



CERTIFICATE OF AUTHENTICITY

THIS IS TO CERTIFY THAT THE FOLLOWING ELECTRONIC RECORDS ARE TRUE AND ACCURATE REPRODUCTIONS OF THE ORIGINAL RECORDS OF JAMES CITY COUNTY GENERAL SERVICES DEPARTMENT- STORMWATER DIVISION; WERE SCANNED IN THE REGULAR COURSE OF BUSINESS PURSUANT TO GUIDELINES ESTABLISHED BY THE LIBRARY OF VIRGINIA AND ARCHIVES; AND HAVE BEEN VERIFIED IN THE CUSTODY OF THE INDIVIDUAL LISTED BELOW.

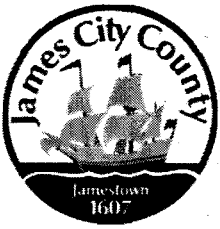
BMP NUMBER: CC-007

DATE VERIFIED: March 21, 2012

QUALITY ASSURANCE TECHNICIAN: Leah Hardenbergh

Leah Hardenbergh

LOCATION: WILLIAMSBURG, VIRGINIA



Stormwater Division

MEMORANDUM

DATE: March 10, 2010
TO: Michael J. Gillis, Virginia Correctional Enterprises Document Management Services
FROM: Jo Anna Ripley, Stormwater
PO: 270712
RE: Files Approved for Scanning

General File ID or BMP ID: CC007

PIN: 4140700007

Subdivision, Tract, Business or Owner

Name (if known):

Suburban Propane

Property Description:

Utility

Site Address:

7234 Merrimac Trail

(For internal use only)

Box 10

Drawer: 6

Agreements: (in file as of scan date)

N

Book or Doc#:

Page:

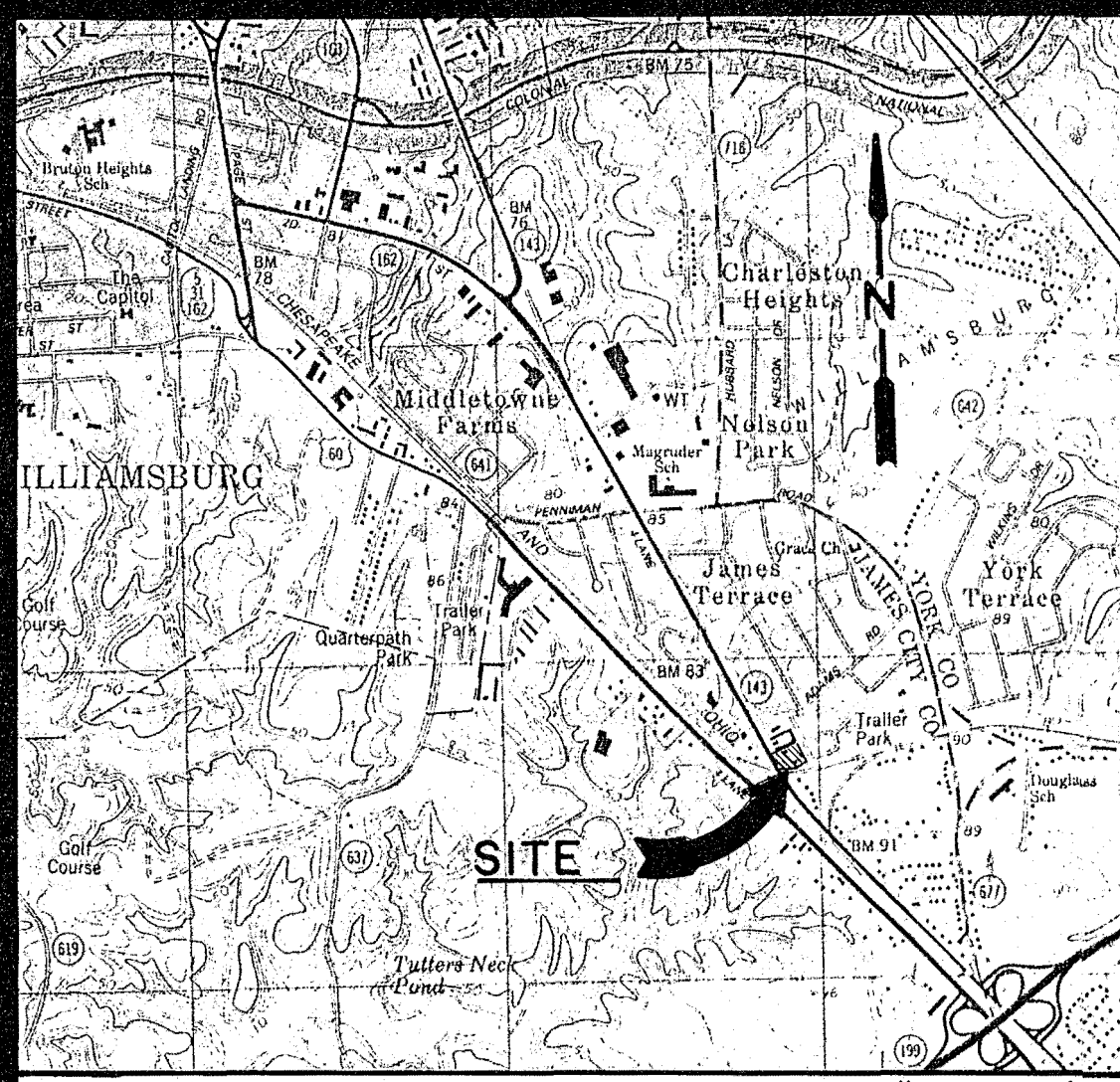
Comments

CC007

Contents for Stormwater Management Facilities As-built Files

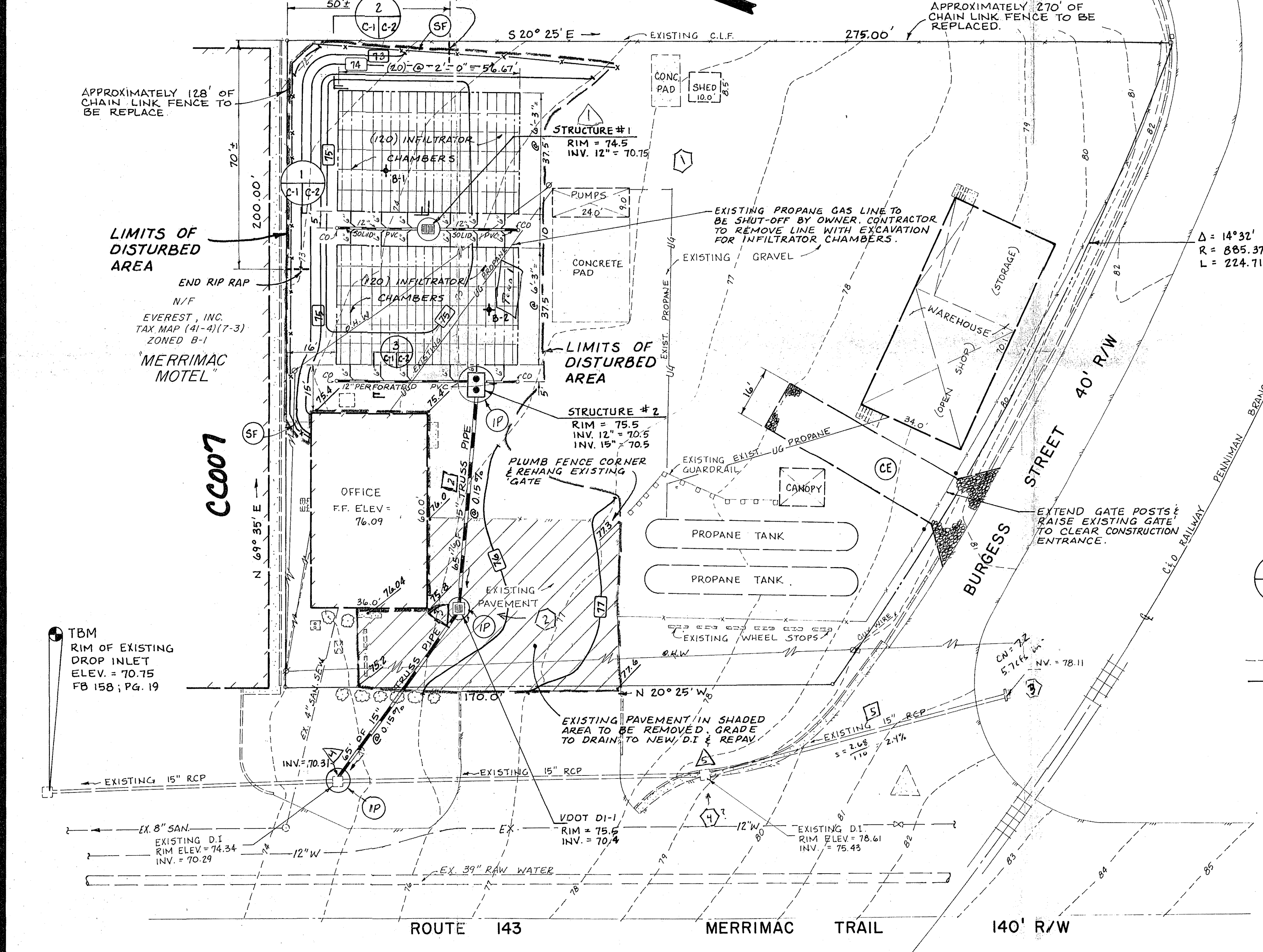
Each file is to contain:

1. As-built plan
2. Completed construction certification
- ③ Construction Plan
- ④ Design Calculations
- ⑤ Watershed Map
6. Maintenance Agreement
7. Correspondence with owners
8. Inspection Records
9. Enforcement Actions



VICINITY MAP

SCALE: 1" = 2000'



SITE PLAN

SCALE: 1" = 20'

OWNER: SUBURBAN PROPANE GAS
7232 MERRIMAC TRAIL
WILLIAMSBURG, VIRGINIA 23185
PHONE: (804) 229-5777
LOCAL MANAGER: RON TURNER

LAND USE SUMMARY
ZONING OF SITE: (B-1) GENERAL BUSINESS
TAX MAP: (41-4) (7-6)
TOTAL SITE AREA: 45,951 S.F. / 1.05 AC.
BUILDING AREA: 4,543 S.F. / 0.10 AC. / 10%
OPEN SPACE: 892 S.F. / 0.02 AC. / 2%
IMPERVIOUS AREA: 98 %

- GENERAL NOTES
- ELEVATIONS REFER TO NATIONAL GEODETIC DATUM (1929) & BASED ON JAMES CITY COUNTY STATION NO. 337 ELEVATION = 81.96
 - THIS PLAN DOES NOT REPRESENT AN ACTUAL BOUNDARY SURVEY BY DJG, INC. BOUNDARY INFORMATION IS BASED ON DEEDS & PLATS OF RECORD.
 - ALL PRIVATE LINES SHOULD BE IDENTIFIED BY THE FACILITY OWNER BEFORE EXCAVATION. NOTIFY MISS UTILITY 48 HOURS PRIOR TO EXCAVATION. 1-800-257-7777
 - FENCE: ALL FENCE, NEW & EXISTING, IS TO HAVE (3) STRANDS OF BARBED WIRE INSTALLED OR REPLACED. EXISTING SUPPORT ARMS TO BE REUSED. DAMAGE SUPPORT ARMS TO BE REPLACED.
 - CONTRACTOR IS TO BE RESPONSIBLE FOR INSURING THAT TEMPORARY FENCING BE IN PLACE AT THE END OF EACH DAYS WORK IN ORDER TO MAINTAIN SECURITY REQUIREMENTS FOR THE FACILITY.

- LEGEND
- 8-1 SOIL BORING
 - LETTER INDICATES DETAIL NUMBER INDICATES SECTION
 - SHEET NUMBER WHERE SECTION OR DETAIL APPEARS
 - SHEET NUMBER WHERE SECTION OR DETAIL IS TAKEN.
 - EXISTING CONTOUR
 - PROPOSED CONTOUR

- EROSION AND SEDIMENT CONTROL NOTES
- The purpose of the erosion and sediment control measures shown on these plans shall be to preclude the transport of all waterborne sediments resulting from construction activities from entering onto adjacent properties or state waters. If field inspection reveals the inadequacy of the plan to confine sediment to the project site, appropriate modifications will be made to correct any plan deficiencies. In addition to these notes, all provisions of the Virginia Erosion and Sediment Control Regulations shall apply to this project.
- All erosion and sediment control measures shall be installed and maintained in accordance with the "Virginia Erosion and Sediment Control Handbook." The contractor shall be thoroughly familiar with all applicable measures contained therein which may be pertinent to this project.
 - All points of construction ingress and egress shall be protected by a temporary construction entrance to prevent tracking of mud onto public right-of-ways. An entrance permit from VDOT is required prior to any construction activities within State right-of-ways.
 - Sediment basins and traps, perimeter dikes, sediment barriers and other measures intended to trap sediment on-site must be constructed as a first step in grading and be made functional before any land disturbance takes place. Earthen structures such as dams, dikes, and diversions must be seeded and mulched immediately after installation. An on-site pre-construction meeting will be held between the Office of Code Compliance and the contractor to identify those measures to be initially installed.
 - Maintenance of erosion and sediment control measures shall include the repair of measures damaged by any subcontractor including those of the public utility companies. At the preconstruction meeting, the contractor will supply Code Compliance with the name of the individual who will be responsible for ensuring maintenance of installed measures on a daily basis.
 - Surface flows over cut and fill slopes shall be controlled by either redirecting flows from traversing the slopes or by installing mechanical devices to safely lower water downslope without causing erosion. A temporary fill diversion (Std. & Spec. 3.10) shall be installed prior to the end of each working day.
 - Sediment control measures may require minor field adjustments at time of construction to insure their intended purpose is accomplished. Office of Code Compliance approval will be required for other deviations from the approved plans.
 - The contractor shall place soil stockpiles at the locations shown on this plan or as directed by the engineer. Soil stockpiles shall be stabilized or protected with sediment trapping measures.
 - The contractor shall complete drainage facilities within 30 days following completion of rough grading at any point within the project. The installation of drainage facilities shall take precedence over all underground utilities. Outfall ditches from drainage structures shall be stabilized immediately after construction of same. This includes installation of erosion control stone where required. Any drainage outfalls required for a street must be completed before street grading begins.
 - Permanent or temporary soil stabilization must be applied to all denuded areas within 7 days after final grade is reached on any portion of the site. Soil stabilization must also be applied to denuded areas which may not be at final grade but will remain dormant (undisturbed) for longer than 30 days. Soil stabilization measures include vegetative establishment, mulching and the early application of gravel base material on areas to be paved.
 - No more than 300 feet of sanitary sewer, storm sewer, waterlines, or underground utility lines are to be open at one time. Following installation of any portion of these items, all disturbed areas are to be immediately stabilized (i.e., the same day).
 - If disturbed area stabilization is to be accomplished during the months of December, January, or February, stabilization shall consist of mulching in accordance with Specification 3.35. Seeding will then take place as soon as the season permits.
 - The term Seeding, Final Vegetative Cover or Stabilization, on this plan shall mean the successful germination and establishment of a stable grass cover from a properly prepared seedbed containing the specified amounts of seed, lime, and fertilizer in accordance with Specification 3.32, Permanent Seeding. Irrigation shall be required as necessary to ensure establishment of grass cover.
 - All slopes steeper than 3:1 shall require the use of erosion control blankets such as excelsior blankets to aid in the establishment of a vegetative cover. Installation shall be in accordance with Specification 3.35, Mulching and Manufacturer's Instructions.
 - Inlet protection in accordance with Specification 3.07 shall be provided for all storm drain inlets as soon as practical following construction of same.
 - Temporary liners, such as polyethylene sheets, shall be provided for all drainage ditches until the permanent concrete liner is installed.
 - Paved ditches shall be required wherever erosion is evident. Particular attention shall be paid to those areas where grades exceed 3 percent.
 - Temporary erosion control measures are not to be removed until all disturbed areas are stabilized. After stabilization is complete, all measures shall be removed within 30 days. Trapped sediment shall be spread and seeded.
 - Off-site waste or borrow areas shall be approved by the Office of Code Compliance prior to the import of any borrow or export of any waste to or from the project site.
 - All paved and/or piped outfalls will be constructed before road grading and utility installation begins.

DC/bkh
2480c

CITY OF JAMES CITY
FINAL SITE PLAN

APPROVALS DATE

Fire Dept.
Health Dept.
Highway Dept.
Plan Comm.
Pub. Wks. Dept.
Zoning Adm.

JAN 1993
RECEIVED
PLANNING DEPARTMENT

No.	Date	Description

ENGINEERS · ARCHITECTS · SURVEYORS
INTERIOR DESIGNERS · PLANNERS

SUBURBAN PROPANE GAS
OF WILLIAMSBURG, VIRGINIA

JAMES CITY COUNTY

SITE PLAN

AS SHOWN SCALE

DESIGNED RSP

DRAWN TCS

CHECKED RSP

DATE JAN. 04, 1993

1920770

COMMISSION NO. SHEET

C - 1

1 OF 2

Proposed Stormwater Management Improvements

Suburban Propane Gas Corporation

Merrimac Trail

Williamsburg, Virginia



THE DEYOUNG-JOHNSON GROUP, INC.

Proposed Stormwater Management Improvements Suburban Propane Gas Corporation

PURPOSE:

In September 1992, DJG, Inc. was retained by Suburban Propane Gas Corporation to assess the impact of stormwater runoff on their property on Route 143 in James City County and to determine the feasibility of constructing a stormwater management system to reduce the risk of property damage from stormwater entering and leaving the site.

SITE DESCRIPTION:

The site is located in James City County on Route 143 at the intersection of Colony Circle about 1 1/2 miles west of the intersection of Route 199 . It consist of approximately 1.1 acres bounded on the south by Colony Circle, on the west by Route 143, on the north by the Merrimac Motel and on the east by a parcel containing several mobile home sites. The site is fully occupied with buildings, parking lot, gravel storage yard and 600 to 800 square feet of grass lawn. It slopes uniformly from south to north at about 2 %.

EXISTING STORMWATER SYSTEMS:

There are no collection or control structures on the property. Stormwater falling on the property runs off at its northern extremity onto the mobile home and motel properties. Gravel fill along these boundaries is subject to erosion and the existing security fencing is being undermined. This condition was aggravated by a significant amount of runoff entering the property due to deficiencies in the Virginia Department of Transportation drainage system. Improvements made by VDOT during the month of October 1992 have essentially eliminated the offsite contribution.

14 hour detention time

6.1 → 3.8 cfs

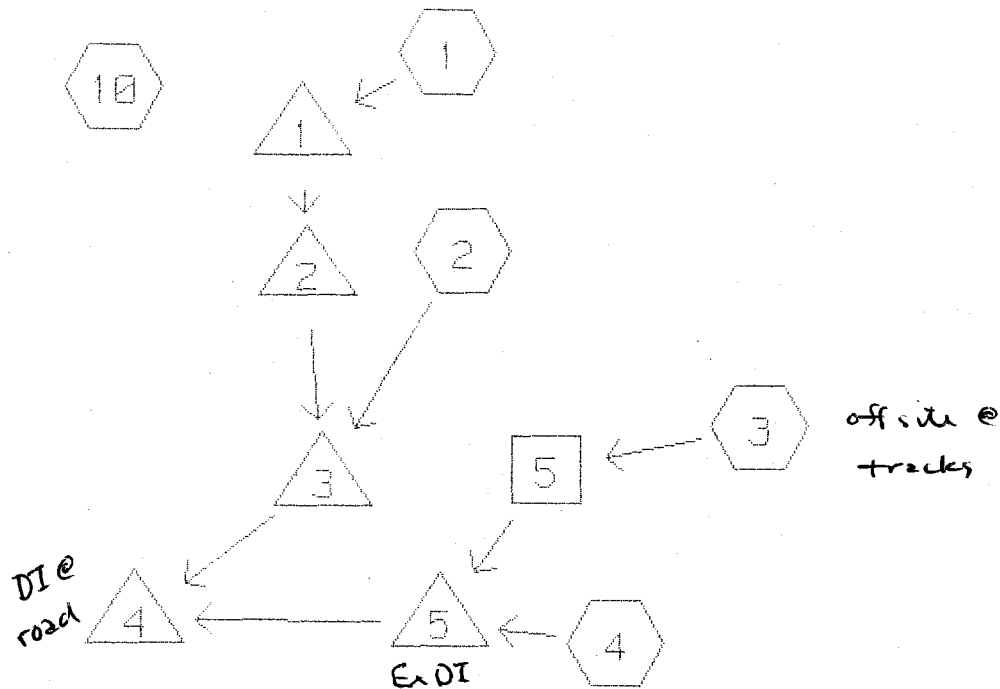
DRAINAGE ANALYSIS:

The numerical analysis contained in the following pages was accomplished with the aid of a computer program using the Soil Conservation Service (SCS) method for stormwater hydrology as outlined in Technical Release No.20 (TR-20). A Type II storm distribution for a 10 year rainfall event was selected as the appropriate basis for analysis. Drainage areas and runoff patterns were estimated using field observation and measurements and plans obtained from the Virginia Department of Transportation and the James City County Planning Department.

SUMMARY:

The DJG analysis indicates that the correction of the functional deficiencies in the VDOT drainage system will reduce the amount of runoff leaving the property at its northern boundaries by as much as 44%. The proposed subsurface detention system will essentially eliminate the remaining 56% by diverting into the existing VDOT drainage system. The rate of runoff will be structurally controlled to prevent overloading of the system.

WATERSHED ROUTING =====



SUBCATCHMENT



REACH



POND



LINK

SUBCATCHMENT 1

STORAGE YARD

ACRES	CN	C= .90
.82	87	GRAVEL STORAGE YARD

SCS TR-20 METHOD

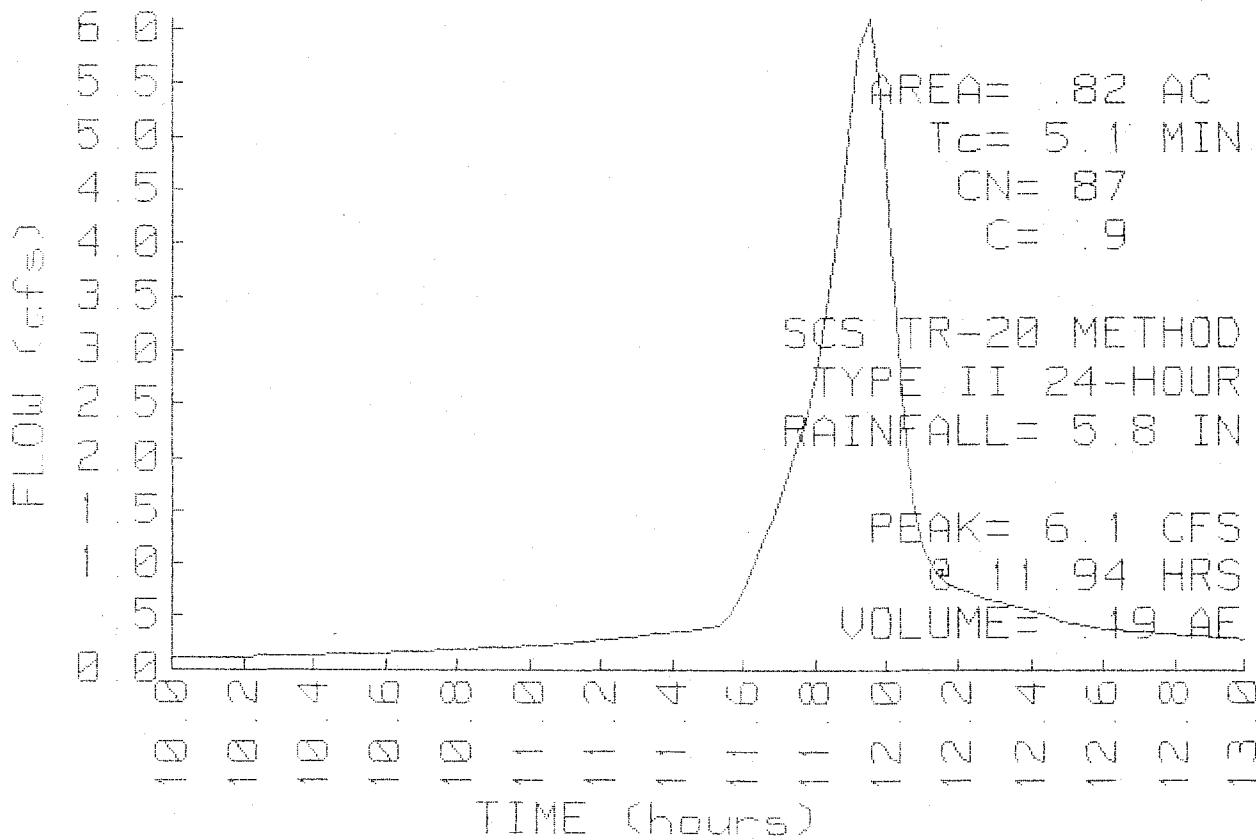
TYPE II 24-HOUR

RAINFALL= 5.8 IN

PEAK= 6.1 CFS @ 11.94 HRS

VOLUME= .19 AF

Method	Comment	Tc (min)
CURVE NUMBER (LAG) METHOD	REAR STORAGE YARD	5.1
L=210' s=.02 '/'		

SUBCATCHMENT 1 RUNOFF
STORAGE YARD

SUBCATCHMENT 2

FRONT PARKING LOT

ACRES	CN	C= .99
.28	98	ROOF & PAVEMENT

SCS TR-20 METHOD

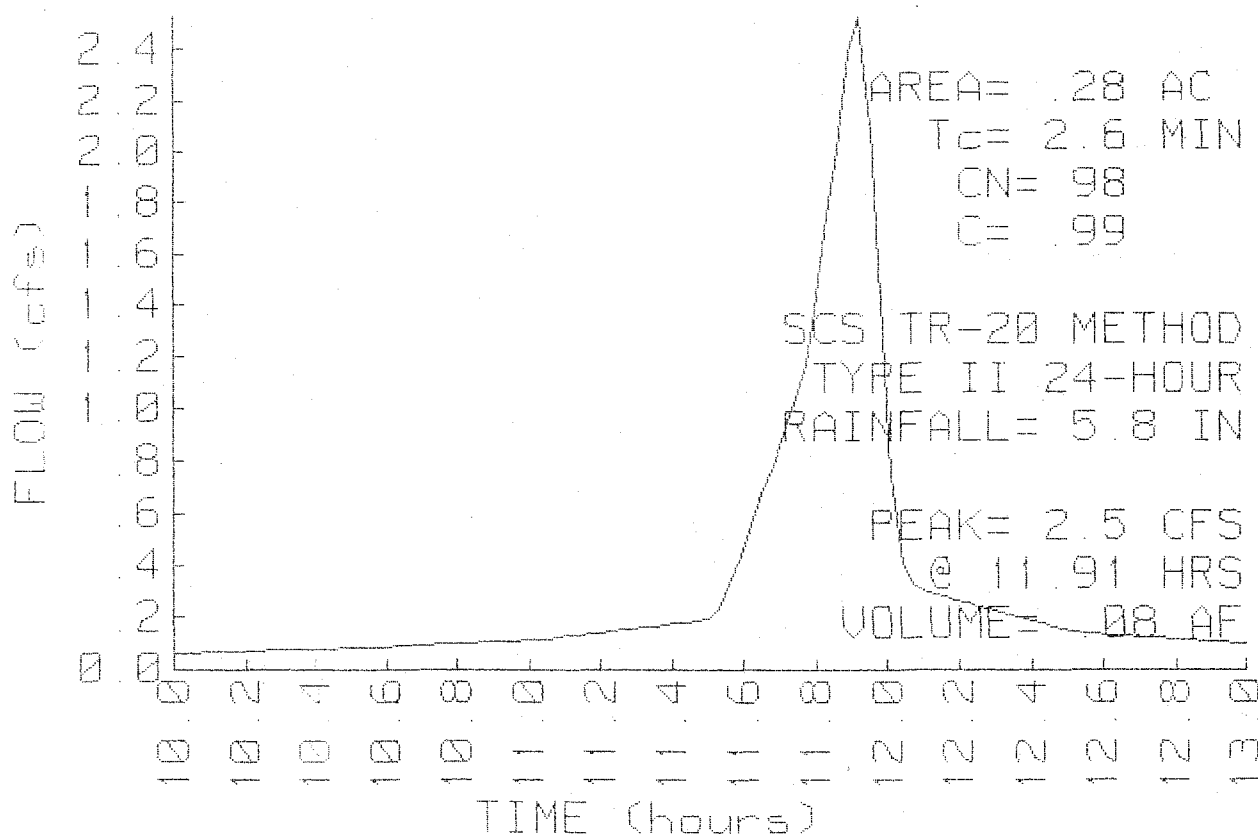
TYPE II 24-HOUR

RAINFALL= 5.8 IN

PEAK= 2.5 CFS @ 11.91 HRS

VOLUME= .08 AF

Method	Comment	Tc (min)
CURVE NUMBER (LAG) METHOD	PARKING LOT	2.6
L=170' s=.02 '/'		

SUBCATCHMENT 2 RUNOFF
FRONT PARKING LOT

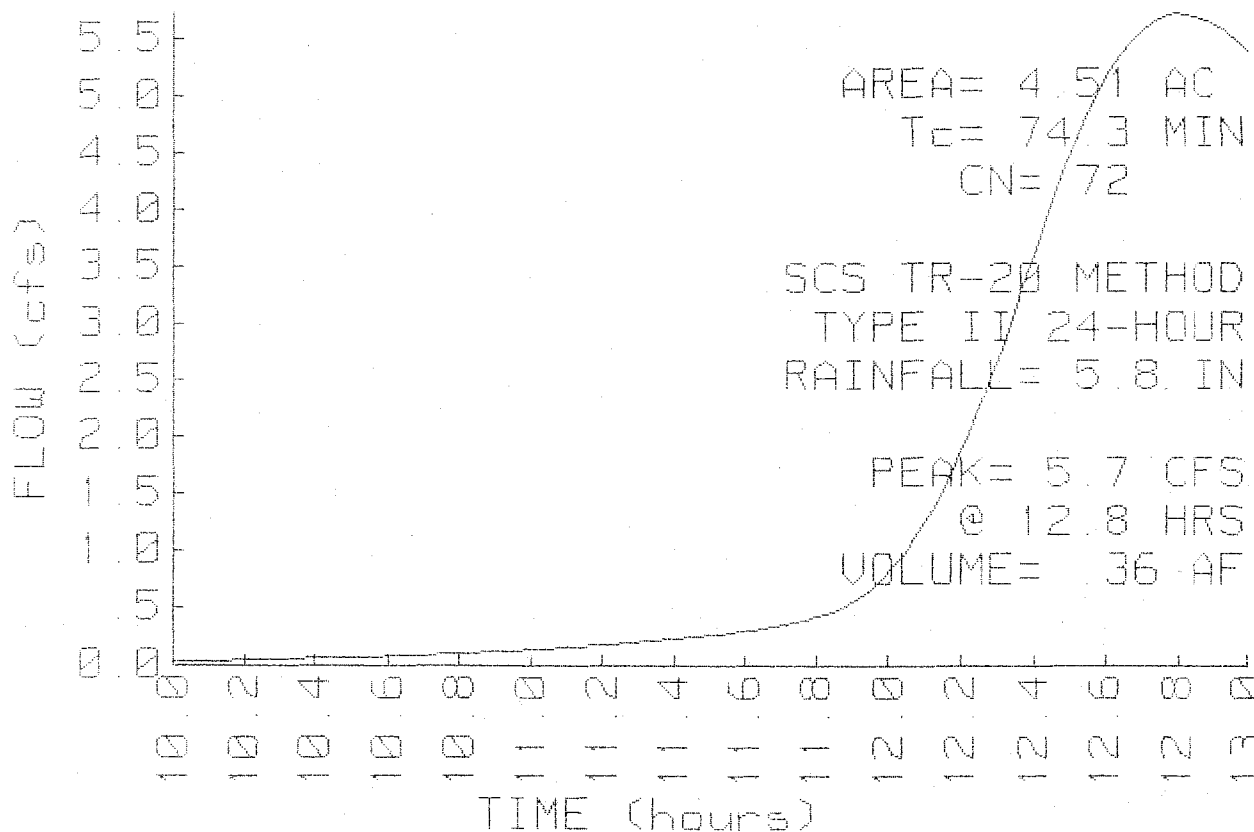
SUBCATCHMENT 3 OFF SITE DRAINAGE AREA

ACRES	CN	
2.97	60	WOODS
.83	98	PAVEMENT
.25	98	ROOFS
.46	87	GRAVEL DRIVEWAYS
4.51	72	

SCS TR-20 METHOD
 TYPE II 24-HOUR
 RAINFALL= 5.8 IN
 PEAK= 5.7 CFS @ 12.80 HRS
 VOLUME= .36 AF

Method	Comment	Tc (min)
CURVE NUMBER (LAG) METHOD	MERRIMAC TRAIL WYE	74.3
L=1100' s=.0034 '/'		

SUBCATCHMENT 3 RUNOFF
 OFF SITE DRAINAGE AREA



REACH 5

VDOT EXISTING STORM SEWER FROM DITCH

DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.0	0.0	0.0
.1	.1	.1
.3	.2	.3
.4	.3	.7
.9	.9	2.9
1.0	1.1	3.4
1.1	1.2	3.7
1.2	1.2	3.7
1.2	1.2	3.7
1.3	1.2	3.5

15" PIPE

n= .013

LENGTH= 92 FT

SLOPE= .0029 FT/FT

2.4%

- engineer redid calcs

w/ proper slope - no impact
to design.

STOR-IND METHOD

MAX. DEPTH= 1.25 FT

PEAK VELOCITY= 3.2 FPS

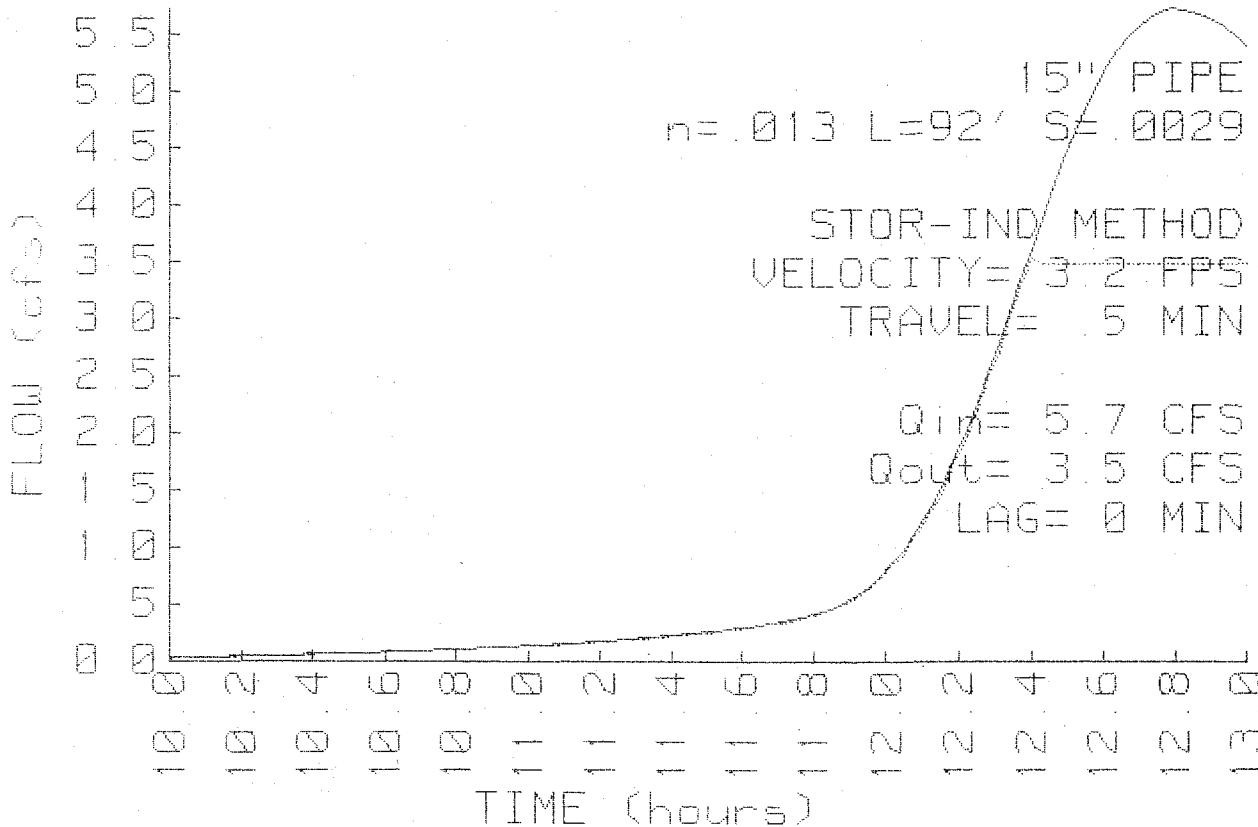
TRAVEL TIME = .5 MIN

Qin = 5.7 CFS @ 12.80 HRS

Qout = 3.5 CFS @ 12.41 HRS

ATTEN= 38 % LAG= 0.0 MIN

IN/OUT= .36 / .27 AF

REACH 5 INFLOW & OUTFLOW
VDOT EXISTING STORM SEWER FROM DITCH

POND 1

SUBSURFACE DETENTION POND 4250 s.f.

STARTING ELEV= 70.5 FT

FLOOD ELEV= 74.0 FT

ELEVATION (FT)	CUM.STOR (CF)
70.5	0
71.0	744
72.3	5270
74.0	5350

STOR-IND METHOD

PEAK ELEVATION= 72.6 FT

PEAK STORAGE = 5283 CF

Qin = 6.1 CFS @ 11.94 HRS

Qout = 3.8 CFS @ 12.03 HRS

ATTEN= 38 % LAG= 5.4 MIN

IN/OUT= .19 / .09 AF

INVERT (FT) OUTLET DEVICES

70.5	2" ORIFICE $Q = .82 \text{ PI } r^2 \text{ SQR}(2g) \text{ SQR}(H-r)$
72.0	3' BROAD-CRESTED RECTANGULAR WEIR $Q = C L H^{1.5} \quad C = 2.61, 2.67, 2.66, 2.72, 0, 0, 0, 0$
71.5	2" ORIFICE $Q = .82 \text{ PI } r^2 \text{ SQR}(2g) \text{ SQR}(H-r)$

TOTAL DISCHARGE vs ELEVATION

FEET	0.0	.1	.2	.3	.4	.5	.6	.7	.8	.9
70.5	0.0	0.0	0.0	.1	.1	.1	.1	.1	.1	.1
71.5	.1	.2	.2	.2	.2	.3	.5	1.0	1.6	2.3
72.5	3.1	4.0	5.0	6.1	7.2	8.4	9.6	10.9	12.3	13.7
73.5	15.1	16.7	18.3	20.0	21.8	23.6				

POND 2

REACH 2 SIMULATION

STARTING ELEV= 70.5 FT

FLOOD ELEV= 74.0 FT

ELEVATION (FT)	CUM.STOR (CF)
70.5	0
72.0	6
73.0	30
74.0	54

STOR-IND METHOD

PEAK ELEVATION= 71.8 FT

PEAK STORAGE = 5 CF

Qin = 3.8 CFS @ 12.03 HRS

Qout = 3.8 CFS @ 12.03 HRS

ATTEN= 0 % LAG= 0.0 MIN

IN/OUT= .09 / .09 AF

INVERT (FT) OUTLET DEVICES

70.5	15" CULVERT ✓ $n = .01 \quad L = 65' \quad S = .0015'/' \quad Ke = .5 \quad Cc = .9 \quad Cd = .6 \quad TW = 1.3'$
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TOTAL DISCHARGE vs ELEVATION

FEET	0.0	.1	.2	.3	.4	.5	.6	.7	.8	.9
70.5	0.0	0.0	.1	.2	.4	.7	1.0	1.3	1.6	2.0
71.5	2.4	2.8	3.2	3.6	4.0	4.3	4.6	4.7	5.1	5.5
72.5	5.9	6.2	6.5	6.8	7.1	7.4	7.7	7.9	8.2	8.4
73.5	8.7	8.9	9.1	9.3	9.5	9.8				

POND 3

PROPOSED 4 FT. DIAM. D.I.

STARTING ELEV= 70.4 FT
 FLOOD ELEV= 74.5 FT

ELEVATION (FT)	CUM.STOR (CF)
70.4	0
71.4	13
72.4	25
73.4	38
74.4	50

STOR-IND METHOD
 PEAK ELEVATION= 71.8 FT
 PEAK STORAGE = 18 CF
 Qin = 4.2 CFS @ 12.03 HRS
 Qout= 4.2 CFS @ 12.03 HRS
 ATTEN= 0 % LAG= 0.0 MIN
 IN/OUT= .17 / .17 AF

INVERT (FT) OUTLET DEVICES
 70.4 15" CULVERT
 n=.01 L=65' S=.0015'/' Ke=.5 Cc=.9 Cd=.6 TW=3.6'

TOTAL DISCHARGE vs ELEVATION

FEET	0.0	.1	.2	.3	.4	.5	.6	.7	.8	.9
70.4	0.0	0.0	.1	.2	.4	.7	1.0	1.3	1.6	2.0
71.4	2.4	2.8	3.2	3.6	4.0	4.3	4.6	4.7	5.1	5.5
72.4	5.9	6.2	6.5	6.8	7.1	7.4	7.7	7.9	8.2	8.4
73.4	8.7	8.9	9.1	9.3	9.5	9.8	10.0	10.2	10.4	10.6
74.4	10.7									

POND 4

EXISTING 4 FT. DIAM. D.I. @ STA. 1269+50

STARTING ELEV= 70.3 FT
 FLOOD ELEV= 74.3 FT

ELEVATION (FT)	CUM.STOR (CF)
70.3	0
71.3	13
72.3	25
73.3	38
74.3	50

STOR-IND METHOD
 PEAK ELEVATION= 73.9 FT
 PEAK STORAGE = 44 CF
 Qin = 10.2 CFS @ 11.91 HRS
 Qout= 10.1 CFS @ 11.91 HRS
 ATTEN= 0 % LAG= .1 MIN
 IN/OUT= .66 / .66 AF

INVERT (FT) OUTLET DEVICES
 70.3 15" CULVERT
 n=.013 L=92' S=.05'/' Ke=.5 Cc=.9 Cd=.6 TW=4'

TOTAL DISCHARGE vs ELEVATION

FEET	0.0	.1	.2	.3	.4	.5	.6	.7	.8	.9
70.3	0.0	0.0	.2	.4	.7	1.1	1.5	2.0	2.5	3.1
71.3	3.6	4.1	4.5	4.9	5.2	5.5	5.8	6.1	6.4	6.7
72.3	6.9	7.2	7.4	7.6	7.9	8.1	8.3	8.5	8.7	8.9
73.3	9.1	9.3	9.5	9.7	9.8	10.0	10.2	10.4	10.5	10.7
74.3	10.9									

Data for SUBURBAN PROPANE, WILLIAMSBURG

Prepared by DJG, INC. WILLIAMSBURG, VA.

HydroCAD 3.02 000388 (c) 1986-1992 Applied Microcomputer Systems

Page 9

9 Jan 9

POND 5

EXISTING 4 FT. DIAM. D.I.@ STA.1268+35

STARTING ELEV= 75.4 FT

FLOOD ELEV= 78.5 FT

ELEVATION (FT)	CUM.STOR (CF)
75.4	0
76.4	13
77.4	25
78.4	38

STOR-IND METHOD

PEAK ELEVATION= 77.6 FT

PEAK STORAGE = 27 CF

Qin = 7.4 CFS @ 11.91 H

Qout= 7.4 CFS @ 11.91 H

ATTEN= 0 % LAG= .1 M

IN/OUT= .48 / .48 A

INVERT (FT)

OUTLET DEVICES

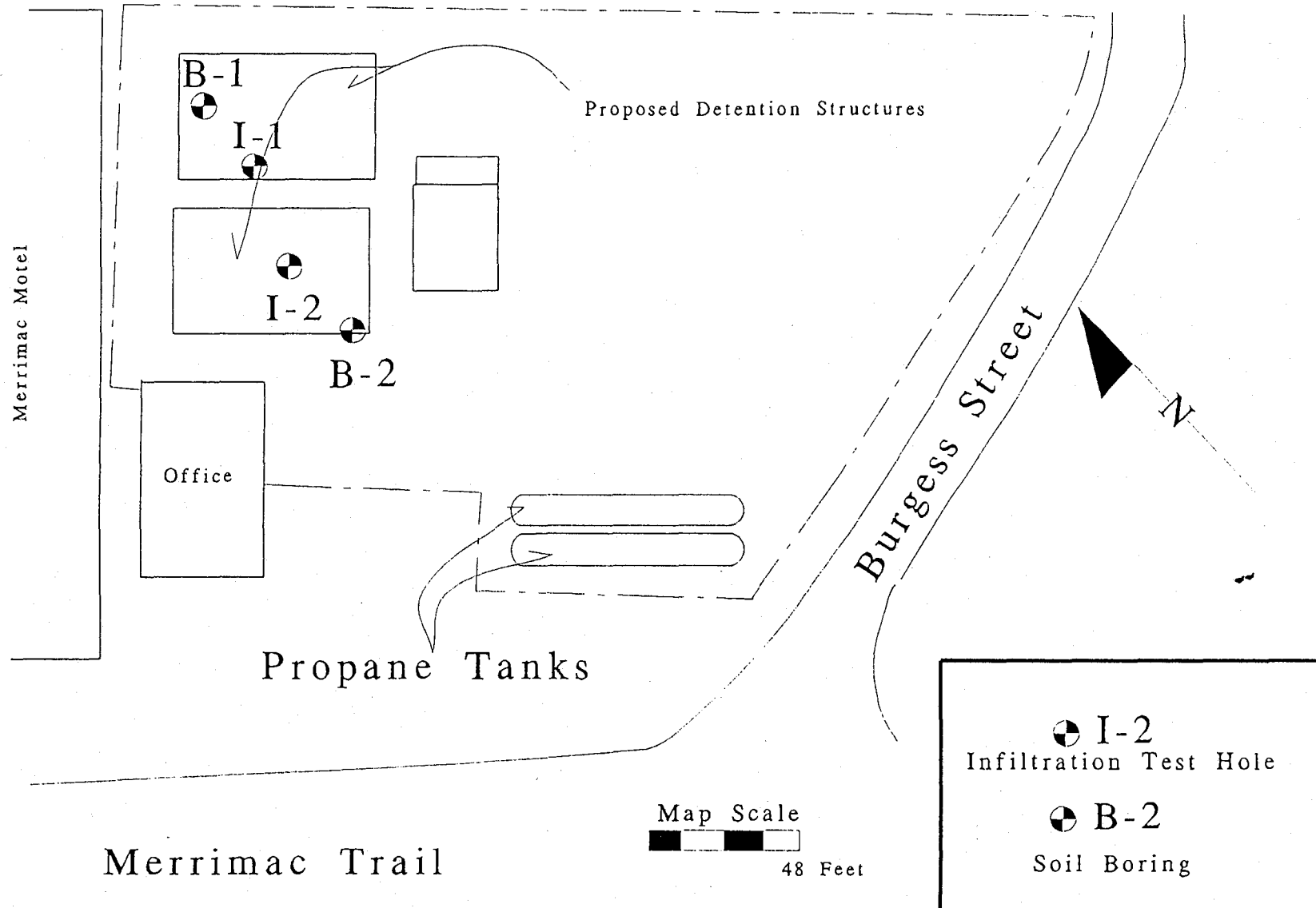
75.4

15" CULVERT

n=.013 L=110' S=.05'/' Ke=.5 Cc=.9 Cd=.6 TW=3.6'

TOTAL DISCHARGE vs ELEVATION

FEET	0.0	.1	.2	.3	.4	.5	.6	.7	.8	.
75.4	0.0	0.0	.2	.4	.7	1.1	1.5	2.0	2.5	3.
76.4	3.6	4.1	4.5	4.9	5.2	5.5	5.8	6.1	6.4	6.
77.4	6.9	7.2	7.4	7.6	7.9	8.1	8.3	8.5	8.7	8.
78.4	9.1									



Suburban Propane
Williamsburg, Virginia

Boring Location Plan

File: R-3246-92

Drawn By: BLK

Des. By: BLK

Date: 12/22/92

App. by: JHB

Figure: 2

COMBINED TECHNOLOGIES, INC.
510 Eastpark Court, Suite 120
Sandston, Virginia 23150
(804) 737-0076

BORING LOG

Project Name		Water Level Data		Date Started	Boring
Suburban Propane		Dry Upon Completion		December 7, 1992	B-1
Size & Bit	Job Number			Date Completed	Page
6" Cutter Head	R-3516-92			December 7, 1992	1 of 1
Drilling Method		Driller	Total Depth		Elevation
Hollow Stem Augers		Ayers & Ayers	12.5 feet		N/A

Depth in Feet	Classification	Sample No.	From To	Blows/ Foot*	Laboratory Results	Remarks
0.0	Stiff orangish brown clayey Silt with granite and quartz gravel (Probable Fill)	1	0.5' 1.5'	9		
2.5	Loose moist yellowish brown sandy Clay with some quartz (Probable Fill)	2	1.5' 2.5'	7		
5.0	Stiff moist yellowish brown sandy Silt with gray mottling	3	4.5' 5.5'	10		
7.5		4	7.5' 8.5'	15		
10.0	Stiff to very stiff moist orangish brown sandy Clay	5	9.5' 10.5'	16		
12.5	Boring Terminated at 12.5 feet	6	11.5 12.5'	21		
15.0						
17.5						
20.0						

BORING LOG

Project Name		Water Level Data		Date Started	Boring	
Suburban Propane		Dry Upon Completion		December 7, 1992	B-2	
Size & Bit	Job Number			Date Completed	Page	
6" Cutter Head	R-3516-92			December 7, 1992	1 of 1	
Drilling Method		Driller	Total Depth		Elevation	
Hollow Stem Augers		Ayers & Ayers	12.5 feet		N/A	
Depth in Feet	Classification	Sample No.	From To	Blows/ Foot*	Laboratory Results	Remarks
2.5	Very firm rounded gravel with quartz fragments	1	0.5' 1.5'	22		
	Very firm brownish orange silty fine Sand	2	2.5' 3.5'	25		
5.0		3	4.5' 5.5'	26		
		4	7.5' 8.5'	18		
7.5	Very stiff light brown silty Clay					
10.0	Very stiff brownish orange clayey Silt	5	9.5' 10.5'	17		
	Firm brownish orange silty Sand	6	11.5' 12.5'	14		
12.5	Boring Terminated at 12.5 feet					
15.0						
17.5						
20.0						



Date Record Created:

Created By:

WS_BMPNO:

CC007

Print Record

PRINTED ON

Wednesday, March 10, 201

2:30:18 PM

WATERSHED

CC

BMP ID NO

007

PLAN NO

SP-01-93

TAX PARCEL

(41-4)(7-6)

PIN NO

4140700006

CONSTRUCTION DATE

7/1/1993

PROJECT NAME

Suburban Propane

FACILITY LOCATION

7232 Merrimac Trail

CITY-STATE

Williamsburg, VA

CURRENT OWNER

Suburban Propane LP

OWNER ADDRESS

One Suburban Plaza

OWNER ADDRESS 2

240 Rt 10 West

CITY-STATE-ZIP CODE

Whippany, NJ 07981

OWNER PHONE

MAINT AGREEMENT

Yes

EMERG ACTION PLAN

No

Get Last BMP No

Return to Menu

MAINTENANCE PLAN

No

SITE AREA acre

1.1

LAND USE

COMM

old BMP TYP

Infiltrators

JCC BMP CODE

C1 Infiltration Trench .5

POINT VALUE

9

SVC DRAIN AREA acres

6.3

SERVICE AREA DESCRI

SFRES&WOODS TO SOUTHEAST-5 ACR

IMPERV AREA acres

RECV STREAM

TUTTERSNECK PND

EXT DET-WQ-CTRL

No

WTR QUAL VOL acre-ft

0.1

CHAN PROT CTRL

No

CHAN PROT VOL acre-ft

0

SW/FLOOD CONTROL

No

GEOTECH REPORT

No

CTRL STRUC DESC

CTRL STRUC SIZE inches

OTLT BARRL DESC

TRUSS P.

OTLT BARRL SIZE Inch

15

EMERG SPILLWAY

No

DESIGN HW ELEV

PERM POOL ELEV

2-YR OUTFLOW cfs

10-YR OUTFLOW cfs

REC DRAWING

No

CONSTR CERTIF

No

LAST INSP DATE

3/20/2001

Inspected by:

INTERNAL RATING

3

MISC/COMMENTS

Additional Comments: