11+80 RT	1.63	1.71	1.20	1.70	GRASS	2.20	7.80	GRASS V-DITCH W/2:1SS, 12" DEEP
17+00 LT	0.23	0.37	<u>6.00</u>	4.90	PAVED	0.49	2.80	PAVED V-DITCH W/2:1SS, 12" DEEP
17+00 RT	1.85	2.91	<u>6.00</u>	8.25	PAVED	3.88	5.90	PAVED V-DITCH W/2:1SS, 12" DEEP
18+75 LT	0.44	0.62	2.60	4.50	PAVED	0.79	3.80	PAVED V-DITCH W/2:1SS, 12" DEEP
18+75 RT	2.58	3.10	2.60	6.00	PAVED	3.95	6.90	PAVED V-DITCH W/2:1SS, 12" DEEP
19+10 LT	0.10	0.16	2.13	1.15	GRASS	0.32	3.80	GRASS V-DITCH W/2:1SS, 12" DEEP
19+10 RT	0.43	0.68	2.13	1.55	GRASS	2.91	6.50	GRASS V-DITCH W/2:1SS, 12" DEEP
22+50 LT	0.14	0.23	<u>7.00</u>	4.70	PAVED	0.31	2.90	PAVED V-DITCH W/2:1SS, 12" DEEP
23+20 RT	0.80	1.08	<u>7.00</u>	6.70	PAVED	1.04	4.00	PAVED V-DITCH W/2:1SS, 12" DEEP
24+50 RT	10.10	8.18	4.50	9.60	PAVED	10.61	9.00	PAVED V-DITCH W/2:1SS, 12" DEEP
24+50 LT	30.19	24.45	4.50	13.00	PAVED	31.70	14.00	PAVED V-DITCH W/2:1SS, 12" DEEP

DITCH CALCULATIONS

STA.	D.A (%)	Q _L (CFS)	SLOPE (%)	V _L (FPS)	LINING TYPE	Q ₁₀ (CFS)	D ₁₀ (IN)	DESCRIPTION
RD "B" - Fox Hunt Tr.								
15+50 RT	0.16	0.24	5.00	1.60	GRASS	0.33	3.80	GRASS V-DITCH w/2:1 SS, 12" D&D
17+50 RT	0.29	0.91	3.00	1.60	GRASS	0.53	4.80	GRASS V-DITCH w/2:1 SS, 12" D&D
15 5+50 LT	3.16	3.31	5.00	3.30	GRASS	4.26	9.60	GRASS V-DITCH w/2:1 SS, 12" D&D
19 9+50 LT	6.43	5.21	3.00	3.00	GRASS	6.75	12.00	TRANSITION TO PAVED V-DITCH w/2:1 SS, 12" D&D
				7.25	PAVED		8.00	
22+00 LT	11.13	9.02	3.00	8.50	PAVED	11.69	9.90	PAVED V-DITCH w/2:1 SS, 12" D&D
22+00 RT	0.28	0.44	3.00	1.60	GRASS	0.58	5.10	GRASS V-DITCH w/2:1 SS, 12" D&D
RD "C" - Grey Fox Ct.								
11 15 LT	0.20	0.31	1.02	0.98	GRASS	0.41	5.50	GRASS V-DITCH w/2:1 SS, 12" D&D
12 15 LT	1.03	1.63	1.02	1.40	GRASS	2.17	10.00	GRASS V-DITCH w/2:1 SS, 12" D&D
13 100 LT	0.14	0.23	4.00	1.50	GRASS	0.31	3.80	GRASS V-DITCH w/2:1 SS, 12" D&D
13 100 RT	0.24	0.38	4.00	1.70	GRASS	0.51	4.60	GRASS V-DITCH w/2:1 SS, 12" D&D
RD "D" - Fox Chase								
15 100 LT	1.43	2.00	3.50	2.40	GRASS	2.56	8.50	GRASS V-DITCH w/2:1 SS, 12" D&D
15 100 RT	2.07	2.89	3.50	2.70	GRASS	3.69	10.00	GRASS V-DITCH w/2:1 SS, 12" D&D
12 100 LT	2.70	2.82	1.00	1.70	GRASS	3.63	12.00	TRANSITION GRASS V-DITCH w/2:1 SS, 12" D&D TO
			3.00	6.00	PAVED		6.90	PAVED V-DITCH w/2:1 SS, 12" D&D

DITCH CALCULATIONS

STA.	D.A. (ft.)	Q ₂ (cfs)	SLOPE (%)	V _L (FPS)	LINING TYPE	Q ₁₀ (cfs)	D ₁₀ (IN)	DESCRIPTION
RD "A"								
12+50 LT	0.17	0.23	1.00	0.88	GRASS	0.31	4.90	GRASS V-DITCH w/2:1 SS, 12" DEEP
12+50 RT	1.15	1.81	1.00	1.50	GRASS	2.41	10.20	GRASS V-DITCH w/2:1 SS, 12" DEEP
13+50 RT	1.38	2.17	4.00	2.60	GRASS	2.89	8.70	TRANSITION GRASS V-DITCH w/2:1 SS, 12" DEEP TO
			<u>7.00</u>	8.00	PAVED		4.90	PAVED V-DITCH w/2:1 SS, 12" DEEP
18+75 RT	3.40	4.23	2.17	6.35	PAVED	5.43	7.90	PAVED V-DITCH w/2:1 SS, 12" DEEP
18+75 LT	0.48	0.67	2.17	4.00	PAVED	0.86	4.00	PAVED V-DITCH w/2:1 SS, 12" DEEP
19+50 RT	3.15	3.85	<u>6.00</u>	8.80	PAVED	4.95	6.20	PAVED V-DITCH w/2:1 SS, 12" DEEP
19+50 LT	0.75	1.18	<u>6.00</u>	6.50	PAVED	1.58	4.20	PAVED V-DITCH w/2:1 SS, 12" DEEP
RD "B"								
11+50 LT	1.03	1.45	2.06	1.90	GRASS	1.84	8.50	TRANSITION GRASS V-DITCH w/2:1 SS, 12" DEEP TO
			<u>7.00</u>	7.20	PAVED		4.30	PAVED V-DITCH w/2:1 SS, 12" DEEP
11+50 RT	0.09	0.14	2.06	1.10	GRASS	0.18	3.60	TRANSITION GRASS V-DITCH w/2:1 SS, 12" DEEP TO
			<u>7.00</u>	4.00	PAVED		1.80	PAVED V-DITCH w/2:1 SS, 12" DEEP
13+20 LT	4.85	4.37	<u>7.00</u>	10.00	PAVED	5.82	6.60	PAVED V-DITCH w/2:1 SS, 12" DEEP
13+90 LT	1.07	0.96	2.00	1.60	GRASS	1.26	7.30	GRASS V-DITCH w/2:1 SS, 12" DEEP
13+20 RT	0.22	0.36	<u>7.00</u>	5.00	PAVED	0.48	2.70	PAVED V-DITCH w/2:1 SS, 12" DEEP
13+90 RT	0.06	0.09	2.00	0.90	GRASS	0.12	3.00	GRASS V-DITCH w/2:1 SS, 12" DEEP

OUTFALL DITCHOUTFALL DITCH BETWEEN LOTS 24925

FROM PREVIOUS CALCULATION OF CULVERT "2"

$Q_2 = 9.07 \text{ CFS}$

$Q_{10} = 11.76 \text{ CFS}$

USE 190' OF PAVED V-DITCH W/2:1 SS, 12" DWHP @ 4.00%

INV @ CULVERT "2" = 81.00'

INV. OUT = 73.90'

$V_2 = 7.0 \text{ CFS}$

$D_{10} = 9.7 \text{ IN.}$

OUTFALL DITCH BETWEEN LOTS 18419 @ CUL-DE-SAC 20' B"

$P.A. = 11.41 \text{ AC}$

$C = 0.30$

$T_c = 30 \text{ MIN.}$

$I_2 = 2.7 \text{ IN/HR}$

$I_o = 3.5 \text{ IN/HR.}$

$Q_2 = (0.30)(2.7)(11.41) = 9.24 \text{ CFS}$

$Q_{10} = (0.30)(3.5)(11.41) = 11.98 \text{ CFS}$

USE 110' OF PAVED V-DITCH W/2:1 SS, 12" DWHP @ 5.75%

INV @ CUL-DE-SAC = 72.30'

INV. OUT = 66.00'

$V_2 = 10.5 \text{ FPS}$

$D_{10} = 9.00 \text{ IN.}$

CULVERTSCULVERT #9

$$D.A. = 4.03 \text{ A.C.} \quad C = 0.30 \quad T_c = 20 \text{ MIN}$$

$$I_2 = 3.5 \text{ IN/HR} \quad I_0 = 4.5 \text{ IN/HR}$$

$$Q_2 = (0.30)(3.5)(4.03) = 4.23 \text{ CFS}$$

$$Q_0 = (0.30)(4.5)(4.03) = 5.44 \text{ CFS}$$

TRY 52' OF 15" @ 6.73%

INLET HW/D = 1.15 HW = 1.44

OUTLET H = 1.25 $f_o = \frac{0.95 + 1.25}{2} = 1.10$ $L_s = (52)(0.0673) = 3.50'$

$$HW = 1.25 + 1.10 \cdot 3.50 = \text{N.A.} \quad \text{INLET CONTROLS}$$

USE 52' OF 15" RCP @ 6.73%

INV. IN = 68.50'

INV. OUT = 65.00'

CULVERT #10

$$D.A. = 17.78 \text{ A.C.} \quad C = 0.25 \quad T_c = 30 \text{ MIN}$$

$$I_2 = 2.7 \text{ IN/HR} \quad I_0 = 3.5 \text{ IN/HR}$$

$$Q_2 = (0.25)(2.7)(17.78) = 12.00 \text{ CFS}$$

$$Q_0 = (0.25)(3.5)(17.78) = 15.56 \text{ CFS}$$

TRY 48' OF 24" RCP @ 1.00%

INLET HW/D = 1.12 HW = 2.24

OUTLET H = 0.8 $f_o = \frac{1.4 + 2.0}{2} = 1.70$ $L_s = (48)(0.01) = 0.48'$

$$HW = 0.8 + 1.70 \cdot 0.48 = 2.02 \quad \text{INLET CONTROLS}$$

USE 48' OF 24" RCP @ 1.00%

INV. IN = 73.35'

INV. OUT = 72.87'

CULVERTSCULVERT #7

$$D.A. = 0.43 \text{ ft} \quad C = 0.35 \quad T_c = 10 \text{ MIN}$$

$$I_2 = 4.6 \text{ in/hr} \quad I_{10} = 6.0 \text{ in/hr}$$

$$Q_2 = (0.35)(4.6)(0.43) = 0.69 \text{ CFS}$$

$$Q_{10} = (0.35)(6.0)(0.43) = 0.91 \text{ CFS}$$

TRY 64' OF 15" RCP @ 1.00%

INLET HW/D = 0.40 HW = 0.50

OUTLET H = N/A $f_0 = \frac{0.45 + 1.25}{2} = 0.85$ $LS = (64)(.01) = 0.64$

$$HW = 0.85 - 0.64 = 0.21 \quad \text{INLET CONTROLS}$$

USE 64' OF 15" RCP @ 1.00%

INV. IN = 89.74'

INV. OUT = 89.10'

CULVERT #8

$$D.A. = 0.69 \text{ ft} \quad C = 0.35 \quad T_c = 10 \text{ MIN}$$

$$I_2 = 4.6 \text{ in/hr} \quad I_{10} = 6.0 \text{ in/hr}$$

$$Q_2 = (0.35)(4.6)(0.69) = 1.11 \text{ CFS}$$

$$Q_{10} = (0.35)(6.0)(0.69) = 1.45 \text{ CFS}$$

TRY 64' OF 15" RCP @ 3.05%

INLET HW/D = 0.54 HW = 0.68

OUTLET H = N/A $f_0 = \frac{0.45 + 1.25}{2} = 0.85$ $LS = (64)(.0305) = 1.95$

$$HW = 0.85 - 1.95 = \text{N.A.} \quad \text{INLET CONTROLS}$$

USE 64' OF 15" RCP @ 3.05%

INV. IN = 79.95'

INV. OUT = 78.00'

CULVERTSCULVERT #5

$$D.A. = 2.59 \text{ AC} \quad L = 0.30 \quad T_c = 20 \text{ MIN}$$

$$I_2 = 3.5 \text{ IN/HR} \quad I_{10} = 4.5 \text{ IN/HR}$$

$$Q_2 = (0.30)(3.5)(2.59) = 2.72 \text{ CFS}$$

$$Q_{10} = (0.30)(4.5)(2.59) = 3.50 \text{ CFS}$$

TRY 60' OF 15" RCP @ 4.17%

INLET $HN/D = 0.86$ $HN = 1.08$

OUTLET $H = 0.35$ $h_0 = \frac{1.25 + 0.75}{2} = 1.0$ $LS_0 = (60)(0.0417) = 2.5$

$$HN = 0.35 + 1.0 - 2.5 = \text{N.A.} \quad \text{INLET CONTROLS}$$

USE 60' OF 15" RCP @ 4.17%

$$\text{INV. IN} = 105.00'$$

$$\text{INV. OUT} = 102.50'$$

CULVERT #6

$$D.A. = 2.58 \text{ AC} \quad L = 0.35 \quad T_c = 15 \text{ MIN}$$

$$I_2 = 4.0 \text{ IN/HR} \quad I_{10} = 5.1 \text{ IN/HR}$$

$$Q_2 = (0.35)(4.0)(2.58) = 3.61 \text{ CFS}$$

$$Q_{10} = (0.35)(5.1)(2.58) = 4.61 \text{ CFS}$$

TRY 64' OF 15" RCP @ 1.00%

INLET $HN/D = 1.05$ $HN = 1.31$

OUTLET $H = 0.60$ $h_0 = \frac{0.85 + 1.25}{2} = 1.05$ $LS_0 = (64)(.01) = 0.64$

$$HN = 0.60 + 1.05 - 0.64 = 1.01 \quad \text{INLET CONTROLS}$$

USE 64' OF 15" RCP @ 1.00%

$$\text{INV. IN} = 89.74'$$

$$\text{INV. OUT} = 89.10'$$

CULVERTS

CULVERT #3

D.A. = 7.91 M.S. C = 0.35 T_c = 20 MIN
 T₂ = 3.5 IN/HR. I₁₀ = 4.5 IN/HR.

$Q_2 = (0.35)(3.5)(7.91) = 9.69 \text{ CFS}$

$Q_{10} = (0.35)(4.5)(7.91) = 12.46 \text{ CFS}$

TRY 64' OF 24" RCP @ 2.73%

INLET HW/D = 0.93 HW = 1.86

OUTLET H = 0.55 $f_0 = \frac{2+1.2}{2} = 1.6$ $L_s = (64)(0.0273) = 1.75$

$HW = 0.55 + 1.6 - 1.75 = 0.40$ INLET CONTROLS

USE 64' OF 24" RCP @ 2.73%

INLET INV. 80.25'

OUTLET INV. 78.50'

CULVERT #4

D.A. = 21.67 M.S. C = 0.30 T_c = 30 MIN
 T₂ = 2.7 IN/HR I₁₀ = 3.5 IN/HR.

$Q_2 = (0.30)(2.7)(21.67) = 17.55 \text{ CFS}$

$Q_{10} = (0.30)(3.5)(21.67) = 22.75 \text{ CFS}$

TRY 60' OF 30" RCP @ 1.67%

INLET HW/D = 0.95 HW = 2.38

OUTLET H = 0.70 $f_0 = \frac{1.5+2.5}{2} = 2.00$ $L_s = (60)(0.0167) = 1.00$

$HW = 0.70 + 2.00 - 1.00 = 1.70$ INLET CONTROLS

USE 60' OF 30" RCP @ 1.67%

INLET INV. 78.50'

OUTLET INV. 77.50'

CULVERTS

CULVERT #1

D.A. = 5.92 A.C. = C = 0.80 T_L = 25 MIN
I₂ = 3.0 IN/HR. I₁₀ = 4.0 IN/HR.

Q₂ = (0.30)(3.0)(5.92) = 5.33 CFS

Q₁₀ = (0.30)(4.0)(5.92) = 7.10 CFS

TRY 40' OF 18" RCP @ 2.00%

INLET HW/D = 1.02 HW = 1.58

OUTLET H = 0.52 S₀ = $\frac{1.5+1.1}{2} = 1.3$ L_S = 40(0.020) = 0.80

HW = 0.52 + 1.3 - 0.80 = 1.02 INLET CONTROLS

USE 40' OF 18" RCP @ 2.00%

INV. IN = 98.30'

INV. OUT = 97.50'

CULVERT #2

D.A. = 9.60 ft. C = 0.35 T_L = 30 MIN
I₂ = 2.7 IN/HR. I₁₀ = 3.5 IN/HR.

Q₂ = (0.35)(2.7)(9.60) = 9.07 CFS

Q₁₀ = (0.35)(3.5)(9.60) = 11.76 CFS

TRY 40' OF 24" RCP @ 1.00%

INLET HW/D = 0.90 HW = 1.80

OUTLET H = 0.56 S₀ = $\frac{2+1.2}{2} = 1.6$ L_S = (40)(.01) = 0.40

HW = 0.56 + 1.6 - 0.40 = 1.76 INLET CONTROLS

USE 40' OF 24" RCP @ 1.00%

INV. IN = 81.40'

INV. OUT = 81.00'

JCC COPY

FOX RIDGE

#7105

BY

ABS, A PROFESSIONAL CORPORATION

DRAINAGE CALCULATIONS

GAM/HWP

PC 003

FILL SECTION 7

SUBGRADE

23
38.2
A'S

TYPICAL SECTION - CATEGORY I, 50' RIW
SCALE 1" = 10' HORIZONTAL, 1" = 3' VERTICAL

1. PROVIDE 2-10' LANES, PAVEMENT WIDTH - 20', SLOPE = 1/4" / FOOT.
2. PROVIDE 6' SHOULDER WIDTH, SLOPE = 3/4" / FOOT.
3. SUBGRADE - CBR OF 10 OR GREATER.
4. BASE 6" OF AGGREGATE; BASE MATERIAL (TYPE I OR TYPE II) OR 4" OF CRUSHER RUN, & 3" OF BITUMINOUS CONCRETE (TYPE B-1).
5. SURFACE - 1 1/2" BITUMINOUS CONCRETE (S-S) OR PRIME AND DOUBLE SEAL.
6. SIDE SLOPES - 6" PER FOOT (2:1) MAXIMUM.

1. PROVIDE 2-11' LANES, PAVEMENT WIDTH
2. PROVIDE 6' SHOULDER WIDTH, SLOPE =
3. SUBGRADE - CBR OF 10 OR GREATER.
4. BASE - 10" OF AGGREGATE BASE MATERIAL OR 4" OF BITUMINOUS CONCRETE (TYPE B-1)
5. SURFACE - PRIME AND DOUBLE SEAL OR
6. SIDE SLOPES - 6" PER FT. (2:1) MAXIMUM

tion
s applicable where the inlet
no greater than 5 percent) where
0.5 cfs) are typical. The
ring concentrated flows, such

SEDIMENT FILTER

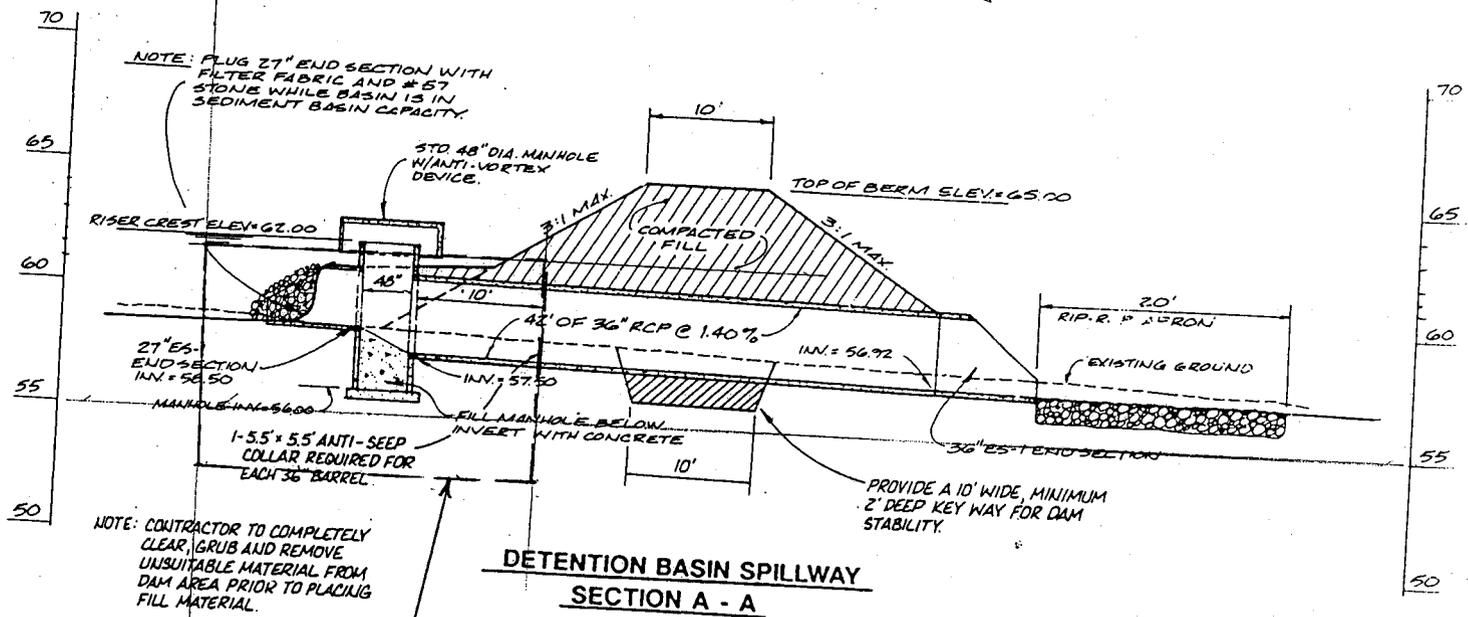
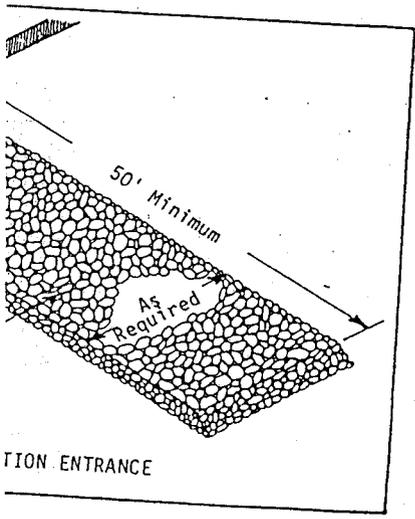


EXHIBIT 4
28' X 28' PLASTIC SHEET PILE BULKHEAD
TOP ELEV 61.5; 6' BURY; INSTALL VDOT
CLASS I RIP-RAP 12" THICK ON FABRIC
INSIDE BULKHEAD; INSTALL 6" DIAMETER
LOW FLOW PIPE AT ELEV 58.5

SCALE:
1" = 5' VERTICAL
1" = 10' HORIZONTAL
NOTE: DUAL INSTALLATION REQUIRED (SEE PLAN VIEW)

U. S. ARMY CORPS OF ENGINEERS
REQUIRE
APR 17 2003
BY

POINT	PLUS	HI	MINUS	ELEV	NOTES
pump sta step				65.7	assumed
	6.9	72.6			
tp / gate			3.75	68.85	
tp/dam			7.33	65.27	
tp/dam	4.63	69.9			
		69.9			
south wall top of dam		69.9	4.63	65.27	
		69.9		69.9	19.5'
sw toe of slope		69.9	11.28	58.62	
		69.9		69.9	16'
sw corner		69.9	10.67	59.23	
		69.9		69.9	28' swcor to nwcor
south invert in		69.9	11.8	58.1	
		69.9		69.9	
south inlet top		69.9	6.83	63.07	
		69.9		69.9	
north inlet in		69.9	11.76	58.14	
		69.9		69.9	
north inlet top		69.9	6.99	62.91	
		69.9		69.9	
northwest corner		69.9	11.48	58.42	
		69.9		69.9	18'
n/wall toe of slope		69.9	11.62	58.28	
		69.9		69.9	17'
north wall top of dam		69.9	4.98	64.92	
		69.9		69.9	
emergency spillway		69.9	6.2	63.7	
		69.9		69.9	
north inv out		69.9	13.2	56.7	
		69.9		69.9	
south inv out		69.9	13.32	56.58	
tp/gate	1.5	70.35		68.85	
		70.35			
transformer base		70.35	4.98	65.37	
		70.35		70.35	
ne cor lot 50		70.35	8.76	61.59	

= 58
 + 3

 61

= 65

 64

= 65.4

 61.6

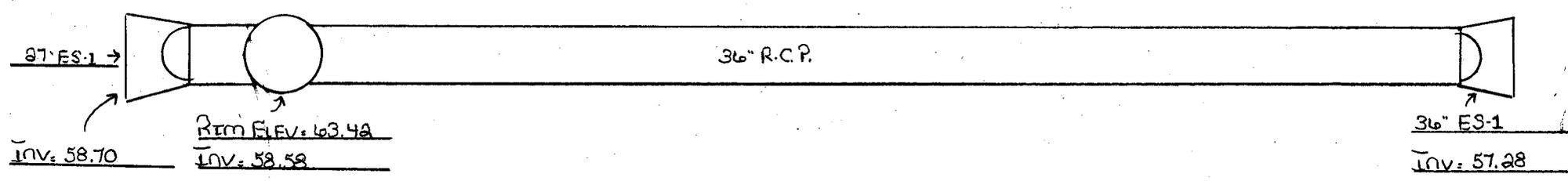
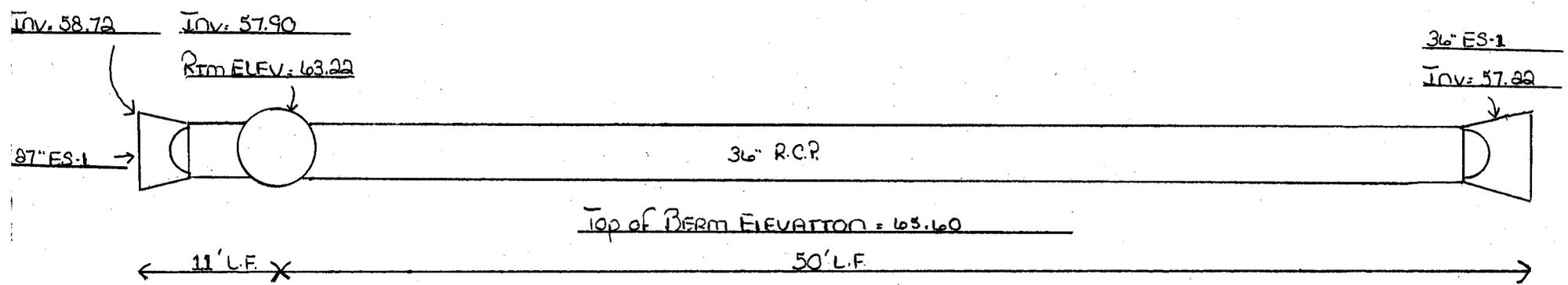
FOX RIDGE RETROFIT



PC 003 S-3-90
● Fox Ridge Subdivision

SPILLWAY @ FOX RIDGE

SPILLWAY ELEVATION = 64.22



Prepared By: Jay Prevatte, Jr., SUPERINTENDENT CENTER CONSTRUCTION Co. Inc.

EXISTING CHANNEL ADEQUACY
CALCULATIONS
IN 7105

7/12/88
PC 003

RAVINE BELOW CULVERT NO 1

$D.A. = 7.89 A_c$

$C = 0.30$

$T_c = 25 \text{ min}$

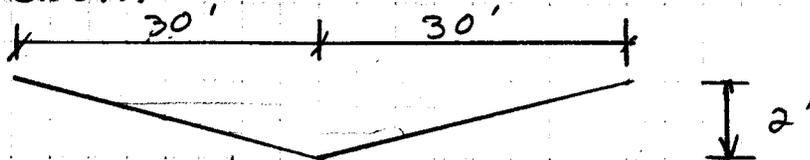
$I_2 = 3.0 \text{ in/hr}$

$I_{10} = 4.0 \text{ in/hr}$

$Q_2 = 0.30 (3.0) (7.89) = 7.10 \text{ cfs}$

AVG. CHANNEL SLOPE = 3.75%

AVG. CHANNEL SECTION



DETERMINE N VALUE

$N_1 = 0.024$

$N_2 = 0.000$

$N_3 = 0.000$

$N_4 = 0.015$

$N_5 = 0.010$

$N_6 = 0.000$

$N = (N_1 + N_2 + N_3 + N_4 + N_5) \times N_6 + (N_1 + N_2 + N_3 + N_4 + N_5)$
 $N = 0.049$

USING MANNING'S EQUATION (MODIFIED)

$Q = \frac{1.49}{N} S^{1/2} R^{2/3} A$

$7.10 = \frac{1.49}{0.049} (0.0375)^{1/2} \left(\frac{15d}{2\sqrt{15d^2+1}} \right)^{2/3} 15d^2$

$\frac{60}{60.13}$

$7.10 = 5.89 \left(\frac{15d}{30.07} \right)^{2/3} 15d^2$

$\frac{7.10}{5.89} = 0.63 d^{2/3} \cdot 15d^2$

$\frac{7.10}{55.44} = d^{2/3} \cdot d^2$

$1.28 = d^{8/3}$

$d = 0.46'$

CHANNEL IS ADEQUATE

$V = \frac{Q}{A} = \frac{7.10}{6.90} = 1.03 \text{ fps}$
 2.2 fps

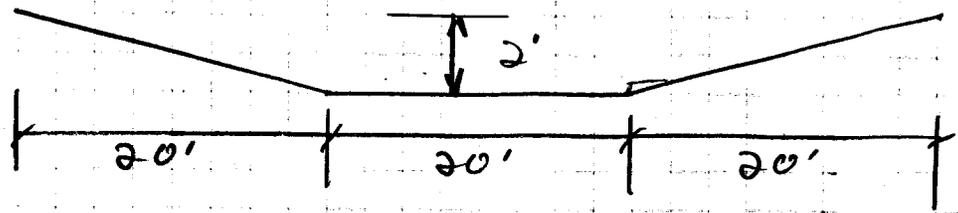
RAVINE BELOW CULVERT NO. 5

$O.A. = 420 \text{ Ac}$ $C = 0.30$ $TC = 25 \text{ min}$
 $I_2 = 3.0 \text{ in/hr}$ $I_{10} = 4.0 \text{ in/hr}$

$Q_2 = 0.30 (3.0) 420 = 3.78 \text{ cfs}$

AVG. CHANNEL SLOPE = 2.67%

AVG. CHANNEL SECTION



N VALUE = 0.049 (SAME CHANNEL CHARACTERISTICS AS PREVIOUS CHANNEL)
 USING MODIFIED MANNINGS EQUATION

$Q = \frac{1.49}{N} S^{1/2} R^{2/3} A$

USING TRIAL AND ERROR PROCESS
 CHOOSE d, SOLVE FOR V, A AND Q

$d = 0.1' \quad A = 2.1' \quad P = 22.2 \quad R = 0.09$

$V = \frac{1.49}{0.049} (0.267)^{1/2} (0.09)^{2/3} = 0.99 \text{ fps} < 3.0 \text{ fps ALLOWABLE}$

$Q = 0.99 \times 2.1 = 2.08 \text{ cfs}$

TRY $d = 0.2'$
 $A = 4.4 \quad P = 24.02 \quad R = 0.18$

$V = \frac{1.49}{0.049} (0.267)^{1/2} (0.18)^{2/3} = 1.57 \text{ fps} < 3.0 \text{ fps ALLOWABLE}$

$Q = 1.57 (4.4) = 6.93 \text{ cfs} > 3.78 \text{ cfs REQUIRED}$

CHANNEL IS MORE THAN ADEQUATE

$d = .14'$
 $Q = 3.8 \text{ cfs}$
 $V = 1.25 \text{ fps}$

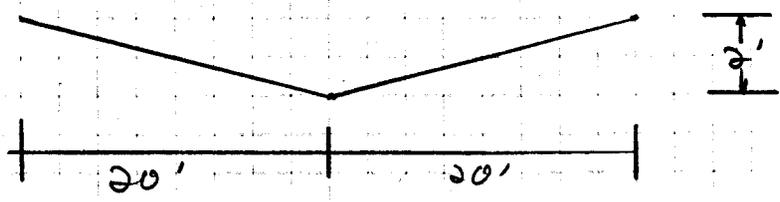
RAVINE BELOW SS # 4

$D.A. = 48.0 A_e$ $T_L = 30 \text{ min}$ $C = 0.25$
 $F_2 = 2.7 \text{ in/hr}$

$Q_2 = 0.25 (2.7) 48.0 = 32.4 \text{ cfs}$

AVG. CHANNEL SLOPE = 1.5890

AVG CHANNEL SECTION



$N = 0.049$ (SAME CHANNEL CHARACTERISTICS AS PREVIOUS CHANNEL)

USING MANNINGS EQUATION

$V = \frac{1.49}{N} S^{1/2} R^{2/3}$

CHECK CHANNEL CAPACITY ASSUMING FULL CHANNEL

$- d = 2'$
 $A = 40 \text{ SF}$ $P = 40.2$ $R = 1.00$

$V = \frac{1.49}{0.049} (0.0158)^{1/2} (1)^{2/3} = 3.82 \text{ FPS}$

$Q = 3.82 (40) = 152.90$

- TRY $d = 1'$

$A = 10 \text{ SF}$ $P = 20.10$ $R = 0.50$

$V = \frac{1.49}{0.049} (0.0158)^{1/2} (0.50)^{2/3} = 2.4 \text{ FPS}$

$Q = V \cdot A = 2.4 (10) = 24 \text{ cfs} < 32.4 \text{ cfs REQUIRED}$

- TRY $d = 1.25'$

$A = 15.63 \text{ SF}$ $P = 25.12$ $R = 0.62$

$V = \frac{1.49}{0.049} (0.0158)^{1/2} (0.62)^{2/3} = 2.77 < 3.0 \text{ cfs ALLOWABLE}$

$Q = 2.77 (15.63) = 43.40 > 32.4 \text{ cfs REQ. CHANNEL IS ADEQUATE}$

$D = 1.13'$ $V = 2.56 \text{ fps}$

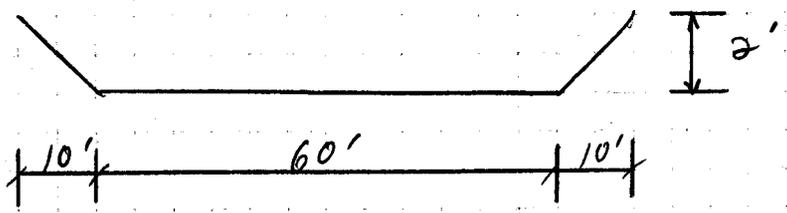
MAIN BOTTOM (AT END OF CUL-DE-SAC ON FOX HUNT TRAIL)

$O.A. = 98,00 \text{ Ac}$ $TC = 40 \text{ min}$ $C = 0,25$
 $I_2 = 2,20 \text{ IN/MI}$

$Q_2 = 0,25 (2,20) 98,00 = 53,90 \text{ cfs}$

AVG CHANNEL SLOPE = 1,86 %

AVG. CHANNEL SECTION



DETERMINE N VALUE

- $N_1 = 0,02$
- $N_2 = 0,010$
- $N_3 = 0,005$
- $N_4 = 0,010$
- $N_5 = 0,020$
- $N_6 = 0,000$

$N = 0,065$

USING MANNINGS EQUATION AND TRIAL AND ERROR

$V = \frac{1,49}{N} S^{1/2} R^{2/3}$

$D = .47'$
 $Q = 53,85$
 $V = 1,8'$

TRY $d = 0,75'$

$A = 47,8 \text{ SF}$ $P = 67,64$ $R = 0,71$

$V = \frac{1,49}{0,065} (0,0186)^{1/2} (0,71)^{2/3} = 2,49 \text{ FPS}$

$Q = 2,49 (47,8) = 118,8 \text{ cfs}$

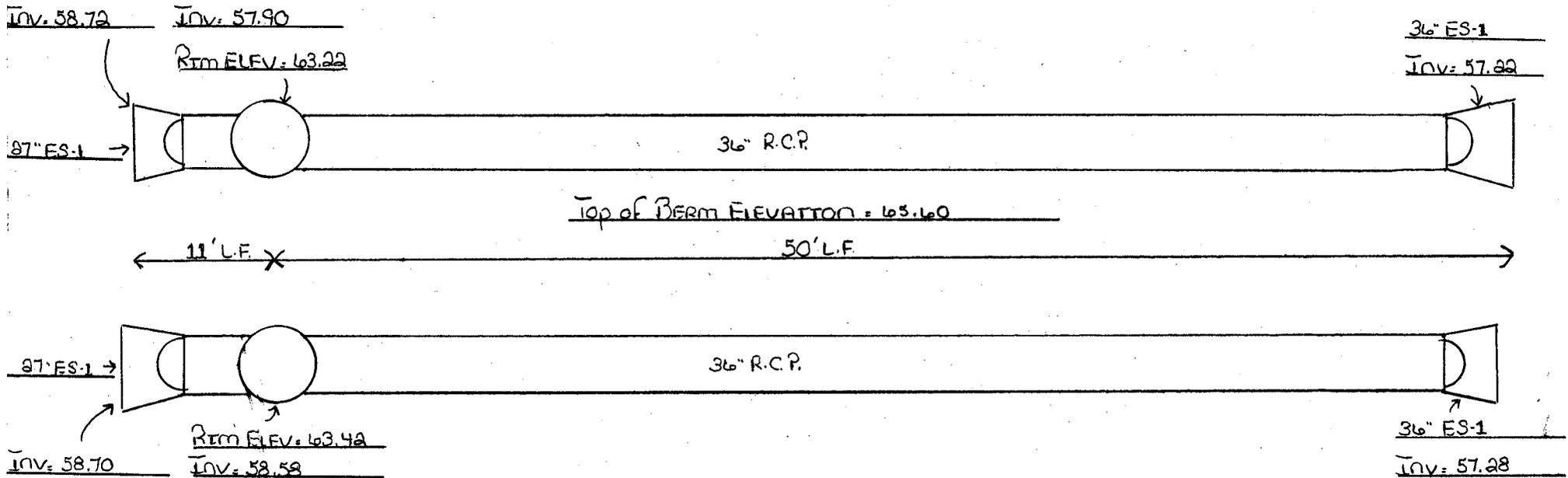
TRY $d = 0,50 \text{ ft}$ $A = 31,25 \text{ SF}$ $P = 65,1 \text{ ft}$ $R = 0,48$

$V = \frac{1,49}{0,065} (0,0186)^{1/2} (0,48)^{2/3} = 1,91 \text{ FPS} < 3,0 \text{ FPS ALLOWABLE}$

$Q = 1,91 (31,25) = 59,75 \text{ cfs} > Q_2$: CHANNEL IS ADEQUATE

SPILLWAY @ FOX RIDGE

SPILLWAY ELEVATION = 64.22

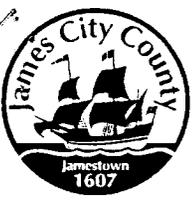


Prepared By: JAY PREVATE, JR., SUPERINTENDENT CENTER CONSTRUCTION Co. Inc.

PC003_FOX_RIDGE_SUBDIVISION - 056

PC 003
5-3-90

**James City County Environmental Division
Stormwater Management / BMP Inspection Report
Detention and Retention Pond Facilities**



S-3-90
GPIN 3110800001A

Database Inventory No. (if known): PC-003

Name of Facility: Fox Ridge BMP No.: 1 of 1 Date: 8/8/00

Location: 6010 Fox Ridge Blvd. (END OF FOX RUN ROAD)

Name of Owner: Fox Ridge Associates

Inspector: _____

Type of Facility: DRY POND

Weather Conditions: _____

If an inspection item is not applicable, mark NA, otherwise mark the appropriate column.

- O.K. - The item checked is in adequate condition and the maintenance program is currently satisfactory.
- Routine - The item checked requires attention, but does not present an immediate threat to the function of the BMP.
- Urgent - The item checked requires immediate attention to keep the BMP operational and prevent damage to the facility.

Provide an explanation and details in the comment column, if routine or urgent are marked.

Facility Item	O.K.	Routine	Urgent	Comments
Embankments and Side Slopes:				
Grass Height	✓			
Vegetated Condition		✓		<i>sparse or thin patch (straw) in place w/ netting</i>
Weed Growth	✓			
Erosion	✓			
Trash & Debris	✓			
Seepage	✓			
Fencing or Benches	NO			
Constructed Wetlands (Interior Landscaped & Planted) Areas:				
Vegetated Conditions	✓			<i>natural veg. well established</i>
Trash & Debris	✓			
Floatables	✓			
Erosion	✓			
Sediment	✓			
Dead Plant	✓			
Aesthetics	✓			
Other				

Facility Item	O.K.	Routine	Urgent	Comments
Water Pools <input type="checkbox"/> Permanent Pool (Retention Basin) <input type="checkbox"/> Shallow Marsh (Detention Basin)				
Shoreline Erosion	✓			
Algae	✓			<i>light to moderate</i>
Trash & Debris	✓			
Sediment	✓			
Aesthetics	✓			
Other				
Inflow Structures (Describe Locations):				
Condition of Structure	✓			
Erosion	✓			
Trash and Debris	✓			
Sediment	✓			
Aesthetics	✓			
Other				
Principal Flow Control Structure - Intake, Riser, etc. (Describe Location):				
Condition of Structure	✓			
Corrosion	✓			
Trash and Debris	✓			
Sediment	✓			
Aesthetics	✓			
Other				
Principal Outlet Structure - Barrel, Conduit, etc. :				
Condition of Structure	✓			
Settlement	✓			
Trash & Debris	✓			
Sediment	✓			
Erosion	✓			
Other				
Emergency Spillway (Overflow):				
Vegetation	✓			<i>SPARSE, NATURAL woodland floor type</i>
Lining	<i>NO</i>			
Erosion	✓			
Trash & Debris	✓			
Other				

Facility Item	O.K.	Routine	Urgent	Comments
Nuisance Type Conditions:				
Mosquito Breeding	✓			
Animal Burrows	✓			
Graffiti	✓			
Other				
Surrounding Perimeter Conditions:				
Land Uses	✓			
Vegetation	✓			
Trash & Debris	✓			
Aesthetics	✓			
Access /Maintenance Roads or Paths	✓			
Other				

Remarks:

Overall Environmental Division Internal Rating: 4

Signature: *W. Rick Hall*
 Title: *Environmental Specialist*

Date: _____

WATERSHED	PC	MAINTENANCE PLAN	No	CTRL STRUC DESC	RCP Riser
BMP ID NO	003	SITE AREA acre	52.2	CTRL STRUC SIZE inches	48
PLAN NO	S-3-90	LAND USE	SF Residential	OTLT BARRL DESC	Dual RCP
TAX PARCEL	(31-1)(08-1A)	old BMP TYP	Dry Pond	OTLT BARRL SIZE Inch	36
PIN NO	311080001A	JCC BMP CODE			
CONSTRUCTION DATE	12/1/1988	POINT VALUE	6	EMERG SPILLWAY	Yes
PROJECT NAME	Fox Ridge Sddivision			DESIGN HW ELEV	
FACILITY LOCATION	6010 Fox Ridge Blvd.			PERM POOL ELEV	
CITY-STATE	Williamsburg, Va. 23188	SVC DRAIN AREA acres	95.58	2-YR OUTFLOW cfs	
CURRENT OWNER	Fox Ridge Associates			10-YR OUTFLOW cfs	
OWNER ADDRESS	2640 Virginia Beach Blvd.			REC DRAWING	Yes
OWNER ADDRESS 2		SERVICE AREA DESCR	FoxRidge and portion of Forest Glen		
CITY-STATE-ZIP CODE	Virginia Beach, Va. 23452	IMPERV AREA acres		CONSTR CERTI	No
OWNER PHONE		RECV STREAM	UT of Powhatan Creek		
MAINT AGREEMENT	No	EXT DET-WQ-CTRL	No	LAST INSP DATE	8/8/2000
EMERG ACTION PLAN	No	WTR QUAL VOL acre-ft	0	INTERNAL RATING	4
		CHAN PROT CTRL	No	MISC/COMMENTS	
		CHAN PROT VOL acre-ft	0	Dual 27-inch pipes into riser. No H&H data on pond design.	
		SW/FLOOD CONTROL	Yes		
		GEOTECH REPORT	No		

Get Last BMP No

Return to Menu



PC 003 S-3-90
● Fox Ridge Subdivision

**Nationwide Permit (43) Stormwater Management Facilities
(3/18/2002)**

Discharges of dredged or fill material into non-tidal waters of the United States, excluding non-tidal wetlands adjacent to tidal waters, for the construction and maintenance of stormwater management facilities, including activities for the excavation of stormwater ponds/facilities, detention basins, and retention basins; the installation and maintenance of water control structures, outfall structures and emergency spillways; and the maintenance dredging of existing stormwater management ponds/facilities and detention and retention basins, provided the activity meets all of the following criteria:

- a. The discharge for the construction of new stormwater management facilities does not cause the loss of greater than ½ acre of non-tidal waters of the United States, excluding non-tidal wetlands adjacent to tidal waters;
- b. The discharge does not cause the loss of greater than 300 linear-feet of a stream bed, unless for intermittent stream beds this criterion is waived in writing pursuant to a determination by the District Engineer, as specified below, that the project complies with all terms and conditions of this NWP and that any adverse impacts of the project on the aquatic environment are minimal, both individually and cumulatively;
- c. For discharges causing the loss of greater than 300 linear feet of intermittent stream beds, the permittee notifies the District Engineer in accordance with the "Notification" General Condition 13. In such cases, to be authorized the District Engineer must determine that the activity complies with the other terms and conditions of the NWP, determine the adverse environmental effects are minimal both individually and cumulatively, and waive this limitation in writing before the permittee may proceed;
- d. The discharges of dredged or fill material for the construction of new stormwater management facilities in perennial streams is not authorized;
- e. For discharges or excavation for the construction of new stormwater management facilities or for the maintenance of existing stormwater management facilities causing the loss of greater than 1/10 acre of non-tidal waters, excluding non-tidal wetlands adjacent to tidal waters, provided the permittee notifies the District Engineer in accordance with General Condition 13. In addition, the notification must include:
 - (1) A maintenance plan. The maintenance plan should be in accordance with State and local requirements, if any such requirements exist;
 - (2) For discharges in special aquatic sites, including wetlands and submerged aquatic vegetation, the notification must include a delineation of affected areas; and
 - (3) A compensatory mitigation proposal that offsets the loss of waters of the United States. Maintenance in constructed areas will not require mitigation provided such maintenance is accomplished in designated maintenance areas and not within compensatory mitigation areas (i.e., district engineers may designate non-maintenance areas, normally at the downstream end of the stormwater management facility, in existing stormwater management facilities). (No mitigation will be required for activities which are exempt from Section 404 permit requirements);

- f. The permittee must avoid and minimize discharges into waters of the United States at the project site to the maximum extent practicable, and the notification must include a written statement to the District Engineer detailing compliance with this condition (i.e., why the discharge must occur in waters of the United States and why additional minimization cannot be achieved);
- g. The stormwater management facility must comply with General Condition 21 and be designed using best management practices (BMPs) and watershed protection techniques. Examples may include forebays (deeper areas at the upstream end of the stormwater management facility that would be maintained through excavation), vegetated buffers, and siting considerations to minimize adverse effects to aquatic resources. Another example of a BMP would be bioengineering methods incorporated into the facility design to benefit water quality and minimize adverse effects to aquatic resources from storm flows, especially downstream of the facility, that provide, to the maximum extent practicable, for long term aquatic resource protection and enhancement;
- h. Maintenance excavation will be in accordance with an approved maintenance plan and will not exceed the original contours of the facility as approved and constructed; and
- i. The discharge is part of a single and complete project.(Section 404)

GENERAL CONDITIONS:

The following general conditions must be followed in order for any authorization by a NWP to be valid:

1. Navigation. No activity may cause more than a minimal adverse effect on navigation.
2. Proper Maintenance. Any structure or fill authorized shall be properly maintained, including maintenance to ensure public safety.
3. Soil Erosion and Sediment Controls. Appropriate soil erosion and sediment controls must be used and maintained in effective operating condition during construction, and all exposed soil and other fills, as well as any work below the ordinary high water mark or high tide line, must be permanently stabilized at the earliest practicable date. Permittees are encouraged to perform work within waters of the United States during periods of low-flow or no-flow.
4. Aquatic Life Movements. No activity may substantially disrupt the necessary life-cycle movements of those species of aquatic life indigenous to the waterbody, including those species which normally migrate through the area, unless the activity's primary purpose is to impound water. Culverts placed in streams must be installed to maintain low flow conditions.
5. Equipment. Heavy equipment working in wetlands must be placed on mats, or other measures must be taken to minimize soil disturbance.
6. Regional and Case-By-Case Conditions. The activity must comply with any regional conditions which may have been added by the division engineer (see 33 CFR 330.4(e)) and with any case specific conditions added by the Corps or by the State or tribe in its Section 401 water quality certification and Coastal Zone Management Act consistency determination.
7. Wild and Scenic Rivers. No activity may occur in a component of the National Wild and Scenic River System; or in a river officially designated by Congress as a "study river" for possible inclusion in the system, while the river is in an official study status; unless the appropriate Federal agency, with direct management responsibility for such river, has determined in writing that the proposed activity will not adversely affect the Wild and Scenic River designation, or study status. Information on Wild and Scenic Rivers may be obtained from the appropriate Federal land management agency in the area (e.g., National Park Service, U.S. Forest Service, Bureau of Land Management, U.S. Fish and Wildlife Service).
8. Tribal Rights. No activity or its operation may impair reserved tribal rights, including, but not limited to, reserved water rights and treaty fishing and hunting rights.

9. **Water Quality.** (a) In certain States and tribal lands an individual 401 water quality certification must be obtained or waived (See 33 CFR 330.4(c)).
- (b) For NWP's 12, 14, 17, 18, 32, 39, 40, 42, 43, and 44, where the State or tribal 401 certification (either generically or individually) does not require or approve a water quality management plan, the permittee must include design criteria and techniques that will ensure that the authorized work does not result in more than minimal degradation of water quality (or the Corps determines that compliance with state or local standards, where applicable, will ensure no more than minimal adverse effect on water quality). An important component of water quality management includes stormwater management that minimizes degradation of the downstream aquatic system, including water quality (refer to General Condition 21 for stormwater management requirements). Another important component of water quality management is the establishment and maintenance of vegetated buffers next to open waters, including streams (refer to General Condition 19 for vegetated buffer requirements for the NWP's).
10. **Coastal Zone Management.** In certain states, an individual state coastal zone management consistency concurrence must be obtained or waived (see 33 CFR 330.4(d)).
11. **Endangered Species.** (a) No activity is authorized under any NWP which is likely to jeopardize the continued existence of a threatened or endangered species or a species proposed for such designation, as identified under the Federal Endangered Species Act, or which will destroy or adversely modify the critical habitat of such species. Non-federal permittees shall notify the District Engineer if any listed species or designated critical habitat might be affected or is in the vicinity of the project, or is located in the designated critical habitat and shall not begin work on the activity until notified by the District Engineer that the requirements of the Endangered Species Act have been satisfied and that the activity is authorized. For activities that may affect Federally-listed endangered or threatened species or designated critical habitat, the notification must include the name(s) of the endangered or threatened species that may be affected by the proposed work or that utilize the designated critical habitat that may be affected by the proposed work. As a result of formal or informal consultation with the FWS or NMFS, the District Engineer may add species-specific regional endangered species conditions to the NWP's.
- (b) Authorization of an activity by a nationwide permit does not authorize the "take" of a threatened or endangered species as defined under the Federal Endangered Species Act. In the absence of separate authorization (e.g., an ESA Section 10 Permit, a Biological Opinion with "incidental take" provisions, etc.) from the U.S. Fish and Wildlife Service or the National Marine Fisheries Service, both lethal and non-lethal "takes" of protected species are in violation of the Endangered Species Act. Information on the location of threatened and endangered species and their critical habitat can be obtained directly from the offices of the U.S. Fish and Wildlife Service and National Marine Fisheries Service or their world wide web pages at <http://www.fws.gov/r9endspp/endspp.html> and http://www.nmfs.noaa.gov/prot_res/overview/es.html respectively.
12. **Historic Properties.** No activity which may affect historic properties listed, or eligible for listing, in the National Register of Historic Places is authorized, until the DE has complied with the provisions of 33 CFR Part 325, Appendix C. The prospective permittee must notify the District Engineer if the authorized activity may affect any historic properties listed, determined to be eligible, or which the prospective permittee has reason to believe may be eligible for listing on the National Register of Historic Places, and shall not begin the activity until notified by the District Engineer that the requirements of the National Historic Preservation Act have been satisfied and that the activity is authorized. Information on the location and existence of historic resources can be obtained from the State Historic Preservation Office and the National Register of Historic Places (see 33 CFR 330.4(g)). For activities that may affect historic properties listed in, or eligible for listing in, the National Register of Historic Places, the notification must state which historic property may be affected by the proposed work or include a vicinity map indicating the location of the historic property.
13. **Notification.**
- (a) **Timing:** Where required by the terms of the NWP, the prospective permittee must notify the District Engineer with a preconstruction notification (PCN) as early as possible. The District Engineer must determine if the PCN is complete within 30 days of the date of receipt and can request the additional information necessary to make the PCN complete only once. However, if the prospective permittee does not provide all of the requested information, then the District Engineer will notify the prospective permittee that the PCN is still incomplete and the PCN review process will not commence until all of the requested information has been received by the District Engineer. The prospective permittee shall not begin the activity:
- (1) Until notified in writing by the District Engineer that the activity may proceed under the NWP with any special conditions imposed by the District or Division Engineer; or
 - (2) If notified in writing by the District or Division Engineer that an individual permit is required; or
 - (3) Unless 45 days have passed from the District Engineer's receipt of the complete notification and the prospective permittee has not received written notice from the District or Division Engineer. Subsequently, the permittee's right to proceed under the NWP may be modified, suspended, or revoked only in accordance with the procedure set forth in 33 CFR 330.5(d)(2).
- (b) **Contents of Notification:** The notification must be in writing and include the following information:
- (1) Name, address, and telephone numbers of the prospective permittee;
 - (2) Location of the proposed project;
- (3) Brief description of the proposed project; the project's purpose; direct and indirect adverse environmental effects the project would cause; any other NWP(s), regional general permit(s), or individual permit(s) used or intended to be used to authorize any part of the proposed project or any related activity. Sketches should be provided when necessary to show that the activity complies with the terms of the NWP (Sketches usually clarify the project and when provided result in a quicker decision.);
- (4) For NWP's 7, 12, 14, 18, 21, 34, 38, 39, 40, 41, 42, and 43, the PCN must also include a delineation of affected special aquatic sites, including wetlands, vegetated shallows (e.g., submerged aquatic vegetation, seagrass beds), and riffle and pool complexes (see paragraph 13(f));
 - (5) For NWP 7 (Outfall Structures and Maintenance), the PCN must include information regarding the original design capacities and configurations of those areas of the facility where maintenance dredging or excavation is proposed.
 - (6) For NWP 14 (Linear Transportation Projects), the PCN must include a compensatory mitigation proposal to offset permanent losses of waters of the United States and a statement describing how temporary losses of waters of the United States will be minimized to the maximum extent practicable.
 - (7) For NWP 21 (Surface Coal Mining Activities), the PCN must include an Office of Surface Mining (OSM) or state-approved mitigation plan, if applicable. To be authorized by this NWP, the District Engineer must determine that the activity complies with the terms and conditions of the NWP and that the adverse environmental effects are minimal both individually and cumulatively and must notify the project sponsor of this determination in writing.
 - (8) For NWP 27 (Stream and Wetland Restoration Activities), the PCN must include documentation of the prior condition of the site that will be reverted by the permittee.
 - (9) For NWP 29 (Single-Family Housing), the PCN must also include:
 - (i) Any past use of this NWP by the individual permittee and/or the permittee's spouse;
 - (ii) A statement that the single-family housing activity is for a personal residence of the permittee;
 - (iii) A description of the entire parcel, including its size, and a delineation of wetlands. For the purpose of this NWP, parcels of land measuring ¼ acre or less will not require a formal on-site delineation. However, the applicant shall provide an indication of where the wetlands are and the amount of wetlands that exists on the property. For parcels greater than ¼ acre in size, a formal wetland delineation must be prepared in accordance with the current method required by the Corps. (See paragraph 13(f));
 - (iv) A written description of all land (including, if available, legal descriptions) owned by the prospective permittee and/or the prospective permittee's spouse, within a one mile radius of the parcel, in any form of ownership (including any land owned as a partner, corporation, joint tenant, co-tenant, or as a tenant-by-the-entirety) and any land on which a purchase and sale agreement or other contract for sale or purchase has been executed;
 - (10) For NWP 31 (Maintenance of Existing Flood Control Facilities), the prospective permittee must either notify the District Engineer with a PCN prior to each maintenance activity or submit a five year (or less) maintenance plan. In addition, the PCN must include all of the following:
 - (i) Sufficient baseline information so as to identify the approved channel depths and configurations and existing facilities. Minor deviations are authorized, provided the approved flood control protection or drainage is not increased;
 - (ii) A delineation of any affected special aquatic sites, including wetlands; and,

15. Use of Multiple Nationwide Permits. The use of more than one NWP for a single and complete project is prohibited, except when the acreage loss of waters of the United States authorized by the NWP does not exceed the acreage limit of the NWP with the highest specified acreage limit. For example, if a road crossing over tidal waters is constructed under NWP 14, with associated bank stabilization authorized by NWP 13, the maximum acreage loss of waters of the United States for the total project cannot exceed 1/3 acre.
16. Water Supply Intakes. No activity, including structures and work in navigable waters of the United States or discharges of dredged or fill material, may occur in the proximity of a public water supply intake except where the activity is for repair of the public water supply intake structures or adjacent bank stabilization.
17. Shellfish Beds. No activity, including structures and work in navigable waters of the United States or discharges of dredged or fill material, may occur in areas of concentrated shellfish populations, unless the activity is directly related to a shellfish harvesting activity authorized by NWP 4.
18. Suitable Material. No activity, including structures and work in navigable waters of the United States or discharges of dredged or fill material, may consist of unsuitable material (e.g., trash, debris, car bodies, asphalt, etc.) and material used for construction or discharged must be free from toxic pollutants in toxic amounts (see Section 307 of the Clean Water Act).
19. Mitigation. The District Engineer will consider the factors discussed below when determining the acceptability of appropriate and practicable mitigation necessary to offset adverse effects on the aquatic environment that are more than minimal.
 - (a) The project must be designed and constructed to avoid and minimize adverse effects to waters of the US to the maximum extent practicable at the project site (i.e., on site).
 - (b) Mitigation in all its forms (avoiding, minimizing, rectifying, reducing or compensating) will be required to the extent necessary to ensure that the adverse effects to the aquatic environment are minimal.
 - (c) Compensatory mitigation at a minimum one-for-one ratio will be required for all wetland impacts requiring a PCN, unless the District Engineer determines in writing that some other form of mitigation would be more environmentally appropriate and provides a project-specific waiver of this requirement. Consistent with National policy, the District Engineer will establish a preference for restoration of wetlands as compensatory mitigation, with preservation used only in exceptional circumstances.
 - (d) Compensatory mitigation (i.e., replacement or substitution of aquatic resources for those impacted) will not be used to increase the acreage losses allowed by the acreage limits of some of the NWP's. For example, 1/4-acre of wetlands cannot be created to change a 3/4-acre loss of wetlands to a 1/2-acre loss associated with NWP 39 verification. However, 1/2-acre of created wetlands can be used to reduce the impacts of a 1/2-acre loss of wetlands to the minimum impact level in order to meet the minimal impact requirement associated with NWP's.
 - (e) To be practicable, the mitigation must be available and capable of being done considering costs, existing technology, and logistics in light of the overall project purposes. Examples of mitigation that may be appropriate and practicable include, but are not limited to: reducing the size of the project; establishing and maintaining wetland or upland vegetated buffers to protect open waters such as streams; and replacing losses of aquatic resource functions and values by creating, restoring, enhancing, or preserving similar functions and values, preferably in the same watershed.
 - (f) Compensatory mitigation plans for projects in or near streams or other open waters will normally include a requirement for the establishment, maintenance, and legal protection (e.g., easements, deed restrictions) of vegetated buffers to open waters. In many cases, vegetated buffers will be the only compensatory mitigation required. Vegetated buffers should consist of native species. The width of the vegetated buffers required will address documented water quality or aquatic habitat loss concerns. Normally, the vegetated buffer will be 25 to 50 feet wide on each side of the stream, but the District Engineers may require slightly wider vegetated buffers to address documented water quality or habitat loss concerns. Where both wetlands and open waters exist on the project site, the Corps will determine the appropriate compensatory mitigation (e.g., stream buffers or wetlands compensation) based on what is best for the aquatic environment on a watershed basis. In cases where vegetated buffers are determined to be the most appropriate form of compensatory mitigation, the District Engineer may waive or reduce the requirement to provide wetland compensatory mitigation for wetland impacts.
- (g) Compensatory mitigation proposals submitted with the "notification" may be either conceptual or detailed. If conceptual plans are approved under the verification, then the Corps will condition the verification to require detailed plans be submitted and approved by the Corps prior to construction of the authorized activity in waters of the US.
- (h) Permittees may propose the use of mitigation banks, in-lieu fee arrangements or separate activity-specific compensatory mitigation. In all cases that require compensatory mitigation, the mitigation provisions will specify the party responsible for accomplishing and/or complying with the mitigation plan.
20. Spawning Areas. Activities, including structures and work in navigable waters of the United States or discharges of dredged or fill material, in spawning areas during spawning seasons must be avoided to the maximum extent practicable. Activities that result in the physical destruction (e.g., excavate, fill, or smother downstream by substantial turbidity) of an important spawning area are not authorized.
21. Management of Water Flows. To the maximum extent practicable, the activity must be designed maintain preconstruction downstream flow conditions (e.g., location, capacity, and flow rates). Furthermore, the activity must not permanently restrict or impede the passage of normal or expected high flows (unless the primary purpose of the fill is to impound waters) and the structure or discharge of dredged or fill material must withstand expected high flows. The activity must, to the maximum extent practicable, provide for retaining excess flows from the site, provide for maintaining surface flow rates from the site similar to preconstruction conditions, and provide for not increasing water flows from the project site, relocating water, or redirecting water flow beyond preconstruction conditions. Stream channelizing will be reduced to the minimal amount necessary, and the activity must, to the maximum extent practicable, reduce adverse effects such as flooding or erosion downstream and upstream of the project site, unless the activity is part of a larger system designed to manage water flows. In most cases, it will not be a requirement to conduct detailed studies and monitoring of water flow. This condition is only applicable to projects that have the potential to affect waterflows. While appropriate measures must be taken, it is not necessary to conduct detailed studies to identify such measures or require monitoring to ensure their effectiveness. Normally, the Corps will defer to state and local authorities regarding management of water flow..
22. Adverse Effects From Impoundments. If the activity, including structures and work in navigable waters of the United States or discharge of dredged or fill material, creates an impoundment of water, adverse effects on the aquatic system caused by the accelerated passage of water and/or the restriction of its flow shall be minimized to the maximum extent practicable.
23. Waterfowl Breeding Areas. Activities, including structures and work in navigable waters of the United States or discharges of dredged or fill material, into breeding areas for migratory waterfowl must be avoided to the maximum extent practicable.
24. Removal of Temporary Fills. Any temporary fills must be removed in their entirety and the affected areas returned to their preexisting elevation.
25. Designated Critical Resource Waters. Critical resource waters include, NOAA-designated marine sanctuaries, National Estuarine Research Reserves, National Wild and Scenic Rivers, critical habitat for Federally listed threatened and endangered species, coral reefs, State natural heritage sites, and outstanding national resource waters or other waters officially designated by a State as having particular environmental or ecological significance and identified by the District Engineer after notice and opportunity for public comment. The District Engineer may also designate additional critical resource waters after notice and opportunity for comment.
 - (a) Except as noted below, discharges of dredged or fill material into waters of the United States are not authorized by NWP's 7, 12, 14, 16, 17, 21, 29, 31, 35, 39, 40, 42, 43, and 44 for any activity within, or directly affecting, critical resource waters, including wetlands adjacent to such waters. Discharges of dredged or fill materials into waters of the United States may be authorized by the above NWP's in National Wild and Scenic Rivers if the activity complies with General Condition 7. Further, such discharges may be authorized in designated critical habitat for Federally listed threatened or endangered species if the activity complies with General Condition 11 and the U.S. Fish and Wildlife Service or the National Marine Fisheries Service has concurred in a determination of compliance with this condition.
 - (b) For NWP's 3, 8, 10, 13, 15, 18, 19, 22, 23, 25, 27, 28, 30, 33, 34, 36, 37, and 38, notification is required in accordance with General Condition 13, for any activity proposed in the designated critical resource waters including wetlands adjacent to those waters. The District Engineer may authorize activities under these NWP's only after he determines that the impacts to the critical resource waters will be no more than minimal.

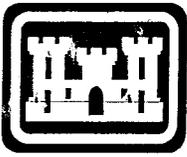
26. Fills Within 100-Year Floodplains. For purposes of this general condition, 100-year floodplains will be identified through the Federal Emergency Management Agency's (FEMA) Flood Insurance Rate Maps or FEMA-approved local floodplain maps.
- (a) Discharges in Floodplain; Below Headwaters. Discharges of dredged or fill material into waters of the US within the mapped 100-year floodplain, below headwaters (i.e. five cfs), resulting in permanent above-grade fills, are not authorized by NWP's 39, 40, 42, 43, and 44.
 - (b) Discharges in Floodway; Above Headwaters. Discharges of dredged or fill material into waters of the US within the FEMA or locally mapped floodway, resulting in permanent above-grade fills, are not authorized by NWP's 39, 40, 42, and 44.
 - (c) The permittee must comply with any applicable FEMA-approved state or local floodplain management requirements.
27. Construction Period. For activities that have not been verified by the Corps and the project was commenced or under contract to commence by the expiration date of the NWP (or modification or revocation date), the work must be completed within 12-months after such date (including any modification that affects the project). For activities that have been verified and the project was commenced or under contract to commence within the verification period, the work must be completed by the date determined by the Corps. For projects that have been verified by the Corps, an extension of a Corps approved completion date maybe requested. This request must be submitted at least one month before the previously approved completion date.

Further Information:

1. District engineers have authority to determine if an activity complies with the terms and conditions of an NWP.
2. NWP's do not obviate the need to obtain other Federal, State, or local permits, approvals, or authorizations required by law.
3. NWP's do not grant any property rights or exclusive privileges.
4. NWP's do not authorize any injury to the property or rights of others.
5. NWP's do not authorize interference with any existing or proposed Federal project.

Section 401 Water Quality Conditions. No additional permit will be required except in the following cases:

1. When compensatory mitigation is in the form of the purchase of mitigation bank credits and the bank is not located within the same hydrologic unit or an adjacent hydrologic unit within the same river watershed as the impacted site by the Hydrologic Unit Map of the United States, 1980, unless the conditions listed in Section 62.1-44.15:5E of the Code of Virginia relating to bank and impact type and location are met.
2. When compensatory mitigation involves only the preservation of wetlands and/or wetland or upland vegetated buffers without accompanying creation or restoration of wetlands or the purchase of mitigation bank credits, or does not meet the goal of no net loss of wetland acreage and function.
3. For the location of a stormwater management facility in perennial stream or in oxygen or temperature impaired waters.
4. For impacts to perennial streams in excess of 500 linear feet and for impacts to intermittent streams in excess of 1500 linear feet.
5. For any water withdrawal project.



U.S. Army Corps of Engineers
 Norfolk District, Eastern Virginia Regulatory Section
 803 Front Street
 Norfolk, Virginia 23510

Project Number: 03-R0421

Waterway: Powhatan Creek

1. Participant:

Wayland Bass, County Engineer
 James City County Development Management
 101-E Mounts Bay Road
 Williamsburg, VA 23187-8784

2. Authorized Agent:

None



3. Address of Job Site:

3955 Fox Hunt Trail (Parcel ID 3110800001C), located east of Centerville Rd, south of Fox Hunt Trail, north of Fox Run, near the eastern limits of the Fox Ridge subdivision

4. Project Description:

Retrofit of an existing stormwater "dry pond" or BMP to accommodate the 1 year 24 hr storm and to ensure channel protection;. The project includes construction of a vinyl weir wall approximately 10 feet upstream of existing BMP, installation of a 6" low flow outlet pipe, and placement of riprap inside weir wall and at outlet. The project will entail fill of approximately 0.003 acres of waters of the U.S. including wetlands. The location and extent of the proposed work is shown on the attached drawings.

5. Findings

On March 18, 2003, Mr. Steven Martin of my staff examined the proposed work described above accompanied by representatives of James City County's Environmental Division. This project will modify an existing stormwater dry pond to treat the 1 year 24 hour storm event. The planned modifications are intended to improve the quality of waters draining from the Fox Ridge development into an unnamed tributary of Powhatan Creek. The work is also intended to reduce channel scouring downstream of the existing stormwater BMP.

The proposed work described in part 4. above and on the attached drawings will be located in waters of the U.S. (including wetlands). Our basis for this determination is the application of the Corps' of Engineers 1987 Wetland Delineation Manual and the positive indicators of wetland hydrology, hydric soils, and hydrophytic vegetation. The affected wetlands are waters of the United States. Both the wetlands within the design pool of the existing stormwater dry pond and the associated intermittent headwater stream are part of a tributary system to interstate waters (33 CFR 328.3(a)). The waterway has an ordinary high water mark along much of its length. This wetland jurisdictional determination is valid for a period of five years from the date of this letter unless new information warrants revision of the delineation before the expiration date.

Much of the construction work associated with the planned retrofit of the existing stormwater BMP would be conducted in waters of the U.S. (including wetlands). However, the proposed work would impact less than 0.1 acre of waters of the U.S. (including wetlands). This activity has been reviewed and found to satisfy the criteria contained in the Corps Nationwide Permit (NWP) 43, attached. (The Corps Nationwide Permits were published in the Federal Register (67 FR 2020) on January 15, 2002 as corrected by Federal Register (67 FR 6692) on February 13, 2002 and Federal Register (67 FR 8579) on February 25, 2002 and the regulations governing their use can be found in 33 CFR 330 published in Volume 56, Number 226 of the Federal Register dated November 22, 1991.)

Provided the enclosed conditions are met, an individual Department of the Army Permit will not be required. You must also adhere to the 401 certification conditions issued by the Virginia Department of Environmental Quality (DEQ) outlined in their letter of March 29, 2002. The Corps' Nationwide Permit conditions and 401 certification conditions are attached.

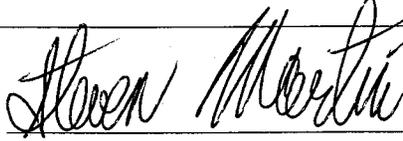
Enclosed is a "compliance certification" form, which must be signed and returned within 30 days of completion of the project (see nationwide permit condition number 14). Your signature on this form certifies that you have completed the work in accordance with the nationwide permit terms and conditions.

This verification is valid for five years from the date of this letter, unless the Norfolk District Engineer uses discretionary authority to modify, suspend or revoke this verification. The Chief of Engineers will periodically review the nationwide permits and their conditions and will decide to either modify, reissue or revoke the permits. These nationwides are scheduled to expire on March 18, 2007. If NWP 43 is reissued without modification or if your activity complies with any subsequent nationwide permit, the expiration date of this verification will not change. However, if NWP 43 is modified or revoked so that the activity listed above would no longer be authorized and you have commenced or are under contract to commence the work, you will have twelve months from the date of that permit change to complete the activity. Activities completed under the authorization of a nationwide permit which was in effect at the time the activity was completed continue to be authorized by that nationwide permit. It is your responsibility to remain informed of changes to the nationwide permits. We will issue a special public notice announcing any changes to the nationwide permits when they occur.

Copy Furnished:

Environmental Division, James City County
Planning Department, James City County

6. Corps Contact: Steven Martin at (757) 441-7787.

for 

Nicholas L. Konchuba
Chief, Eastern Virginia
Regulatory Section

NAO FL 13 REVISED DEC 90

**CERTIFICATE OF COMPLIANCE
WITH
ARMY CORPS OF ENGINEERS PERMIT**

Permit Number: 03-R0421-18

Name of Permittee: Wayland Bass, County Engineer
James City County

Date of Issuance: July 7, 2003

Permit Type: Nationwide Permit 43 (< 0.1 ac)

Within 30 days of completion of the activity authorized by this permit and any mitigation required by the permit, sign this certification and return it to the following address:

Steven Martin
c/o Regulatory Branch
Norfolk District Corps of Engineers
803 Front Street
Norfolk, Va. 23510-1096

Please note that your permitted activity is subject to a compliance inspection by a U.S. Army Corps of Engineers representative. If you fail to comply with this permit you are subject to permit suspension, modification or revocation.

I hereby certify that the work authorized by the above referenced permit has been completed in accordance with the terms and conditions of the said permit, and required mitigation has been completed in accordance with the permit conditions.

Signature of Permittee

Date

Scott Thomas

From: Scott Thomas
Sent: Tuesday, April 15, 2003 2:28 PM
To: Wayland Bass
Subject: RE: FOX RIDGE BMP UPGRADE

Since there's not enough volume in the pond to get the "true" 1-year storm volume and release over 24 hours, the minimum pipe size we would allow is 3-inch. Based on my observations of BMPs the smaller sizes tend to clog more, I have seen better success with size 6-inch and up.

I recommend six inch.

-----Original Message-----

From: Wayland Bass
Sent: Tuesday, April 15, 2003 10:57 AM
To: Scott Thomas
Subject: FOX RIDGE BMP UPGRADE

SCOTT; I HAVE STARTED THE COE PERMIT APPLICATION. WHAT SIZE PIPE DO YOU WANT TO SHOW THROUGH THE BULKHEAD?

THANKS WNB

*check
stage-discharge
data.*

*BMP Retrofit
Analyses*

*convert EXIST BMP
with an ED
weir wall structure.*

Scott Thomas

From: Scott Thomas
Sent: Tuesday, April 15, 2003 3:23 PM
To: Wayland Bass
Subject: FOX RIDGE BMP UPGRADE (2)

I ran through some rough calcs.

If top of wall is at El. 61.1 and base of wall/invert pipe is at El. 58.1, then the wall is around 3 feet high. Storage elevation estimated at this elevation is about 30,077.8 cubic feet. Volume of the 1-year storm for the 106 acre drainage area at CN 81, Tc = 37.6 minutes is 10.297 acre-ft. Therefore, there is no way to store the 1-year storm volume at the top of wall elevation.

The top of wall volume is just under the 1" rainfall applied to the drainage area (runoff 0.10 inches). The 1" rainfall volume is about 38,000 c.f.

Using actual storage volume of 30,077.8 cf at the top of the proposed wall,

1. For 24-hour detention, pipe size is 2-3/4 inch diameter.
2. For 18-hour detention, pipe size is about 3-1/4 inch diameter.
3. For 12-inch pipe, detention time would be about 1.27 hours.
4. For 8-inch pipe, detention time would be about 2.87 hours.
5. For 6-inch pipe, detention time would be about 5.1 hours
6. For 4-inch pipe, detention time would be about 11.5 hours
7. For 3-inch pipe, detention time would be about 20.5 hours
8. Any smaller size would exceed the USACOE detention time of 18 hours

Recommend 6-inch, clogging and lack of maintenance would result in orifice size equal to 3 or 4 inch, giving us about 15 hours of detention time.

Scott

-----Original Message-----

From: Scott Thomas
Sent: Tuesday, April 15, 2003 2:28 PM
To: Wayland Bass
Subject: RE: FOX RIDGE BMP UPGRADE

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SCOTT; I HAVE STARTED THE COE PERMIT APPLICATION. WHAT SIZE PIPE DO YOU WANT TO SHOW THROUGH THE BULKHEAD?

4/15/2003

PC003_FOX_RIDGE_SUBDIVISION - 071



EXHIBIT 1 LOCATION MAP

1"=200'

CENTERVILLE ROAD

FOX RIDGE SUBDIVISION

BMP

U.S. ARMY CORPS OF ENGINEERS
DATE: APR 17 2005
BY: [Signature]

