



CERTIFICATE OF AUTHENTICITY

THIS IS TO CERTIFY THAT THE FOLLOWING ELECTRONIC RECORDS ARE TRUE AND ACCURATE REPRODUCTIONS OF THE ORIGINAL RECORDS OF JAMES CITY COUNTY GENERAL SERVICES DEPARTMENT- STORMWATER DIVISION; WERE SCANNED IN THE REGULAR COURSE OF BUSINESS PURSUANT TO GUIDELINES ESTABLISHED BY THE LIBRARY OF VIRGINIA AND ARCHIVES; AND HAVE BEEN VERIFIED IN THE CUSTODY OF THE INDIVIDUAL LISTED BELOW.

BMP NUMBER: CC-018

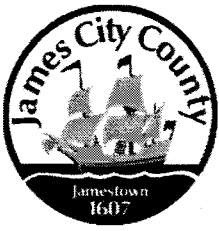
DATE VERIFIED: March 15, 2012

QUALITY ASSURANCE TECHNICIAN:

Leah Hardenbergh

Leah Hardenbergh

LOCATION: WILLIAMSBURG, VIRGINIA



Stormwater Division

MEMORANDUM

DATE: March 11, 2010
TO: Michael J. Gillis, Virginia Correctional Enterprises Document Management Services
FROM: Jo Anna Ripley, Stormwater
PO: 270712
RE: Files Approved for Scanning

General File ID or BMP ID: CC018

PIN: 5020900013

Subdivision, Tract, Business or Owner

Name (if known): Busch Corporate Center

Property Description: Site 13

Site Address: 348 Mcclaws Circle

(For internal use only)

Box 12

Drawer: 7

Agreements: (in file as of scan date)

N

Book or Doc#:

Page:

Comments

As of 3/11/2010 the owner of the property is Goodfarb Family Virginia Land LLC

CC018

Contents for Stormwater Management Facilities As-built Files

Each file is to contain:

- ① As-built plan
2. Completed construction certification
3. Construction Plan
- ④ Design Calculations
- ⑤ Watershed Map
6. Maintenance Agreement
7. Correspondence with owners
8. Inspection Records
9. Enforcement Actions

9/27/82

FIFTH AMENDMENT TO CONTRACT BETWEEN

BUSCH PROPERTIES, INC.

AND

LOUIS C. GOODFARD

for the purchase of Site 13

THIS AMENDMENT entered into by and between BUSCH PROPERTIES, INC., a Delaware Corporation (Seller) and LOUIS C. GOODFARB, Trading as VIRBY REALTY COMPANY, a sole partnership (Buyer).

WHEREAS, Seller and Buyer have heretofore entered into a contract dated July 13, 1979 and amended on July 22, 1980, July 14, 1981 and July 22, 1982, which provided that Buyer shall purchase from Seller certain property located within the Busch Corporate Center - Williamsburg, and identified as Site 13 (11.24 acres) on that certain plat entitled, "Busch Corporate Center - Williamsburg, Plat of Site 13, and 18, to be Conveyed to Louis C. Goodfarb, Trading as Virby Realty Company, from Busch Properties, Inc., James City County, Virginia", dated June 1, 1979, revised June 21, 1979, revised June 22, 1979, prepared by Langley and McDonald, Consulting Engineers, Virginia Beach, Virginia, and recorded in the Circuit Court for the City of Williamsburg and County of James City in Plat Book 35 on Page 86, and;

WHEREAS, Buyer and Seller mutually agree to alter certain terms of the aforesaid contract;

NOW, THEREFORE in consideration of and reliance upon the premises set forth above and the undertakings hereinafter set forth, the parties hereto agree as follows:

1. Plat. The aforesaid plat dated June 1, 1979, revised June 21, 1979 and June 22, 1979, shall be extinguished by Seller and a new plat entitled "Busch Corporate Center - Williamsburg, Plat of Phase IV, to be conveyed to Louis C. Goodfarb, Trading as Virby Realty Company, from Busch Properties, Inc.", dated August 4, 1982, prepared by Langley and McDonald, Consulting Engineers, Virginia Beach, Virginia incorporated herewith as Exhibit "D", and to be recorded by Seller.

2. Property. The "Property" to be purchased by Buyer is hereby reduced to 5.41 acres, to include 3.03 acres hereafter referred to as Site 13 plus the 2.38 acre storm water retention pond between Site 13 and Buyer's Phase I, II and III property, to be conveyed to Buyer at no cost as per the provisions of paragraph 19 of the Contract, and shown on Exhibit "D".

Contract

18. ROAD AND UTILITY IMPROVEMENTS. This Agreement is contingent upon Seller completing construction of an extension of McLaws Circle Road shown on Exhibit "A" from its present terminus at the eastern property line of Site 13 to the western property line of Site 12 by November 30, 1979, but only if Closing shall take place on or before August 15, 1979; however, if Closing shall not take place until after August 15, 1979, then construction of the road will be completed by July 15, 1980.

19. CONVEYANCE OF DRAINAGE POND. Seller agrees to convey at no cost to Buyer and Buyer agrees to accept title to that area ^{2.38 acres} between Phase IV and Phases I, II and III, purchased by buyer under a contract of sale with Seller dated April 13, 1977, currently used as a storm water retention pond so that the southeasterly property line of Phase IV will be coterminous with that of Phase III. Seller agrees to have his representatives on the Williamsburg Corporate Center Association Board of Trustees support a resolution for the Williamsburg Corporate Center Association to accept responsibility for the maintenance of this storm water retention pond. Buyer agrees that he will support similar resolutions regarding all other storm water retention ponds constructed or to be constructed within the area shown on Exhibit "A".

These provisions shall survive Closing.

IN WITNESS WHEREOF, the parties have executed this Agreement as of the day and year hereinbelow written. The latter of the dates appearing below shall be the effective date of this Agreement.

SELLER:

BUSCH PROPERTIES, INC.

By W. Randolph Baker

Date: 4/27/79

BUYER:

VIRBY REALTY COMPANY

By Mrs. Clotfelter

Date: 7/13/79

9/29/82

AMENDMENT

THIS AMENDMENT, entered into by and between BUSCH PROPERTIES, INC., a Delaware Corporation (Seller), and LOUIS C. GOODFARB, Trading as VIRBY REALTY COMPANY, a sole proprietorship (Buyer),

WHEREAS, Seller and Buyer have entered into a contract of sale dated July 13, 1979 which provides that Buyer shall purchase certain real estate from Seller located within the Busch Corporate Center-Williamsburg identified in said contract as "Phase IV (Contract)," and further identified as Site 13 on a plat dated June 20, 1979 by Langley and McDonald Engineers of property to be conveyed to Buyer; and

WHEREAS, Seller and Buyer desire to modify and amend the Contract as hereinafter set forth, and

WHEREAS, it will be necessary for Seller to construct a sanitary sewer line across Site 13 at a future date generally from the right of way of McLaws Circle on the east and north of Site 13 to a 30' sanitary sewer easement retained by Seller on the west and south of Site 13 to serve Buyer's property and that of others, and

WHEREAS, the location of the sanitary sewer line easement has been deferred by Seller to allow it to be located to better serve the buildings Buyer is to construct on Site 13,

NOW, THEREFORE, in consideration of and reliance upon the premises set forth above and the undertakings hereinafter set forth, the parties hereto agree as follows:

1. Buyer hereby agrees that there shall be a 20' or 30' wide sanitary sewer easement crossing Site 13 from McLaws Circle on the north or east and the 30' wide sanitary sewer easement on the south or west of Site 13.

2. The location of said easement is to be subsequently determined by Langley and McDonald Consulting Engineers in consultation with Buyer, Seller and requisite public officials.
3. Buyer hereby agrees to convey said sanitary sewer easement to Seller or James City County, as appropriate, at no cost, in a timely manner when its location has been determined. Buyer further agrees to allow Seller and its contractors, engineers, surveyors, or public officials and their equipment and material to enter onto Site 13 to survey, design and construct the sanitary sewer line.
4. The provisions of this Amendment shall survive delivery of the Deed at closing.

WITNESS the following signatures and seals:

SELLER:

BUSCH PROPERTIES, INC.

By W. L. Baker

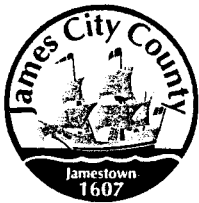
Date 9/27/82

BUYER:

VIRBY REALTY COMPANY

By Ther. Cloninger

Date 9/8/82



DEVELOPMENT MANAGEMENT

101-E MOUNTS BAY ROAD, P.O. BOX 8784, WILLIAMSBURG, VIRGINIA 23187-8784
(757) 253-6671 Fax: (757) 253-6850 E-MAIL: devtman@james-city.va.us

CODE COMPLIANCE

(757) 253-6626

codecomp@james-city.va.us

ENVIRONMENTAL DIVISION

(757) 253-6670

environ@james-city.va.us

PLANNING

(757) 253-6685

planning@james-city.va.us

COUNTY ENGINEER

(757) 253-6678

INTEGRATED PEST MANAGEMENT

(757) 253-2620

April 22, 1998

Mr. Paul Cathcart
Printpack, Inc.
400 Packets Court
Williamsburg, VA 23185

RE: Printpack Expansion and BMP Maintenance

Dear Mr. Cathcart:

Attached are documents from the Busch Corporate Center Association that address the long-term maintenance of the Best Management Practice (BMP), or detention pond adjacent to the Printpack property. The information presents that the BMP's maintenance is the responsibility of the Association. This information satisfies the concerns the Environmental Division raised during the review of the Printpack expansion site plan allowing approval of the plan by the Division.

Please contact me at 253-6673 if you have any further questions.

Sincerely,

Darryl E. Cook, P.E.
Environmental Director



April 15, 1998

Mr. Darryl Cook
Civil Engineer, Environmental Services
James City County
P.O. Box 8784
Williamsburg, Virginia 23187

RE: Retention Pond - Busch Corporate Center

Dear Mr. Cook:

Please find attached property transfer documents identifying the responsibility of drainage maintenance and repair for the retention pond located in the Busch Corporate Center as the responsibility of the Busch Corporate Center Association.

Should the County, during the course of inspections, find the need to repair or address maintenance issues relative to the drainage system within the Busch Corporate Center please contact the Association office at 253-3950.

If we may be of further assistance please contact us.

Sincerely,

A handwritten signature in black ink, appearing to read "William B. Voliva, Jr.", written over a horizontal line.

William B. Voliva, Jr.
President
Busch Corporate Center Association

cc: Mr. William F. Brown

Attachments



MEMORANDUM

USEG/FILE COPY

Langley and McDonald, P.C.

Engineers
Surveyors
Planners
Landscape Architects
Environmental Consultants

Main Office: 5544 Greenwich Road, Virginia Beach, VA 23462
(757) 473-2000 FAX#: (757) 497-7933
Williamsburg Office: 201 Packets Court, Wmbg, VA 23185
(757) 253-2975 FAX#: (757) 229-0049
e-mail address: langley @norfolk.infi.net

CC-018

To: Darryl Cook, James City County

From: Diana Browne, L&M

Date: January 29, 1998

Subject: Printpack Expansion
L&M 86120-103

As requested, enclosed please find additional calculations regarding the 100-year storm for the referenced project. The results indicate that the existing 100-year flood elevation for the detention pond is 73.23 feet, which overtops the dam crest (dam crest elevation is 73.1). Under the proposed conditions, the 100-year flood elevation is 73.4 feet.

Please call us if you have any questions or comments.

Printpack Plant Expansion Stormwater Management

L&M Job 86120-103

January 16, 1998

Printpack is located at 400 Packets Court in the Busch Corporate Center in James City County. The Printpack property is approximately 9.7 acres in area, and consists of two buildings, paved parking and sidewalks, and open areas. Printpack is proposing to expand their plant by constructing a 70' x 160' addition to one of the existing buildings, along with a new gravel road for fire truck access. Since the building expansion will be located over an existing paved area, there will be no increase in impervious area with the proposed expansion.

The Printpack site currently drains to an off-site dry detention pond that is owned by Busch. The drainage area to the pond is 18.7 acres, which includes the Printpack site, as well as Eastern International and the adjacent parking lot. No increase in peak flowrates to the pond from the contributing watershed are anticipated with the proposed plant expansion.

Under the County's Chesapeake Bay Preservation Regulations, the proposed expansion is considered redevelopment, and a 10% reduction in pollutant loads from the project is required. Since the site currently drains to a stormwater management pond, James City County has indicated that no additional treatment is necessary as long as the pond is in good operating condition. Based upon field observations, the existing outfall structure is a 24" CMP riser with 1" diameter holes around its perimeter, and a 24" CMP barrel. The riser structure needs to be upgraded for the pond to be considered in good operating condition.

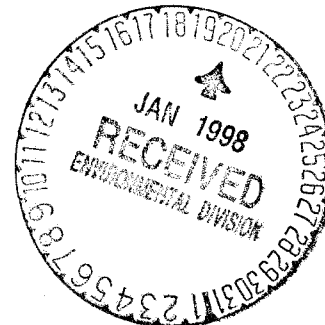
A 48" RCP riser is proposed to replace the existing 24" CMP riser. The riser crest elevation will increase from the existing elevation of 67.43 to 70.6 to allow for additional water quality drawdown volume. The volume of water at Elev. 70.6 (41,347 c.f.) is equivalent to 0.6 inches of runoff over the 18.7-acre drainage area. An 8" diameter orifice at the bottom of the riser is proposed to allow gradual drawdown for water quality enhancement. No changes to the barrel or dam are anticipated. The modeling results for existing and proposed conditions are as follows:



Langley and McDonald, P.C.

Engineers
Surveyors
Planners
Landscape Architects
Environmental Consultants

GEORGE E. LANGLEY
Consultant
T. JOSEPH McDONALD
1906-1982



January 19, 1998

Mr. Darryl E. Cook
Environmental Director
James City County
P.O. Box 8784
Williamsburg, VA 23187-8784

Re: Retrofit of BMP at Packets Court for Printpack Inc.

Dear Darryl:

Thank you for your prompt reply to our request for a meeting with Diana Brown and myself yesterday. Both Diana and I were pleased with the meeting and have worked to meet the modified criteria you spoke of.

The results of this are provided at enclosure, Printpack Plant Expansion Stormwater Management, Langley and McDonald Job Number 86120-103, January 16, 1998.

As this project is a County expedited review process, we request your comments as soon as possible so that we can make any necessary changes and/or adjustments.

Respectfully yours,

LANGLEY AND McDONALD, P.C.

Willard E. Gwilliam, F.A.I.A.
Senior Project Manager

WEG/tmp
Enclosure

cc: Mr. Paul Holt, w/ enclosure
Mr. Paul Cathcart, w/enclosure

100 year storm ?

MAIN OFFICE
5544 Greenwich Road
Virginia Beach, VA 23462
(757) 473-2000
FAX: (757) 497-7933

201 Packets Court
Williamsburg, VA 23185
(757) 253-2975
FAX: (757) 229-0049

**PRINTPACK PLANT EXPANSION
STORMWATER MANAGEMENT**

**L&M JOB 86120-103
JANUARY 16, 1998**

Printpack Plant Expansion Stormwater Management

L&M Job 86120-103

January 16, 1998

Printpack is located at 400 Packets Court in the Busch Corporate Center in James City County. The Printpack property is approximately 9.7 acres in area, and consists of two buildings, paved parking and sidewalks, and open areas. Printpack is proposing to expand their plant by constructing a 70' x 160' addition to one of the existing buildings, along with a new gravel road for fire truck access. Since the building expansion will be located over an existing paved area, there will be no increase in impervious area with the proposed expansion.

The Printpack site currently drains to an off-site dry detention pond that is owned by Busch. The drainage area to the pond is 18.7 acres, which includes the Printpack site, as well as Eastern International and the adjacent parking lot. No increase in peak flowrates to the pond from the contributing watershed are anticipated with the proposed plant expansion.

Under the County's Chesapeake Bay Preservation Regulations, the proposed expansion is considered redevelopment, and a 10% reduction in pollutant loads from the project is required. Since the site currently drains to a stormwater management pond, James City County has indicated that no additional treatment is necessary as long as the pond is in good operating condition. Based upon field observations, the existing outfall structure is a 24" CMP riser with 1" diameter holes around its perimeter, and a 24" CMP barrel. The riser structure needs to be upgraded for the pond to be considered in good operating condition.

A 48" RCP riser is proposed to replace the existing 24" CMP riser. The riser crest elevation will increase from the existing elevation of 67.43 to 70.6 to allow for additional water quality drawdown volume. The volume of water at Elev. 70.6 (41,347 c.f.) is equivalent to 0.6 inches of runoff over the 18.7-acre drainage area. An 8" diameter orifice at the bottom of the riser is proposed to allow gradual drawdown for water quality enhancement. No changes to the barrel or dam are anticipated. The modeling results for existing and proposed conditions are as follows:

Existing Conditions

Storm Event	Peak Inflow	Peak Outflow	Peak Elevation
2-year	54 cfs	26 cfs	70.16
10-year	91 cfs	32 cfs	71.69

Drawdown time from Elev 70.6 = 3.7 hours

67.43 - riser

Proposed Conditions

Storm Event	Peak Inflow	Peak Outflow	Peak Elevation
2-year	54 cfs	24 cfs	71.20
10-year	91 cfs	47 cfs	72.10

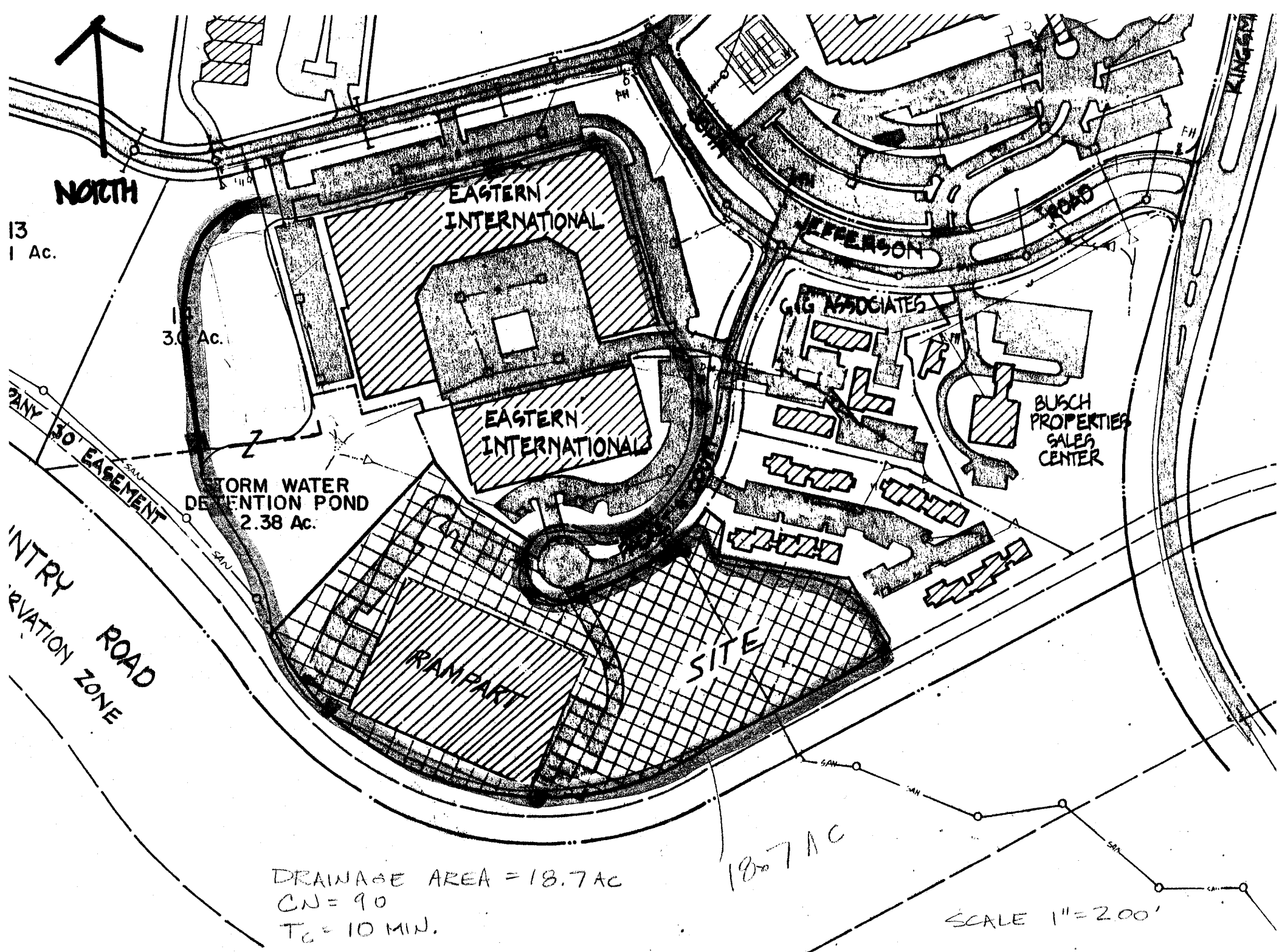
Drawdown time from Elev 70.6 = 5.1 hours

70.6 - riser
elev

Under proposed conditions,

- there is a reduction in the 2-year peak flowrate from 26 cfs to 24 cfs.
- the drawdown time for water quality enhancement is increased.
- the 10-year peak elevation increases by 0.4 feet; however, 1 foot of freeboard is maintained (top of dam elev. = 73.1).

With the proposed changes in outlet structure, the pond should continue to provide stormwater management for the entire 18.7-acre drainage area.



EXISTING CONDITIONS

TR-55 TABULAR HYDROGRAPH METHOD

Type II. Distribution
(24 hr. Duration Storm)

Executed: 01-16-1998 11:11:39

Watershed file: --> C:\86120--1\WSHED .MOP

Hydrograph file: --> C:\86120--1\SITE2YR.HYD

PrintPack

86120-103

Hydrographs for drainage area to existing BMP
No changes in hydrographs for Print Pack project

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip. (in)	Runoff (in)	Ia/p input/used
area to BMP	18.70	90.0	0.20	0.00	3.36	2.32	I.07 .10

* Travel time from subarea outfall to composite watershed outfall point.

I -- Subarea where user specified interpolation between Ia/p tables.

Total area = 18.70 acres or 0.02922 sq.mi

Peak discharge = 54 cfs

>>>> Computer Modifications of Input Parameters <<<<

Subarea Description	Input Values		Rounded Values		Ia/p	Ia/p Messages
	Tc (hr)	* Tt (hr)	Tc (hr)	* Tt (hr)	Interpolated (Yes/No)	
area to BMP	0.17	0.00	0.20	0.00	No	Computed Ia/p < .1

* Travel time from subarea outfall to composite watershed outfall point.

TR-55 TABULAR HYDROGRAPH METHOD
Type II. Distribution
(24 hr. Duration Storm)

Executed: 01-16-1998 11:11:39
Watershed file: --> C:\86120--1\WSHED .MOP
Hydrograph file: --> C:\86120--1\SITE2YR.HYD

PrintPack
86120-103

Hydrographs for drainage area to existing BMP
No changes in hydrographs for Print Pack project

>>>> Summary of Subarea Times to Peak <<<<

Subarea	Peak Discharge at Composite Outfall (cfs)	Time to Peak at Composite Outfall (hrs)
----- area to BMP -----	----- 54 -----	----- 12.2 -----
Composite Watershed	54	12.2

TR-55 TABULAR HYDROGRAPH METHOD

Type II. Distribution
(24 hr. Duration Storm)

Executed: 01-16-1998 11:11:39

Watershed file: --> C:\86120--1\WSHED .MOP

Hydrograph file: --> C:\86120--1\SITE2YR.HYD

PrintPack
86120-103Hydrographs for drainage area to existing BMP
No changes in hydrographs for Print Pack project

Composite Hydrograph Summary (cfs)

Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
area to BMP	2	2	3	14	27	50	54	33	17
Total (cfs)	2	2	3	14	27	50	54	33	17

Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	13.0 hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
area to BMP	11	9	7	6	5	4	4	3	3
Total (cfs)	11	9	7	6	5	4	4	3	3

Subarea Description	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
area to BMP	3	2	2	2	2	2	1	1	1
Total (cfs)	3	2	2	2	2	2	1	1	1

Subarea Description	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr
area to BMP	1	1	1	1	0
Total (cfs)	1	1	1	1	0

TR-55 TABULAR HYDROGRAPH METHOD
Type II. Distribution
(24 hr. Duration Storm)

Executed: 01-16-1998 11:11:39

Watershed file: --> C:\86120--1\WSHED .MOP

Hydrograph file: --> C:\86120--1\SITE10YR.HYD

PrintPack

86120-103

Hydrographs for drainage area to existing BMP
No changes in hydrographs for Print Pack project

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip. (in)	Runoff (in)	Ia/p input/used
area to BMP	18.70	90.0	0.20	0.00	5.04	3.91	I.04 .10

* Travel time from subarea outfall to composite watershed outfall point.

I -- Subarea where user specified interpolation between Ia/p tables.

Total area = 18.70 acres or 0.02922 sq.mi

Peak discharge = 91 cfs

>>>> Computer Modifications of Input Parameters <<<<

Subarea Description	Input Values		Rounded Values		Ia/p	Ia/p Messages
	Tc (hr)	* Tt (hr)	Tc (hr)	* Tt (hr)	Interpolated (Yes/No)	
area to BMP	0.17	0.00	0.20	0.00	No	Computed Ia/p < .1

* Travel time from subarea outfall to composite watershed outfall point.

TR-55 TABULAR HYDROGRAPH METHOD
Type II. Distribution
(24 hr. Duration Storm)

Executed: 01-16-1998 11:11:39
Watershed file: --> C:\86120-~1\WSHED .MOP
Hydrograph file: --> C:\86120-~1\SITE10YR.HYD

PrintPack
86120-103

Hydrographs for drainage area to existing BMP
No changes in hydrographs for Print Pack project

>>>> Summary of Subarea Times to Peak <<<<

Subarea	Peak Discharge at Composite Outfall (cfs)	Time to Peak at Composite Outfall (hrs)
----- area to BMP -----	91	12.2
----- Composite Watershed	91	12.2

TR-55 TABULAR HYDROGRAPH METHOD

Type II. Distribution
(24 hr. Duration Storm)

Executed: 01-16-1998 11:11:39

Watershed file: --> C:\86120--1\WSHED .MOP

Hydrograph file: --> C:\86120--1\SITE10YR.HYD

PrintPack

86120-103

Hydrographs for drainage area to existing BMP
No changes in hydrographs for Print Pack project

Composite Hydrograph Summary (cfs)

Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
area to BMP	3	4	5	24	46	84	91	55	29
Total (cfs)	3	4	5	24	46	84	91	55	29

Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	13.0 hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
area to BMP	19	15	12	10	8	7	6	6	5
Total (cfs)	19	15	12	10	8	7	6	6	5

Subarea Description	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
area to BMP	5	4	4	3	3	3	2	2	2
Total (cfs)	5	4	4	3	3	3	2	2	2

Subarea Description	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr
area to BMP	2	2	1	1	0
Total (cfs)	2	2	1	1	0

Print Pack
86120-103
existing volume from 87 calculations

CALCULATED 01-16-1998 11:12:41
DISK FILE: c:\86120-~1\EXIST .VOL

Planimeter scale: 1 inch = 1 ft.

Elevation (ft)	Planimeter (sq.in.)	Area (sq.ft)	A1+A2+sq ^r (A1*A2) (sq.ft)	* Volume (cubic-ft)	Volume Sum (cubic-ft)
66.50	0.00	0	0	0	0
67.00	1,980.00	1,980	1,980	330	330
67.30	*I*	2,926	7,313	731	1,061
67.40	*I*	3,282	7,812	1,042	1,372
68.00	5,850.00	5,850	11,233	3,744	4,074
69.00	12,690.00	12,690	27,156	9,052	13,126
69.40	*I*	15,090	41,618	5,549	18,676
69.90	*I*	18,383	46,346	13,904	27,030
70.00	19,080.00	19,080	47,330	15,777	28,903
70.30	*I*	20,728	59,695	5,970	34,873
70.60	*I*	22,445	62,219	12,444	41,347
71.00	24,840.00	24,840	65,690	21,897	50,800
72.00	31,140.00	31,140	83,792	27,931	78,731
73.00	37,440.00	37,440	102,725	34,242	112,972

I ---> Interpolated area from closest two planimeter readings.

$$IA = (\text{sq.rt}(\text{Area1}) + ((E_i - E_1) / (E_2 - E_1)) * (\text{sq.rt}(\text{Area2}) - \text{sq.rt}(\text{Area1})))^2$$

where: E1, E2 = Closest two elevations with planimeter data
E_i = Elevation at which to interpolate area
Area1, Area2 = Areas computed for E1, E2, respectively
IA = Interpolated area for E_i

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (EL2 - EL1) * (\text{Area1} + \text{Area2} + \text{sq.rt.}(\text{Area1} * \text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
Area1, Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

***** COMPOSITE OUTFLOW SUMMARY *****

Elevation (ft)	Q (cfs)	Contributing Structures
-----	-----	-----
66.50	0.0	1 +7
67.00	0.2	1 +7
67.50	0.8	3 +2 +6 +1 +7
68.00	9.4	3 +2 +6 +1 +7
68.50	16.3	3 +2 +6 +1 +7
69.00	19.7	3 +2 +6 +1 +7
69.50	22.5	3 +2 +6 +1 +7
70.00	25.1	3 +2 +6 +1 +7
70.50	27.4	3 +2 +6 +1 +7
71.00	29.6	3 +2 +6 +1 +7
71.50	31.6	3 +2 +6 +1 +7
72.00	33.4	3 +2 +6 +1 +7
72.50	35.2	3 +2 +6 +1 +7
73.00	36.9	3 +2 +6 +1 +7

1 1 1 1 1
2' riser 1" ϕ ? 1" ϕ ?

#4- 24" barrel

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

Outlet Structure File: c:\86120-~1\EXIST .STR
Planimeter Input File: c:\86120-~1\EXIST .VOL
Rating Table Output File: c:\86120-~1\EXIST .PND

Min. Elev.(ft) = 66.5 Max. Elev.(ft) = 73 Incr.(ft) = .5

Additional elevations (ft) to be included in table:
* * * * *

SYSTEM CONNECTIVITY

Structure	No.	Q Table	Q Table
-----	---	-----	-----
STAND PIPE	3		-> 3
CULVERT-CR	4	? 3	-> 5
ORIFICE-VC	2		-> 2
TABLE	6		-> 6
ORIFICE-VC	1		-> 1
TABLE	7		-> 7

Outflow rating table summary was stored in file:
c:\86120-~1\EXIST .PND

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

>>>>> Structure No. 3 <<<<<<
(Input Data)

STAND PIPE

Stand Pipe with weir or orifice flow

E1 elev.(ft)?	67.43
E2 elev.(ft)?	73.001
Crest elev.(ft)?	67.43
Diameter (ft)?	2
Weir coefficient?	3.3
Orifice coefficient?	0.6
Start transition elev.(ft) @ ?	
Transition height (ft)?	

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

>>>>> Structure No. 4 <<<<<<
(Input Data)

CULVERT-CR
Circular Culvert (With Inlet Control)

E1 elev.(ft)?	67.43
E2 elev.(ft)?	73.001
Diam. (ft)?	2
Inv. el.(ft)?	63.62
Slope (ft/ft)?	.02
T1 ratio?	
T2 ratio?	
K Coeff.?	.0078
M Coeff.?	2.0
c Coeff.?	.0379
Y Coeff.?	.69
Form 1 or 2?	1
Slope factor?	-0.5

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

>>>>> Structure No. 2 <<<<<
(Input Data)

ORIFICE-VC

Orifice - Vertical Circular

E1 elev.(ft)?	67.31
E2 elev.(ft)?	73.001
Orifice coeff.?	0.6
Invert elev.(ft)?	67.23
Datum elev.(ft)?	67.27
Diameter (ft)?	.083

1" ϕ

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

>>>>> Structure No. 6 <<<<<
(Input Data)

TABLE

Input your own rating table.

E1 (ft) =67.31 E2 (ft) = 73.001

Constant (ft) added to each elevation was:

Elev. (ft)	Q (cfs)
-----	-----
66.5	0
67	0
68	.18
69	.28
70	.35
71	.41
72	.46
73	.5

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

>>>>> Structure No. 1 <<<<<
(Input Data)

ORIFICE-VC

Orifice - Vertical Circular

E1 elev.(ft)?	66.2
E2 elev.(ft)?	73.001
Orifice coeff.?	0.6
Invert elev.(ft)?	66.12
Datum elev.(ft)?	66.16
Diameter (ft)?	.083

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

>>>>> Structure No. 7 <<<<<<
(Input Data)

TABLE

Input your own rating table.

E1 (ft) =66.2 E2 (ft) = 73.001

Constant (ft) added to each elevation was:

Elev. (ft)	Q (cfs)
-----	-----
66.5	0
67	.19
68	.28
69	.35
70	.41
71	.46
72	.51
73	.55

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

Outflow Rating Table for Structure #3

STAND PIPE Stand Pipe with weir or orifice flow

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
66.50	0.0	E < Inv.El.= 67.43
67.00	0.0	E < El=67.43
67.50	0.4	Weir: H =.07
68.00	8.9	Weir: H =.570
68.50	15.6	Orifice: H =1.07
69.00	19.0	Orifice: H =1.57
69.50	21.8	Orifice: H =2.07
70.00	24.2	Orifice: H =2.57
70.50	26.5	Orifice: H =3.07
71.00	28.6	Orifice: H =3.57
71.50	30.5	Orifice: H =4.07
72.00	32.3	Orifice: H =4.57
72.50	34.1	Orifice: H =5.07
73.00	35.7	Orifice: H =5.57

Weir Cw = 3.3 Weir length = 6.283186 ft

Orifice Co = .6 Orifice area = 3.141593 sq.ft.

$Q \text{ (cfs)} = (Cw * L * H^{1.5}) \text{ or } (Co * A * \text{sqr}(2*g*H))$

No transition used, transition height = 0.0

Weir equation = Orifice equation @ elev.= 68.15955 ft

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

Outflow Rating Table for Structure #4
CULVERT-CR Circular Culvert (With Inlet Control)

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation	Messages
66.50	0.0	E < E1=67.43	
67.00	0.0	E < E1=67.43	
67.50	25.7	Submerged:	HW =3.88
68.00	28.0	Submerged:	HW =4.38
68.50	30.3	Submerged:	HW =4.88
69.00	32.3	Submerged:	HW =5.38
69.50	34.3	Submerged:	HW =5.88
70.00	36.1	Submerged:	HW =6.38
70.50	37.9	Submerged:	HW =6.88
71.00	39.6	Submerged:	HW =7.38
71.50	41.2	Submerged:	HW =7.880
72.00	42.8	Submerged:	HW =8.38
72.50	44.3	Submerged:	HW =8.88
73.00	45.7	Submerged:	HW =9.38

Used Unsubmerged Equ. Form (1) for elev. less than 65.87 ft
Used Submerged Equation for elevations greater than 66.189 ft
HW=Headwater (ft) dc=Critical depth (ft) Ac=Area (sq.ft) at dc

Transition flows interpolated from the following values:
E1=65.87 ft; Q1=15.55 cfs; Dc=1.42 ft; E2=66.189 ft; Q2=17.77 cfs

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

Outflow Rating Table for Structure #2
ORIFICE-VC Orifice - Vertical Circular

Elevation (ft)	Q (cfs)	Computation Messages
66.50	0.0	E < E1=67.31
67.00	0.0	E < E1=67.31
67.50	0.0	H =.23
68.00	0.0	H =.73
68.50	0.0	H =1.23
69.00	0.0	H =1.73
69.50	0.0	H =2.23
70.00	0.0	H =2.73
70.50	0.0	H =3.23
71.00	0.1	H =3.73
71.50	0.1	H =4.23
72.00	0.1	H =4.73
72.50	0.1	H =5.23
73.00	0.1	H =5.73

C = .6 A = 5.410608E-03 sq.ft.

H (ft) = Table elev. - Datum elev. (67.27 ft)

Q (cfs) = C * A * $\text{sqr}(2g * H)$

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

Outflow Rating Table for Structure #6
TABLE Input your own rating table.

Elevation (ft)	Q (cfs)	Computation Messages
-----	-----	-----
66.50	0.0	E < E1=67.31
67.00	0.0	E < E1=67.31
67.50	0.1	Interpolated from input table
68.00	0.2	
68.50	0.2	Interpolated from input table
69.00	0.3	
69.50	0.3	Interpolated from input table
70.00	0.3	
70.50	0.4	Interpolated from input table
71.00	0.4	
71.50	0.4	Interpolated from input table
72.00	0.5	
72.50	0.5	Interpolated from input table
73.00	0.5	

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

Outflow Rating Table for Structure #1
ORIFICE-VC Orifice - Vertical Circular

Elevation (ft)	Q (cfs)	Computation Messages
66.50	0.0	H =.34
67.00	0.0	H =.84
67.50	0.0	H =1.34
68.00	0.0	H =1.84
68.50	0.0	H =2.34
69.00	0.0	H =2.84
69.50	0.0	H =3.34
70.00	0.1	H =3.84
70.50	0.1	H =4.34
71.00	0.1	H =4.84
71.50	0.1	H =5.34
72.00	0.1	H =5.84
72.50	0.1	H =6.34
73.00	0.1	H =6.84

C = .6 A = 5.410608E-03 sq.ft.

H (ft) = Table elev. - Datum elev. (66.16 ft)

Q (cfs) = C * A * $\text{sqr}(2g * H)$

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

Outflow Rating Table for Structure #7
TABLE Input your own rating table.

Elevation (ft)	Q (cfs)	Computation Messages
-----	-----	-----
66.50	0.0	
67.00	0.2	
67.50	0.2	Interpolated from input table
68.00	0.3	
68.50	0.3	Interpolated from input table
69.00	0.3	
69.50	0.4	Interpolated from input table
70.00	0.4	
70.50	0.4	Interpolated from input table
71.00	0.5	
71.50	0.5	Interpolated from input table
72.00	0.5	
72.50	0.5	Interpolated from input table
73.00	0.6	

Outlet Structure File: EXIST .STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
existing pond

Outflow Rating Table 5
Table 5 = 3 ? 4

Elevation (ft)	Q (cfs)	Contributing Structures
-----	-----	-----
66.50	0.0	-
67.00	0.0	-
67.50	0.4	3
68.00	8.9	3
68.50	15.6	3
69.00		

```

*****
*                                     *
*                               PrintPack. *
*                               86120-103  *
* Routing for existing BMP, existing structures *
*                                     *
*                                     *
*****
  
```

Inflow Hydrograph: c:\86120-~1\SITE2YR .HYD
 Rating Table file: c:\86120-~1\EXIST .PND

----INITIAL CONDITIONS----
 Elevation = 66.50 ft
 Outflow = 0.00 cfs
 Storage = 0 cu-ft

GIVEN POND DATA			INTERMEDIATE ROUTING COMPUTATIONS	
ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	2S/t (cfs)	2S/t + 0 (cfs)
66.50	0.0	0	0.0	0.0
67.00	0.2	330	1.8	2.0
67.50	0.8	1,718	9.5	10.3
68.00	9.4	4,075	22.6	32.0
68.50	16.3	7,745	43.0	59.3
69.00	19.7	13,126	72.9	92.6
69.50	22.5	20,216	112.3	134.8
70.00	25.1	28,903	160.6	185.7
70.50	27.4	39,132	217.4	244.8
71.00	29.6	50,800	282.2	311.8
71.50	31.6	63,978	355.4	387.0
72.00	33.4	78,731	437.4	470.8
72.50	35.2	95,064	528.1	563.3
73.00	36.9	112,972	627.6	664.5

Time increment (t) = 6.0 min.

POND-2 Version: 5.20 S/N:
EXECUTED: 01-16-1998 11:13:45

Page 6
Return Freq: 2 years

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: c:\86120--1\EXIST .PND
Inflow Hydrograph: c:\86120--1\SITE2YR .HYD
Outflow Hydrograph: c:\86120--1\OUT-2 .HYD

Starting Pond W.S. Elevation = 66.50 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow	=	54.00 cfs
Peak Outflow	=	25.83 cfs
Peak Elevation	=	70.16 ft

***** Summary of Approximate Peak Storage *****

Initial Storage	=	0 cu-ft
Peak Storage From Storm	=	32,150 cu-ft

Total Storage in Pond	=	32,150 cu-ft

Warning: Inflow hydrograph truncated on left side.

```

*****
*                                     *
*                               PrintPack *
*                               86120-103 *
*   Routing for existing BMP, existing structures *
*                                     *
*                                     *
*****
  
```

Inflow Hydrograph: c:\86120-~1\SITE10YR.HYD
 Rating Table file: c:\86120-~1\EXIST .PND

----INITIAL CONDITIONS----
 Elevation = 66.50 ft
 Outflow = 0.00 cfs
 Storage = 0 cu-ft

GIVEN POND DATA			INTERMEDIATE ROUTING COMPUTATIONS	
ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	2S/t (cfs)	2S/t + 0 (cfs)
66.50	0.0	0	0.0	0.0
67.00	0.2	330	1.8	2.0
67.50	0.8	1,718	9.5	10.3
68.00	9.4	4,075	22.6	32.0
68.50	16.3	7,745	43.0	59.3
69.00	19.7	13,126	72.9	92.6
69.50	22.5	20,216	112.3	134.8
70.00	25.1	28,903	160.6	185.7
70.50	27.4	39,132	217.4	244.8
71.00	29.6	50,800	282.2	311.8
71.50	31.6	63,978	355.4	387.0
72.00	33.4	78,731	437.4	470.8
72.50	35.2	95,064	528.1	563.3
73.00	36.9	112,972	627.6	664.5

Time increment (t) = 6.0 min.

POND-2 Version: 5.20 S/N:
EXECUTED: 01-16-1998 11:13:46

Page 6
Return Freq: 10 years

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: c:\86120-~1\EXIST .PND
Inflow Hydrograph: c:\86120-~1\SITE10YR.HYD
Outflow Hydrograph: c:\86120-~1\OUT-10 .HYD

Starting Pond W.S. Elevation = 66.50 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow	=	91.00 cfs
Peak Outflow	=	32.28 cfs
Peak Elevation	=	71.69 ft

***** Summary of Approximate Peak Storage *****

Initial Storage =

PROPOSED CONDITIONS

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

***** COMPOSITE OUTFLOW SUMMARY *****

Elevation (ft)	Q (cfs)	Contributing Structures
-----	-----	-----
66.50	0.0	1 - 8" orificia - weir
67.00	0.8	1
67.18	1.0	2 - 8" orificia
67.50	1.4	2
68.00	1.8	2
68.50	2.2	2
69.00	2.5	2
69.50	2.8	2
70.00	3.0	2
70.50	3.3	2 - 4' ϕ riser
71.00	14.0	3 +2
71.50	39.1	3 +2
72.00	46.6	4 +2
72.50	48.3	4 +2
73.00	49.9	4 +2

2' ϕ barrel

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outlet Structure File: c:\86120--1\MODEXST3.STR
Planimeter Input File: c:\86120--1\EXIST .VOL
Rating Table Output File: c:\86120--1\MODEXST3.PND

Min. Elev.(ft) = 66.5 Max. Elev.(ft) = 73 Incr.(ft) = .5

Additional elevations (ft) to be included in table:

* * * * *

67.18

SYSTEM CONNECTIVITY

Structure	No.	Q Table	Q Table
-----	---	-----	-----
STAND PIPE	3		-> 3
CULVERT-CR	4	? 3	-> 5
ORIFICE-VC	2		-> 2
TABLE	1		-> 1

Outflow rating table summary was stored in file:
c:\86120--1\MODEXST3.PND

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack

86120-103

change riser structure

>>>>> Structure No. 3 <<<<<<
(Input Data)

STAND PIPE

Stand Pipe with weir or orifice flow

E1 elev.(ft)?	70.6
E2 elev.(ft)?	73.001
Crest elev.(ft)?	70.6
Diameter (ft)?	4
Weir coefficient?	3.3
Orifice coefficient?	0.6
Start transition elev.(ft) @ ?	
Transition height (ft)?	

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

>>>>> Structure No. 4 <<<<<<
(Input Data)

CULVERT-CR

Circular Culvert (With Inlet Control)

E1 elev.(ft)?	70.6
E2 elev.(ft)?	73.001
Diam. (ft)?	2
Inv. el.(ft)?	63.62
Slope (ft/ft)?	.02
T1 ratio?	
T2 ratio?	
K Coeff.?	.0078
M Coeff.?	2.0
c Coeff.?	.0379
Y Coeff.?	.69
Form 1 or 2?	1
Slope factor?	-0.5

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

>>>>> Structure No. 2 <<<<<<
(Input Data)

ORIFICE-VC
Orifice - Vertical Circular

E1 elev.(ft)?	67.17
E2 elev.(ft)?	73.001
Orifice coeff.?	0.6
Invert elev.(ft)?	66.5
Datum elev.(ft)?	66.83
Diameter (ft)?	0.67

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

>>>>> Structure No. 1 <<<<<<
(Input Data)

TABLE

Input your own rating table.

E1 (ft) =66.5 E2 (ft) =67.16

Constant (ft) added to each elevation was:

Elev. (ft)	Q (cfs)
-----	-----
66.5	0
66.83	.5
67.16	1

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table for Structure #3
STAND PIPE Stand Pipe with weir or orifice flow

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
-----	-----	-----
66.50	0.0	E < Inv.El. = 70.6
67.00	0.0	E < E1=70.6
67.18	0.0	E < E1=70.6
67.50	0.0	E < E1=70.6
68.00	0.0	E < E1=70.6
68.50	0.0	E < E1=70.6
69.00	0.0	E < E1=70.6
69.50	0.0	E < E1=70.6
70.00	0.0	E < E1=70.6
70.50	0.0	E < E1=70.6
71.00	10.5	Weir: H =.4
71.50	35.4	Weir: H =.9
72.00	68.7	Weir: H =1.4
72.50	83.4	Orifice: H =1.9
73.00	93.7	Orifice: H =2.4

Weir Cw = 3.3 Weir length = 12.56637 ft

Orifice Co = .6 Orifice area = 12.56637 sq.ft.

$Q \text{ (cfs)} = (Cw * L * H^{1.5}) \text{ or } (Co * A * \text{sqr}(2*g*H))$

No transition used, transition height = 0.0

Weir equation = Orifice equation @ elev. = 72.05908 ft

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table for Structure #4
CULVERT-CR Circular Culvert (With Inlet Control)

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
66.50	0.0	E < E1=70.6
67.00	0.0	E < E1=70.6
67.18	0.0	E < E1=70.6
67.50	0.0	E < E1=70.6
68.00	0.0	E < E1=70.6
68.50	0.0	E < E1=70.6
69.00	0.0	E < E1=70.6
69.50	0.0	E < E1=70.6
70.00	0.0	E < E1=70.6
70.50	0.0	E < E1=70.6
71.00	39.6	Submerged: HW =7.38
71.50	41.2	Submerged: HW =7.880
72.00	42.8	Submerged: HW =8.38
72.50	44.3	Submerged: HW =8.88
73.00	45.7	Submerged: HW =9.38

Used Unsubmerged Equ. Form (1) for elev. less than 65.87 ft
Used Submerged Equation for elevations greater than 66.189 ft
HW=Headwater (ft) dc=Critical depth (ft) Ac=Area (sq.ft) at dc

Transition flows interpolated from the following values:
E1=65.87 ft; Q1=15.55 cfs; Dc=1.42 ft; E2=66.189 ft; Q2=17.77 cfs

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table for Structure #2
ORIFICE-VC Orifice - Vertical Circular

Elevation (ft)	Q (cfs)	Computation Messages
-----	-----	-----
66.50	0.0	E < E1=67.17
67.00	0.0	E < E1=67.17
67.18	1.0	H =.35
67.50	1.4	H =.67
68.00	1.8	H =1.17
68.50	2.2	H =1.67
69.00	2.5	H =2.17
69.50	2.8	H =2.67
70.00	3.0	H =3.17
70.50	3.3	H =3.67
71.00	3.5	H =4.17
71.50	3.7	H =4.67
72.00	3.9	H =5.17
72.50	4.0	H =5.67
73.00	4.2	H =6.17

C = .6 A = .3525653 sq.ft.

H (ft) = Table elev. - Datum elev. (66.83 ft)

Q (cfs) = C * A * sqr(2g * H)

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table for Structure #1

TABLE Input your own rating table.

Elevation (ft)	Q (cfs)	Computation Messages
-----	-----	-----
66.50	0.0	
67.00	0.8	Interpolated from input table
67.18	0.0	E = or > E2=67.16
67.50	0.0	E = or > E2=67.16
68.00	0.0	E = or > E2=67.16
68.50	0.0	E = or > E2=67.16
69.00	0.0	E = or > E2=67.16
69.50	0.0	E = or > E2=67.16
70.00	0.0	E = or > E2=67.16
70.50	0.0	E = or > E2=67.16
71.00	0.0	E = or > E2=67.16
71.50	0.0	E = or > E2=67.16
72.00	0.0	E = or > E2=67.16
72.50	0.0	E = or > E2=67.16
73.00	0.0	E = or > E2=67.16

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table 5
Table 5 = 3 ? 4

Elevation (ft)	Q (cfs)	Contributing Structures
66.50	0.0	-
67.00	0.0	-
67.18	0.0	-
67.50	0.0	-
68.00	0.0	-
68.50	0.0	-
69.00	0.0	-
69.50	0.0	-
70.00	0.0	-
70.50	0.0	-
71.00	10.5	3
71.50	35.4	3
72.00	42.8	4
72.50	44.3	4
73.00	45.7	4

```

*****
*                                     *
*                               PrintPack *
*                               86120-103 *
* Routing for existing BMP, modified structure *
*                                     *
*                                     *
*****
  
```

Inflow Hydrograph: c:\86120-~1\SITE2YR .HYD
 Rating Table file: c:\86120-~1\MODEXST3.PND

----INITIAL CONDITIONS----

Elevation = 66.50 ft
 Outflow = 0.00 cfs
 Storage = 0 cu-ft

GIVEN POND DATA

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)
66.50	0.0	0
67.00	0.8	330
67.18	1.0	734
67.50	1.4	1,718
68.00	1.8	4,075
68.50	2.2	7,745
69.00	2.5	13,126
69.50	2.8	20,216
70.00	3.0	28,903
70.50	3.3	39,132
71.00	14.0	50,800
71.50	39.1	63,978
72.00	46.6	78,731
72.50	48.3	95,064
73.00	49.9	112,972

INTERMEDIATE ROUTING
 COMPUTATIONS

2S/t (cfs)	2S/t + 0 (cfs)
0.0	0.0
1.8	2.6
4.1	5.1
9.5	10.9
22.6	24.4
43.0	45.2
72.9	75.4
112.3	115.1
160.6	163.6
217.4	220.7
282.2	296.2
355.4	394.5
437.4	484.0
528.1	576.4
627.6	677.5

Time increment (t) = 6.0 min.

POND-2 Version: 5.20 S/N:
EXECUTED: 01-16-1998 11:14:56

Page 6
Return Freq: 2 years

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: c:\86120--1\MODEXST3.PND
Inflow Hydrograph: c:\86120--1\SITE2YR .HYD
Outflow Hydrograph: c:\86120--1\MOD2 .HYD

Starting Pond W.S. Elevation = 66.50 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow	=	54.00 cfs
Peak Outflow	=	23.90 cfs
Peak Elevation	=	71.20 ft

***** Summary of Approximat

```

*****
*
*                      PrintPack
*                      86120-103
* Routing for existing BMP, modified structure
*
*
*****
  
```

Inflow Hydrograph: c:\86120-~1\SITE10YR.HYD
 Rating Table file: c:\86120-~1\MODEXST3.PND

----INITIAL CONDITIONS----
 Elevation = 66.50 ft
 Outflow = 0.00 cfs
 Storage = 0 cu-ft

GIVEN POND DATA			INTERMEDIATE ROUTING COMPUTATIONS	
ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)	2S/t (cfs)	2S/t + 0 (cfs)
66.50	0.0	0	0.0	0.0
67.00	0.8	330	1.8	2.6
67.18	1.0	734	4.1	5.1
67.50	1.4	1,718	9.5	10.9
68.00	1.8	4,075	22.6	24.4
68.50	2.2	7,745	43.0	45.2
69.00	2.5	13,126	72.9	75.4
69.50	2.8	20,216	112.3	115.1
70.00	3.0	28,903	160.6	163.6
70.50	3.3	39,132	217.4	220.7
71.00	14.0	50,800	282.2	296.2
71.50	39.1	63,978	355.4	394.5
72.00	46.6	78,731	437.4	484.0
72.50	48.3	95,064	528.1	576.4
73.00	49.9	112,972	627.6	677.5

Time increment (t) = 6.0 min.

POND-2 Version: 5.20 S/N:
EXECUTED: 01-16-1998 11:14:57

Page 6
Return Freq: 10 years

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: c:\86120--1\MODEXST3.PND
Inflow Hydrograph: c:\86120--1\SITE10YR.HYD
Outflow Hydrograph: c:\86120--1\MOD10 .HYD

Starting Pond W.S. Elevation = 66.50 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow	=	91.00 cfs
Peak Outflow	=	46.94 cfs
Peak Elevation	=	72.10 ft

***** Summary of Approximat

DRAWDOWN CALCULATIONS

EXECUTED: 01-16-1998 11:22:27

drawdown

Return Freq: 1 years

```

*****
*
*   Drawdown calculations *
*   Existing conditions   *
*
*
*
*****

```

Inflow Hydrograph: c:\86120--1\DRAWDOWN.HYD

Rating Table file: c:\86120--1\EXIST .PND

----INITIAL CONDITIONS----

Elevation = 70.60 ft
 Outflow = 27.84 cfs
 Storage = 41,465 cu-ft

GIVEN POND DATA

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)
66.50	0.0	0
67.00	0.2	330
67.50	0.8	1,718
68.00	9.4	4,075
68.50	16.3	7,745
69.00	19.7	13,126
69.50	22.5	20,216
70.00	25.1	28,903
70.50	27.4	39,132
71.00	29.6	50,800
71.50	31.6	63,978
72.00	33.4	78,731
72.50	35.2	95,064
73.00	36.9	112,972

INTERMEDIATE ROUTING
COMPUTATIONS

2S/t (cfs)	2S/t + 0 (cfs)
0.0	0.0
1.8	2.0
9.5	10.3
22.6	32.0
43.0	59.3
72.9	92.6
112.3	134.8
160.6	185.7
217.4	244.8
282.2	311.8
355.4	387.0
437.4	470.8
528.1	563.3
627.6	664.5

Time increment (t) = 6.0 min.

EXISTING

DRAWDOWN TIME = 882 - 660 = 222 MIN
 OR 3.7 HR

```

*****
*
*   Drawdown calculations
*   Change riser structure
*
*
*
*****

```

Inflow Hydrograph: c:\86120--1\DRAWDOWN.HYD

Rating Table file: c:\86120--1\MODEXST3.PND

----INITIAL CONDITIONS----

Elevation = 70.60 ft

Outflow = 5.44 cfs

Storage = 41,465 cu-ft

GIVEN POND DATA

ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (cu-ft)
66.50	0.0	0
67.00	0.8	330
67.18	1.0	734
67.50	1.4	1,718
68.00	1.8	4,075
68.50	2.2	7,745
69.00	2.5	13,126
69.50	2.8	20,216
70.00	3.0	28,903
70.50	3.3	39,132
71.00	14.0	50,800
71.50	39.1	63,978
72.00	46.6	78,731
72.50	48.3	95,064
73.00	49.9	112,972

INTERMEDIATE ROUTING
COMPUTATIONS

2S/t (cfs)	2S/t + 0 (cfs)
0.0	0.0
1.8	2.6
4.1	5.1
9.5	10.9
22.6	24.4
43.0	45.2
72.9	75.4
112.3	115.1
160.6	163.6
217.4	220.7
282.2	296.2
355.4	394.5
437.4	484.0
528.1	576.4
627.6	677.5

Time increment (t) = 6.0 min.

PROPOSED

DRAWDOWN TIME = 966 - 660 = 306 MIN

OR 5.1 HR

Pond File: c:\86120--1\MODEXST3.PND

Inflow Hydrograph: c:\86120--1\DRAWDOWN.HYD

Outflow Hydrograph: c:\86120--1\OUT.HYD

INFLOW HYDROGRAPH

ROUTING COMPUTATIONS

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - O (cfs)	2S/t + O (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
660.0	0.00	-----	224.9	235.8	5.44	70.60
666.0	0.00	0.0	217.1	224.9	3.90	70.53
672.0	0.00	0.0	210.6	217.1	3.28	70.47
678.0	0.00	0.0	204.1	210.6	3.25	70.41
684.0	0.00	0.0	197.6	204.1	3.21	70.35
690.0	0.00	0.0	191.3	197.6	3.18	70.30
696.0	0.00	0.0	185.0	191.3	3.15	70.24
702.0	0.00	0.0	178.8	185.0	3.11	70.19
708.0	0.00	0.0	172.6	178.8	3.08	70.13
714.0	0.00	0.0	166.5	172.6	3.05	70.08
720.0	0.00	0.0	160.5	166.5	3.02	70.03
726.0	0.00	0.0	154.5	160.5	2.99	69.97
732.0	0.00	0.0	148.6	154.5	2.96	69.91
738.0	0.00	0.0	142.7	148.6	2.94	69.85
744.0	0.00	0.0	136.9	142.7	2.91	69.78
750.0	0.00	0.0	131.1	136.9	2.89	69.72
756.0	0.00	0.0	125.4	131.1	2.87	69.66
762.0	0.00	0.0	119.7	125.4	2.84	69.61
768.0	0.00	0.0	114.0	119.7	2.82	69.55
774.0	0.00	0.0	108.5	114.0	2.79	69.49
780.0	0.00	0.0	103.0	108.5	2.75	69.42
786.0	0.00	0.0	97.5	103.0	2.71	69.35
792.0	0.00	0.0	92.2	97.5	2.67	69.28
798.0	0.00	0.0	87.0	92.2	2.63	69.21
804.0	0.00	0.0	81.8	87.0	2.59	69.15
810.0	0.00	0.0	76.7	81.8	2.55	69.08
816.0	0.00	0.0	71.7	76.7	2.51	69.02
822.0	0.00	0.0	66.7	71.7	2.46	68.94
828.0	0.00	0.0	61.9	66.7	2.41	68.86
834.0	0.00	0.0	57.2	61.9	2.37	68.78
840.0	0.00	0.0	52.5	57.2	2.32	68.70
846.0	0.00	0.0	48.0	52.5	2.27	68.62
852.0	0.00	0.0	43.5	48.0	2.23	68.55
858.0	0.00	0.0	39.2	43.5	2.17	68.46
864.0	0.00	0.0	35.0	39.2	2.08	68.36
870.0	0.00	0.0	31.0	35.0	2.00	68.26
876.0	0.00	0.0	27.2	31.0	1.93	68.16
882.0	0.00	0.0	23.5	27.2	1.85	68.07
888.0	0.00	0.0	19.9	23.5	1.77	67.96
894.0	0.00	0.0	16.6	19.9	1.67	67.83
900.0	0.00	0.0	13.5	16.6	1.57	67.71
906.0	0.00	0.0	10.5	13.5	1.47	67.59
912.0	0.00	0.0	7.8	10.5	1.37	67.48
918.0	0.00	0.0	5.4	7.8	1.18	67.33
924.0	0.00	0.0	3.4	5.4	1.02	67.20

Pond File: c:\86120--1\MODEXST3.PND
 Inflow Hydrograph: c:\86120--1\DRAWDOWN.HYD
 Outflow Hydrograph: c:\86120--1\OUT.HYD

INFLOW HYDROGRAPH

ROUTING COMPUTATIONS

TIME (min)	INFLOW (cfs)	I1+I2 (cfs)	2S/t - O (cfs)	2S/t + O (cfs)	OUTFLOW (cfs)	ELEVATION (ft)
930.0	0.00	0.0	1.6	3.4	0.86	67.05
936.0	0.00	0.0	0.6	1.6	0.50	66.81
942.0	0.00	0.0	0.3	0.6	0.20	66.62
948.0	0.00	0.0	0.1	0.3	0.08	66.55
954.0	0.00	0.0	0.0	0.1	0.03	66.52
960.0	0.00	0.0	0.0	0.0	0.01	66.51
966.0	0.00	0.0	0.0	0.0	0.00	66.50
972.0	0.00	0.0	0.0	0.0	0.00	66.50
978.0	0.00	0.0	0.0	0.0	0.00	66.50
984.0	0.00	0.0	0.0	0.0	0.00	66.50
990.0	0.00	0.0	0.0	0.0	0.00	66.50
996.0	0.00	0.0	0.0	0.0	0.00	66.50
1002.0	0.00	0.0	0.0	0.0	0.00	66.50
1008.0	0.00	0.0	0.0	0.0	0.00	66.50
1014.0	0.00	0.0	0.0	0.0	0.00	66.50
1020.0	0.00	0.0	0.0	0.0	0.00	66.50
1026.0	0.00	0.0	0.0	0.0	0.00	66.50
1032.0	0.00	0.0	0.0	0.0	0.00	66.50
1038.0	0.00	0.0	0.0	0.0	0.00	66.50
1044.0	0.00	0.0	0.0	0.0	0.00	66.50
1050.0	0.00	0.0	0.0	0.0	0.00	66.50
1056.0	0.00	0.0	0.0	0.0	0.00	66.50
1062.0	0.00	0.0	0.0	0.0	0.00	66.50
1068.0	0.00	0.0	0.0	0.0	0.00	66.50
1074.0	0.00	0.0	0.0	0.0	0.00	66.50
1080.0	0.00	0.0	0.0	0.0	0.00	66.50
1086.0	0.00	0.0	0.0	0.0	0.00	66.50
1092.0	0.00	0.0	0.0	0.0	0.00	66.50
1098.0	0.00	0.0	0.0	0.0	0.00	66.50
1104.0	0.00	0.0	0.0	0.0	0.00	66.50
1110.0	0.00	0.0	0.0	0.0	0.00	66.50
1116.0	0.00	0.0	0.0	0.0	0.00	66.50
1122.0	0.00	0.0	0.0	0.0	0.00	66.50
1128.0	0.00	0.0	0.0	0.0	0.00	66.50
1134.0	0.00	0.0	0.0	0.0	0.00	66.50
1140.0	0.00	0.0	0.0	0.0	0.00	66.50
1146.0	0.00	0.0	0.0	0.0	0.00	66.50
1152.0	0.00	0.0	0.0	0.0	0.00	66.50
1158.0	0.00	0.0	0.0	0.0	0.00	66.50
1164.0	0.00	0.0	0.0	0.0	0.00	66.50
1170.0	0.00	0.0	0.0	0.0	0.00	66.50
1176.0	0.00	0.0	0.0	0.0	0.00	66.50
1182.0	0.00	0.0	0.0	0.0	0.00	66.50
1188.0	0.00	0.0	0.0	0.0	0.00	66.50
1194.0	0.00	0.0	0.0	0.0	0.00	66.50
1200.0	0.00	0.0	0.0	0.0	0.00	66.50

TR-55 TABULAR HYDROGRAPH METHOD
 Type II. Distribution
 (24 hr. Duration Storm)

Executed: 01-29-1998 12:53:58
 Watershed file: --> C:\86120--1\WSHED .MOP
 Hydrograph file: --> C:\86120--1\SITE100.HYD

PrintPack
 86120-103

Hydrographs for drainage area to existing BMP
 No changes in hydrographs for Print Pack project

>>>> Input Parameters Used to Compute Hydrograph <<<<

Subarea Description	AREA (acres)	CN	Tc (hrs)	* Tt (hrs)	Precip. (in)	Runoff (in)	Ia/p input/used
area to BMP	18.70	90.0	0.20	0.00	7.68	6.49	I.03 .10

* Travel time from subarea outfall to composite watershed outfall point.
 I -- Subarea where user specified interpolation between Ia/p tables.

Total area = 18.70 acres or 0.02922 sq.mi
 Peak discharge = 152 cfs

>>>> Computer Modifications of Input Parameters <<<<

Subarea Description	Input Values		Rounded Values		Ia/p	Ia/p Messages
	Tc (hr)	* Tt (hr)	Tc (hr)	* Tt (hr)	Interpolated (Yes/No)	
area to BMP	0.17	0.00	0.20	0.00	No	Computed Ia/p < .1

* Travel time from subarea outfall to composite watershed outfall point.

TR-55 TABULAR HYDROGRAPH METHOD
Type II. Distribution
(24 hr. Duration Storm)

Executed: 01-29-1998 12:53:58
Watershed file: --> C:\86120-~1\WSHED .MOP
Hydrograph file: --> C:\86120-~1\SITE100.HYD

PrintPack
86120-103

Hydrographs for drainage area to existing BMP
No changes in hydrographs for Print Pack project

>>>> Summary of Subarea Times to Peak <<<<

Subarea	Peak Discharge at Composite Outfall (cfs)	Time to Peak at Composite Outfall (hrs)
-----	-----	-----
area to BMP	152	12.2
-----	-----	-----
Composite Watershed	152	12.2

TR-55 TABULAR HYDROGRAPH METHOD
 Type II. Distribution
 (24 hr. Duration Storm)

Executed: 01-29-1998 12:53:58
 Watershed file: --> C:\86120-~1\WSHED .MOP
 Hydrograph file: --> C:\86120-~1\SITE100.HYD

PrintPack
 86120-103

Hydrographs for drainage area to existing BMP
 No changes in hydrographs for Print Pack project

Composite Hydrograph Summary (cfs)

Subarea Description	11.0 hr	11.3 hr	11.6 hr	11.9 hr	12.0 hr	12.1 hr	12.2 hr	12.3 hr	12.4 hr
area to BMP	4	6	9	40	76	140	152	91	47
Total (cfs)	4	6	9	40	76	140	152	91	47

Subarea Description	12.5 hr	12.6 hr	12.7 hr	12.8 hr	13.0 hr	13.2 hr	13.4 hr	13.6 hr	13.8 hr
area to BMP	31	24	19	16	13	12	10	9	8
Total (cfs)	31	24	19	16	13	12	10	9	8

Subarea Description	14.0 hr	14.3 hr	14.6 hr	15.0 hr	15.5 hr	16.0 hr	16.5 hr	17.0 hr	17.5 hr
area to BMP	8	7	6	6	5	5	4	4	4
Total (cfs)	8	7	6	6	5	5	4	4	4

Subarea Description	18.0 hr	19.0 hr	20.0 hr	22.0 hr	26.0 hr
area to BMP	3	3	2	2	0
Total (cfs)	3	3	2	2	0

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

***** COMPOSITE OUTFLOW SUMMARY *****

Elevation (ft)	Q (cfs)	Contributing Structures
-----	-----	-----
66.50	0.0	1
67.00	0.8	1
67.18	1.0	2
67.50	1.4	2
68.00	1.8	2
68.50	2.2	2
69.00	2.5	2
69.50	2.8	2
70.00	3.0	2
70.50	3.3	2
71.00	14.0	3 +2
71.50	39.1	3 +2
72.00	46.6	4 +2
72.50	48.3	4 +2
73.00	49.9	4 +2
73.50	127.4	4 +2 +8
74.00	309.1	4 +2 +8

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outlet Structure File: C:\86120-~1\MODEXST3.STR
Planimeter Input File: C:\86120-~1\EXIST.VOL
Rating Table Output File: C:\86120-~1\MODEXST3.PND

Min. Elev.(ft) = 66.5 Max. Elev.(ft) = 74 Incr.(ft) = .5

Additional elevations (ft) to be included in table:

* * * * *

67.18

SYSTEM CONNECTIVITY

Structure	No.	Q Table	Q Table
-----	---	-----	-----
STAND PIPE	3		-> 3
CULVERT-CR	4	? 3	-> 5
ORIFICE-VC	2		-> 2
TABLE	1		-> 1
WEIR-VR	8		-> 8

Outflow rating table summary was stored in file:

C:\86120-~1\MODEXST3.PND

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

>>>>> Structure No. 3 <<<<<<
(Input Data)

STAND PIPE

Stand Pipe with weir or orifice flow

E1 elev.(ft)?	70.6
E2 elev.(ft)?	74.001
Crest elev.(ft)?	70.6
Diameter (ft)?	4
Weir coefficient?	3.3
Orifice coefficient?	0.6
Start transition elev.(ft) @ ?	
Transition height (ft)?	

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

>>>>> Structure No. 4 <<<<<<
(Input Data)

CULVERT-CR
Circular Culvert (With Inlet Control)

E1 elev.(ft)?	70.6
E2 elev.(ft)?	74.001
Diam. (ft)?	2
Inv. el.(ft)?	63.62
Slope (ft/ft)?	.02
T1 ratio?	
T2 ratio?	
K Coeff.?	.0078
M Coeff.?	2.0
c Coeff.?	.0379
Y Coeff.?	.69
Form 1 or 2?	1
Slope factor?	-0.5

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20
Date Executed:

S/N:
Time Executed:

Print Pack
86120-103
change riser structure

>>>>> Structure No. 2 <<<<<
(Input Data)

ORIFICE-VC
Orifice - Vertical Circular

E1 elev.(ft)?	67.17
E2 elev.(ft)?	74.001
Orifice coeff.?	0.6
Invert elev.(ft)?	66.5
Datum elev.(ft)?	66.83
Diameter (ft)?	0.67

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

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>>>>> Structure No. 1 <<<<<<
(Input Data)

TABLE

Input your own rating table.

E1 (ft) =66.5 E2 (ft) =67.16

Constant (ft) added to each elevation was:

Elev. (ft)	Q (cfs)
-----	-----
66.5	0
66.83	.5
67.16	1

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

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86120-103
change riser structure

>>>>> Structure No. 8 <<<<<<
(Input Data)

WEIR-VR

Weir - Vertical Rectangular

E1 elev.(ft)?	73.1
E2 elev.(ft)?	74.001
Weir coefficient?	3.0
Weir elev.(ft)?	73.1
Length (ft)?	100
Contracted/Suppressed (C/S)?	S

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

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Time Executed:

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86120-103
change riser structure

Outflow Rating Table for Structure #3

STAND PIPE Stand Pipe with weir or orifice flow

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation	Messages
-----	-----	-----	-----
66.50	0.0	E < Inv.El.=	70.6
67.00	0.0	E < E1=	70.6
67.18	0.0	E < E1=	70.6
67.50	0.0	E < E1=	70.6
68.00	0.0	E < E1=	70.6
68.50	0.0	E < E1=	70.6
69.00	0.0	E < E1=	70.6
69.50	0.0	E < E1=	70.6
70.00	0.0	E < E1=	70.6
70.50	0.0	E < E1=	70.6
71.00	10.5	Weir:	H =.4
71.50	35.4	Weir:	H =.9
72.00	68.7	Weir:	H =1.4
72.50	83.4	Orifice:	H =1.9
73.00	93.7	Orifice:	H =2.4
73.50	103.0	Orifice:	H =2.9
74.00	111.6	Orifice:	H =3.4

Weir Cw = 3.3 Weir length = 12.56637 ft
Orifice Co = .6 Orifice area = 12.56637 sq.ft.
 $Q \text{ (cfs)} = (Cw * L * H^{1.5}) \text{ or } (Co * A * \text{sqr}(2 * g * H))$
No transition used, transition height = 0.0
Weir equation = Orifice equation @ elev.= 72.05908 ft

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

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86120-103
change riser structure

Outflow Rating Table for Structure #4
CULVERT-CR Circular Culvert (With Inlet Control)

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation	Messages
-----	-----	-----	-----
66.50	0.0	E < E1=70.6	
67.00	0.0	E < E1=70.6	
67.18	0.0	E < E1=70.6	
67.50	0.0	E < E1=70.6	
68.00	0.0	E < E1=70.6	
68.50	0.0	E < E1=70.6	
69.00	0.0	E < E1=70.6	
69.50	0.0	E < E1=70.6	
70.00	0.0	E < E1=70.6	
70.50	0.0	E < E1=70.6	
71.00	39.6	Submerged:	HW =7.38
71.50	41.2	Submerged:	HW =7.880
72.00	42.8	Submerged:	HW =8.38
72.50	44.3	Submerged:	HW =8.88
73.00	45.7	Submerged:	HW =9.38
73.50	47.1	Submerged:	HW =9.88
74.00	48.4	Submerged:	HW =10.38

Used Unsubmerged Equ. Form (1) for elev. less than 65.87 ft
Used Submerged Equation for elevations greater than 66.189 ft
HW=Headwater (ft) dc=Critical depth (ft) Ac=Area (sq.ft) at dc

Transition flows interpolated from the following values:
E1=65.87 ft; Q1=15.55 cfs; Dc=1.42 ft; E2=66.189 ft; Q2=17.77 cfs

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20
Date Executed:

S/N:
Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table for Structure #2
ORIFICE-VC Orifice - Vertical Circular

Elevation (ft)	Q (cfs)	Computation Messages
-----	-----	-----
66.50	0.0	E < E1=67.17
67.00	0.0	E < E1=67.17
67.18	1.0	H =.35
67.50	1.4	H =.67
68.00	1.8	H =1.17
68.50	2.2	H =1.67
69.00	2.5	H =2.17
69.50	2.8	H =2.67
70.00	3.0	H =3.17
70.50	3.3	H =3.67
71.00	3.5	H =4.17
71.50	3.7	H =4.67
72.00	3.9	H =5.17
72.50	4.0	H =5.67
73.00	4.2	H =6.17
73.50	4.4	H =6.670
74.00	4.5	H =7.170

C = .6 A = .3525653 sq.ft.
H (ft) = Table elev. - Datum elev. (66.83 ft)
Q (cfs) = C * A * sqr(2g * H)

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table for Structure #1
TABLE Input your own rating table.

Elevation (ft)	Q (cfs)	Computation Messages
-----	-----	-----
66.50	0.0	
67.00	0.8	Interpolated from input table
67.18	0.0	E = or > E2=67.16
67.50	0.0	E = or > E2=67.16
68.00	0.0	E = or > E2=67.16
68.50	0.0	E = or > E2=67.16
69.00	0.0	E = or > E2=67.16
69.50	0.0	E = or > E2=67.16
70.00	0.0	E = or > E2=67.16
70.50	0.0	E = or > E2=67.16
71.00	0.0	E = or > E2=67.16
71.50	0.0	E = or > E2=67.16
72.00	0.0	E = or > E2=67.16
72.50	0.0	E = or > E2=67.16
73.00	0.0	E = or > E2=67.16
73.50	0.0	E = or > E2=67.16
74.00	0.0	E = or > E2=67.16

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table for Structure #8
WEIR-VR Weir - Vertical Rectangular

***** INLET CONTROL ASSUMED *****

Elevation (ft)	Q (cfs)	Computation Messages
-----	-----	-----
66.50	0.0	E < Inv.El. = 73.1
67.00	0.0	E < Inv.El. = 73.1
67.18	0.0	E < Inv.El. = 73.1
67.50	0.0	E < Inv.El. = 73.1
68.00	0.0	E < Inv.El. = 73.1
68.50	0.0	E < Inv.El. = 73.1
69.00	0.0	E < Inv.El. = 73.1
69.50	0.0	E < Inv.El. = 73.1
70.00	0.0	E < Inv.El. = 73.1
70.50	0.0	E < Inv.El. = 73.1
71.00	0.0	E < Inv.El. = 73.1
71.50	0.0	E < Inv.El. = 73.1
72.00	0.0	E < Inv.El. = 73.1
72.50	0.0	E < Inv.El. = 73.1
73.00	0.0	E < Inv.El. = 73.1
73.50	75.9	H = .4
74.00	256.1	H = .9

C = 3 L (ft) = 100

H (ft) = Table elev. - Invert elev. (73.1 ft)

Q (cfs) = C * L * (H**1.5) -- Suppressed Weir

Outlet Structure File: MODEXST3.STR

POND-2 Version: 5.20

S/N:

Date Executed:

Time Executed:

Print Pack
86120-103
change riser structure

Outflow Rating Table 5

Table 5 = 3 ? 4

Elevation (ft)	Q (cfs)	Contributing Structures
-----	-----	-----
66.50	0.0	-
67.00	0.0	-
67.18	0.0	-
67.50	0.0	-
68.00	0.0	-
68.50	0.0	-
69.00	0.0	-
69.50	0.0	-
70.00	0.0	-
70.50	0.0	-
71.00	10.5	3
71.50	35.4	3
72.00	42.8	4
72.50	44.3	4
73.00	45.7	4
73.50	47.1	4
74.00	48.4	4

```

*****
*
*   Print Pack   *
*   86120-103   *
*   existing pond *
*
*
*****

```

Inflow Hydrograph: C:\86120-~1\SITE100 .HYD
Rating Table file: C:\86120-~1\EXIST .PND

----INITIAL CONDITIONS----
Elevation = 66.50 ft
Outflow = 0.00 cfs
Storage = 0.00 ac-ft

GIVEN POND DATA			INTERMEDIATE ROUTING COMPUTATIONS	
ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (ac-ft)	2S/t (cfs)	2S/t + 0 (cfs)
66.50	0.0	0.000	0.0	0.0
67.00	0.2	0.008	1.8	2.0
67.50	0.8	0.039	9.5	10.3
68.00	9.4	0.094	22.6	32.0
68.50	16.3	0.178	43.0	59.3
69.00	19.7	0.301	72.9	92.6
69.50	22.5	0.464	112.3	134.8
70.00	25.1	0.664	160.6	185.7
70.50	27.4	0.898	217.4	244.8
71.00	29.6	1.166	282.2	311.8
71.50	31.6	1.469	355.4	387.0
72.00	33.4	1.807	437.4	470.8
72.50	35.2	2.182	528.1	563.3
73.00	36.9	2.593	627.6	664.5
73.50	114.4	3.023	731.6	846.0
74.00	296.2	3.453	835.6	1131.8

Time increment (t) = 6.0 min.

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: C:\86120-~1\EXIST .PND
Inflow Hydrograph: C:\86120-~1\SITE100 .HYD
Outflow Hydrograph: C:\86120-~1\OUT .HYD

Starting Pond W.S. Elevation = 66.50 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow = 152.00 cfs
Peak Outflow = 72.37 cfs
Peak Elevation = 73.23 ft

***** Summary of Approximate Peak Storage *****

Initial Storage = 0.00 ac-ft
Peak Storage From Storm = 2.79 ac-ft

Total Storage in Pond = 2.79 ac-ft

Warning: Inflow hydrograph truncated on left side.

Pond File: C:\86120-~1\EXIST .PND
Inflow Hydrograph: C:\86120-~1\SITE100 .HYD
Outflow Hydrograph: C:\86120-~1\OUT .HYD

EXECUTED: 01-29-1998
12:52:12

Peak Inflow = 152.00 cfs
Peak Outflow = 72.37 cfs
Peak Elevation = 73.23 ft

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*****
*
*          Print Pack          *
*          86120-103          *
*   change riser structure    *
*
*
*****
  
```

Inflow Hydrograph: C:\86120-~1\SITE100 .HYD
 Rating Table file: C:\86120-~1\MODEXST3.PND

----INITIAL CONDITIONS----
 Elevation = 66.50 ft
 Outflow = 0.00 cfs
 Storage = 0.00 ac-ft

GIVEN POND DATA			INTERMEDIATE ROUTING COMPUTATIONS	
ELEVATION (ft)	OUTFLOW (cfs)	STORAGE (ac-ft)	2S/t (cfs)	2S/t + 0 (cfs)
66.50	0.0	0.000	0.0	0.0
67.00	0.8	0.008	1.8	2.6
67.18	1.0	0.017	4.1	5.1
67.50	1.4	0.039	9.5	10.9
68.00	1.8	0.094	22.6	24.4
68.50	2.2	0.178	43.0	45.2
69.00	2.5	0.301	72.9	75.4
69.50	2.8	0.464	112.3	115.1
70.00	3.0	0.664	160.6	163.6
70.50	3.3	0.898	217.4	220.7
71.00	14.0	1.166	282.2	296.2
71.50	39.1	1.469	355.4	394.5
72.00	46.6	1.807	437.4	484.0
72.50	48.3	2.182	528.1	576.4
73.00	49.9	2.593	627.6	677.5
73.50	127.4	3.023	731.6	859.0
74.00	309.1	3.453	835.6	1144.7

Time increment (t) = 6.0 min.

***** SUMMARY OF ROUTING COMPUTATIONS *****

Pond File: C:\86120-~1\MODEXST3.PND
Inflow Hydrograph: C:\86120-~1\SITE100 .HYD
Outflow Hydrograph: C:\86120-~1\OUT .HYD

Starting Pond W.S. Elevation = 66.50 ft

***** Summary of Peak Outflow and Peak Elevation *****

Peak Inflow = 152.00 cfs
Peak Outflow = 111.46 cfs
Peak Elevation = 73.40 ft

***** Summary of Approximate Peak Storage *****

Initial Storage = 0.00 ac-ft
Peak Storage From Storm = 2.93 ac-ft

Total Storage in Pond = 2.93 ac-ft

Warning: Inflow hydrograph truncated on left side.

Pond File: C:\86120-~1\MODEXST3.PND
Inflow Hydrograph: C:\86120-~1\SITE100 .HYD
Outflow Hydrograph: C:\86120-~1\OUT .HYD

EXECUTED: 01-29-1998
12:52:30

Peak Inflow = 152.00 cfs
Peak Outflow = 111.46 cfs
Peak Elevation = 73.40 ft

Date Record Created:
Created By:

WS_BMPNO:
CC018

Print
Record

WATERSHED CC
BMP ID NO 018
PLAN NO SP-21-98
TAX PARCEL (50-2)(9-13)
PIN NO 5020900013
CONSTRUCTION DATE
PROJECT NAME PrintPack-Busch Corporate Center
FACILITY LOCATION 400 Packets Court
CITY-STATE Williamsburg, Va. 23185
CURRENT OWNER Busch Corporate Center Association
OWNER ADDRESS 100 Kingsmill Road
OWNER ADDRESS 2
CITY-STATE-ZIP CODE Williamsburg, Va. 23185
OWNER PHONE 253-3933
MAINT AGREEMENT Yes
EMERG ACTION PLAN No

PRINTED ON
Thursday, March 11, 2010
11:13:39 AM

MAINTENANCE PLAN No
SITE AREA acre 9.7
LAND USE
old BMP TYP Dry Pond
JCC BMP CODE F2 Dry ED with forebay
POINT VALUE 4

SVC DRAIN AREA acres 18.7

SERVICE AREA DESCRI Printpack & Eastern Intern
IMPERV AREA acres 0.00
RECV STREAM UT of Halfway Creek
EXT DET-WQ-CTRL Yes
WTR QUAL VOL acre-ft 0.94
CHAN PROT CTRL No
CHAN PROT VOL acre-ft 0
SW/FLOOD CONTROL Yes
GEOTECH REPORT No

CTRL STRUC DESC RCP Riser
CTRL STRUC SIZE inches 48
OTLT BARRL DESC CMP Barrel
OTLT BARRL SIZE Inch 24

EMERG SPILLWAY No
DESIGN HW ELEV 73.4
PERM POOL ELEV na
2-YR OUTFLOW cfs 23.90
10-YR OUTFLOW cfs 46.94
REC DRAWING No
CONSTR CERTIF No
LAST INSP DATE
INTERNAL RATING
MISC/COMMENTS
Offsite BMP, old BMP, riser modified to RCP.

Inspected by:

Get Last BMP No
Return to Menu

Additional Comments:

